

A P P E N D I X F

TRANSPORTATION IMPACT
ANALYSIS





HEXAGON TRANSPORTATION CONSULTANTS, INC.



Hyatt House Hotel

Transportation Impact Analysis



Prepared for:

Placeworks

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Executive Summary

This report presents the results of the Transportation Impact Analysis (TIA) prepared for the proposed Hyatt House hotel project on the KCR-owned parcel at the southeast quadrant of Wolfe Road and I-280 in the City of Cupertino, California. The approximately 2-acre site is within the Vallco Shopping District, and is currently used as a parking lot. The hotel is planned to be a 5-story building with 148 hotel rooms, with a full service restaurant, bar, and meeting space. The proposed development includes 73 spaces of surface parking and 83 spaces of garage parking. The project site provides two access driveways on Perimeter Road. Figure 1 shows the study area and project site location. Figure 2 shows the site plan.

This study was conducted for the purpose of identifying potential traffic impacts related to the proposed development. The impacts of the project were evaluated following the standards and methodologies set forth by the City of Cupertino, and the Valley Transportation Agency (VTA). The VTA administers the Santa Clara County Congestion Management Program (CMP). Since the project would generate less than 100 peak hour gross vehicle trips, a CMP analysis was not conducted. The study determined the traffic impacts of the project on nine (9) signalized intersections, and one (1) unsignalized intersection in the vicinity of the project site during the weekday AM and PM peak periods of traffic. Project impacts on other transportation facilities, such as bicycle facilities and transit services, were determined on the basis of engineering judgment.

Project Trip Generation

The rates for Hotel (ITE land use code 310) were applied to estimate the project trips. No trip credits were taken because the existing site is currently being used as a parking lot. Based on the ITE trip generation rates, the proposed project would generate a total of 1,209 daily trips, with 78 trips occurring during the AM peak hour and 89 trips occurring during the PM peak hour period.

Intersection Levels of Service

City of Cupertino Intersection Analysis

The results of the level of service analysis under all project conditions show that all study intersections would operate at an acceptable level of service (LOS D or better) during both the AM and PM peak hours of traffic.

CMP Intersection Analysis

The results of the level of service analysis under all project conditions show that all of the study CMP intersections would operate at an acceptable level of service (LOS E or better) during the AM and PM peak hours of traffic.

Other Transportation Issues

There are currently no sidewalks on Perimeter Road or on the connecting ramps to Wolfe Road. Thus, there are no good existing pedestrian connections from the hotel site to Wolfe Road, Vallco Parkway, or Stevens Creek Boulevard. The project will build a sidewalk on the north side of Perimeter Road, and two crosswalks connecting the project site to the rest of the Vallco Shopping District will be provided. Also, the proposed site plan shows a pedestrian connection from the hotel to the new sidewalk and crosswalks on Perimeter Road.

The project will also provide a new pathway along the west side of the project site. This path will connect to a future trail along the south side of I-280. Although the trail on the south side of I-280 has not yet been fully studied, the pathway connection provided by the project will significantly enhance bicycle and pedestrian connectivity if it is ever implemented.

Parking

The project will provide 156 parking spaces, which is a rate of 1.05 parking spaces per room. Hexagon surveyed parking utilization at three similar hotels in Cupertino, and the highest ratio was found to be 0.81 occupied spaces per room. The proposed 156 parking spaces would be adequate, based on the highest surveyed peak hour parking demand.

Based on Hexagon's site observations of unoccupied parking spaces, the 2-acre hotel site is not needed as part of the parking supply for the Vallco Shopping District. Therefore, the removal of the existing 2-acre parking lot would not adversely impact the availability of parking for the Vallco Shopping District. .

Table ES 1
Intersection Level of Service Summary

Study Number	Intersection	Peak Hour	Count Date	Existing		Existing + Project		Background		Background + Project				Cumulative		Cumulative + Project			
				Avg. Delay (sec)	LOS	Avg. Delay (sec)	LOS	Avg. Delay (sec)	LOS	Avg. Delay (sec)	LOS	Incr. In Crit. Delay (sec)	Incr. In Crit. V/C	Avg. Delay (sec)	LOS	Avg. Delay (sec)	LOS	Incr. In Crit. Delay (sec)	Incr. In Crit. V/C
1	Wolfe Rd & Homestead Rd	AM	05/05/11	34.0	C	34.0	C	40.9	D	41.2	D	0.4	0.004	39.9	D	40.0	D	0.3	0.004
		PM	05/05/11	39.2	D	39.1	D	50.5	D	50.8	D	0.4	0.003	51.6	D	51.9	D	0.5	0.003
2	Wolfe Rd & I-280 NB Ramp*	AM	05/05/11	12.8	B	12.7	B	18.0	B	18.0	B	0.1	0.001	18.1	B	18.1	B	0.1	0.001
		PM	05/05/11	13.4	B	13.4	B	26.8	C	27.6	C	0.7	0.003	27.6	C	27.9	C	0.2	0.003
3	Wolfe Rd & I-280 SB Ramp*	AM	05/05/11	10.6	B	10.6	B	17.9	B	18.1	B	0.4	0.004	19.2	B	19.4	B	0.5	0.004
		PM	05/05/11	6.5	A	6.5	A	11.2	B	11.4	B	0.3	0.009	29.3	C	30.9	C	2.8	0.009
4	Wolfe Rd & Vallco Pkwy	AM	05/04/11	18.1	B	18.1	B	27.3	C	27.3	C	0.0	0.002	28.0	C	28.0	C	0.0	0.002
		PM	05/04/11	29.8	C	29.9	C	38.8	D	38.9	D	0.1	0.002	35.0	C	35.1	D	0.1	0.002
5	Wolfe Rd & Stevens Creek Blvd*	AM	05/05/11	37.4	D	37.4	D	39.5	D	39.6	D	0.1	0.003	39.9	D	40.0	D	0.2	0.003
		PM	05/05/11	38.3	D	38.3	D	43.8	D	44.0	D	0.3	0.003	49.2	D	49.6	D	0.6	0.003
6	Perimeter Rd & Vallco Pkwy	AM	05/15/14	6.1	A	7.0	A	4.1	A	4.3	A	0.2	0.007	4.1	A	4.2	A	0.2	0.007
		PM	05/15/14	11.1	B	11.5	B	6.8	A	7.1	A	0.3	0.008	6.0	A	6.1	A	0.1	0.008
7	Finch Ave & Vallco Pkwy ¹²	AM	05/13/14	9.4	B	9.5	B	25.7	C	25.9	C	0.2	0.008	25.8	C	26.0	C	0.2	0.008
		PM	05/13/14	1.3	B	1.3	B	37.9	D	39.0	D	1.1	0.009	38.1	D	39.2	D	1.1	0.009
8	Finch Ave & Stevens Creek Blvd	AM	05/12/11	23.5	C	23.6	C	23.8	C	23.8	C	0.0	0.000	26.3	C	26.3	C	0.0	0.000
		PM	05/12/11	23.5	C	23.6	C	31.9	C	31.9	C	0.0	0.000	44.5	D	44.5	D	0.0	0.000
9	N Tantau Ave & Vallco Pkwy	AM	06/01/11	24.1	C	24.5	C	28.4	C	28.5	C	0.1	0.001	27.2	C	27.3	C	0.1	0.001
		PM	06/01/11	27.8	C	28.2	C	34.0	C	34.3	C	0.4	0.008	33.4	C	33.7	C	0.4	0.008
10	N Tantau Ave & Stevens Creek Blvd	AM	05/12/11	38.9	D	38.7	D	43.2	D	43.2	D	0.0	0.000	43.4	D	43.4	D	0.0	0.000
		PM	05/12/11	38.8	D	38.6	D	46.8	D	47.0	D	0.2	0.001	49.4	D	49.6	D	0.3	0.001

Notes:

* denotes CMP intersection

¹ This intersection was analyzed as an unsignalized intersection under existing conditions and as a signalized intersection under background and cumulative conditions.² LOS for unsignalized intersection represents worst movement LOS**Bold** indicates a significant impact

1.

Introduction

This report presents the results of the Transportation Impact Analysis (TIA) prepared for the proposed Hyatt House hotel project on the KCR-owned parcel at the southeast quadrant of Wolfe Road and I-280 in the City of Cupertino, California. The approximately 2-acre site is within the Vallco Shopping District, and is currently used as a parking lot. The hotel is planned to be a 5-story building with 148 hotel rooms, with a full service restaurant, bar, and meeting space. The proposed development includes 73 spaces of surface parking and 83 spaces of garage parking. The project site provides two access driveways on Perimeter Road. Figure 1 shows the study area and project site location. Figure 2 shows the site plan.

Scope of Study

This study was conducted for the purpose of identifying potential traffic impacts related to the proposed development. The impacts of the project were evaluated following the standards and methodologies set forth by the City of Cupertino, and the Valley Transportation Agency (VTA). The VTA administers the Santa Clara County Congestion Management Program (CMP). Since the project would generate less than 100 peak hour gross vehicle trips, a CMP analysis was not conducted. The study determined the traffic impacts of the project on nine (9) signalized intersections, and one (1) unsignalized intersection in the vicinity of the project site during the weekday AM and PM peak periods of traffic. The following intersections were studied.

Study Intersections

1. Wolfe Road and Homestead Road
2. Wolfe Road and I-280 NB Ramp*
3. Wolfe Road and I-280 SB Ramp*
4. Wolfe Road and Vallco Parkway
5. Wolfe Road and Stevens Creek Boulevard*
6. Perimeter Road and Vallco Parkway
7. Finch Avenue and Vallco Parkway (unsignalized)
8. Finch Avenue and Stevens Creek Boulevard
9. N Tantau Avenue and Vallco Parkway
10. N Tantau Avenue and Stevens Creek Boulevard

Asterisk (*) denotes CMP intersections.

Currently, Finch Road is being reconstructed between Vallco Parkway and Stevens Creek Boulevard as part of the Main Street Development and is closed to traffic. The intersection of Vallco Parkway and Finch Road was analyzed as an unsignalized intersection under existing conditions as this intersection operated with stop control on Finch Avenue at the time the counts were conducted. This intersection is proposed to be signalized with the completion of the Main Street project.



LEGEND

= Project Site Location

(X) = Study Intersection

Figure 1
Site Location and Study Intersections

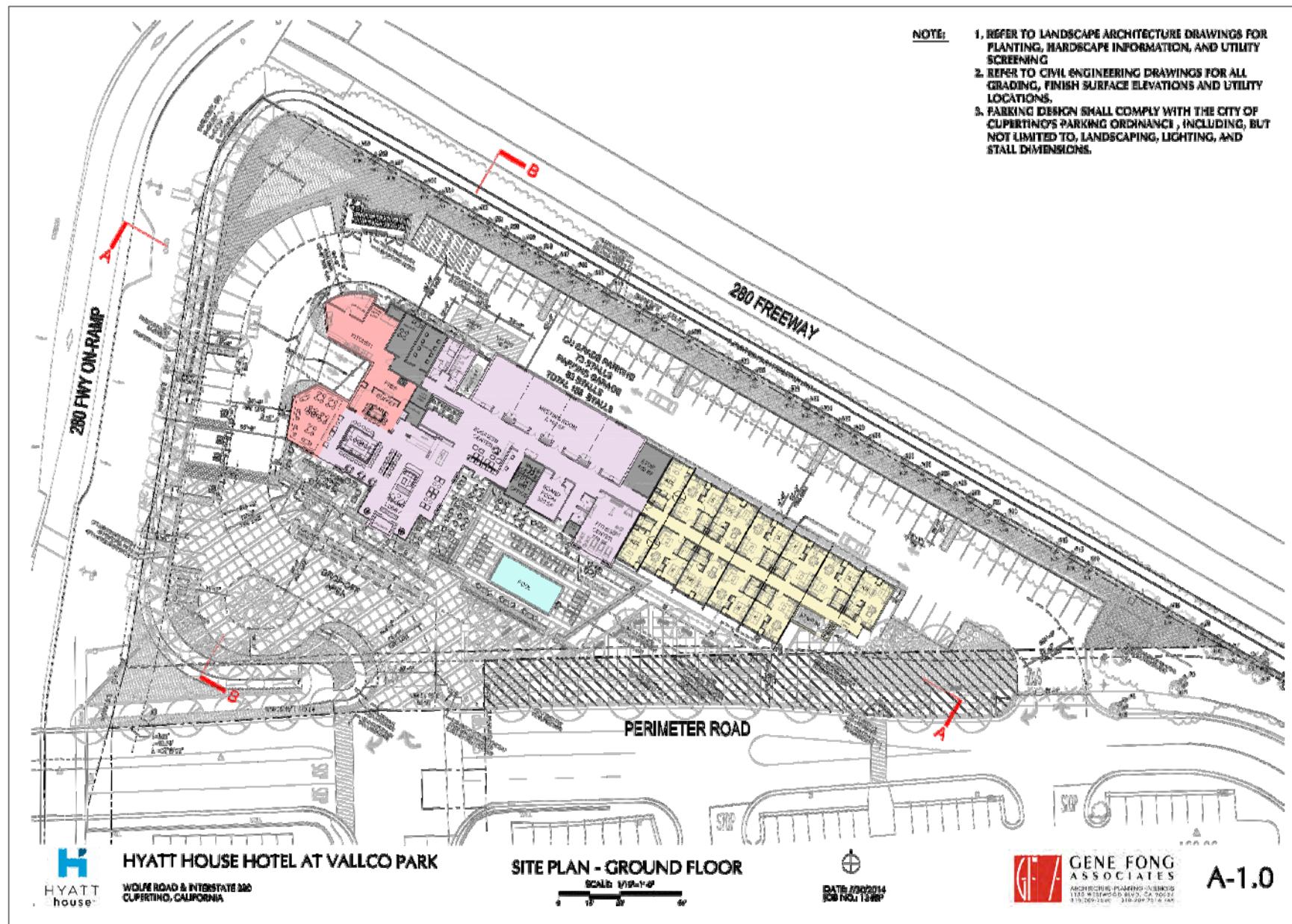


Figure 2
Project Site Plan

Traffic conditions at the study intersections were analyzed for the weekday AM and PM peak hours of traffic. The AM peak hour of traffic is generally between 7:00 and 9:00 AM, and the PM peak hour of traffic is typically between 4:00 and 6:00 PM. It is during these periods on an average weekday that the most congested traffic conditions occur.

Traffic conditions were evaluated for the following scenarios:

Scenario 1: *Existing Conditions.* Existing traffic volumes for seven of the study intersections were obtained from the City of Cupertino (consistent with the Apple Campus 2 EIR traffic counts), the 2012 CMP count database, and new 2014 manual turning-movement counts (at Vallco Parkway/Perimeter Road and Vallco Parkway/Finch Avenue). The new count data are included in Appendix A.

Scenario 2: *Existing Plus Project Conditions.* Existing plus project peak hour traffic volumes were estimated by adding to existing traffic volumes the additional traffic generated by the project. Existing plus project conditions were evaluated relative to existing conditions in order to determine the effects the project would have on existing traffic conditions.

Scenario 3: *Background Conditions.* Background traffic volumes were estimated by adding to existing peak hour volumes the projected volumes from approved but not yet completed developments. The added traffic from approved but not yet completed developments was based on trips generated by approved projects assumed for the Apple Campus 2 EIR, including the Apple Campus. Background conditions are consistent with the background plus project conditions assumed for the Apple Campus 2 EIR.

Scenario 4: *Background Plus Project Conditions.* Projected near-term peak hour traffic volumes with the project were estimated by adding to background traffic volumes the additional traffic generated by the project. Background plus project conditions were evaluated relative to background conditions in order to determine potential project impacts according to the City of Cupertino and VTA Level of Service Policy.

Scenario 5: *Cumulative Conditions.* Cumulative traffic conditions were analyzed by adding the traffic generated by pending developments to background plus project conditions. The traffic generated by pending developments was based on the cumulative plus project conditions presented in the Apple Campus 2 EIR. This traffic scenario was evaluated in order to fulfill the CEQA and CMP requirements.

Study Methodology

This section describes the methods used to determine the traffic conditions for each scenario described above. It includes descriptions of the data requirements, the analysis methodologies, and the applicable level of service standards.

Data Requirements

The data required for the analysis were obtained from new traffic counts, the City of Cupertino, the 2012 CMP Annual Monitoring Report, previous traffic studies and field observations. The following data were collected from these sources:

- existing traffic volumes
- intersection lane configurations
- signal timing and phasing
- approved & pending project trips

Analysis Methodologies and Level of Service Standards

Traffic conditions at the study intersections were evaluated using level of service (LOS). *Level of Service* is a qualitative description of operating conditions ranging from LOS A, or free-flow conditions with little or

no delay, to LOS F, or jammed conditions with excessive delays. The various analysis methods are described below.

City of Cupertino Intersections

The City of Cupertino level of service methodology for signalized intersections is the 2000 *Highway Capacity Manual* (HCM) method. This method is applied using the TRAFFIX software. The 2000 HCM operations method evaluates signalized intersection operations on the basis of average control delay time for all vehicles at the intersection. Since TRAFFIX is also the CMP-designated intersection level of service methodology, the City of Cupertino methodology employs the CMP default values for the analysis parameters.

The City of Cupertino level of service standard for signalized intersections is LOS D or better. The correlation between average control delay and level of service is shown in Table 2.

CMP Intersections

The designated level of service methodology for the CMP also is the 2000 HCM operations method for signalized intersections, using TRAFFIX. The only difference in level of service standards is that in the City of Cupertino the standard is LOS D or better, and the CMP level of service standard for signalized intersections is LOS E or better.

Table 2
Intersection Level of Service Definitions for Signalized Intersections

Level of Service	Description	Average Control Delay Per Vehicle (sec.)
A	Signal progression is extremely favorable. Most vehicles arrive during the green phase and do not stop at all. Short cycle lengths may also contribute to the very low vehicle delay.	10.0 or less
B	Operations characterized by good signal progression and/or short cycle lengths. More vehicles stop than with LOS A, causing higher levels of average vehicle delay.	10.1 to 20.0
C	Higher delays may result from fair signal progression and/or longer cycle lengths. Individual cycle failures may begin to appear at this level. The number of vehicles stopping is significant, though may still pass through the intersection without stopping.	20.1 to 35.0
D	The influence of congestion becomes more noticeable. Longer delays may result from some combination of unfavorable signal progression, long cycle lengths, or high volume-to-capacity (V/C) ratios. Many vehicles stop and individual cycle failures are noticeable.	35.1 to 55.0
E	This is considered to be the limit of acceptable delay. These high delay values generally indicate poor signal progression, long cycle lengths, and high volume-to-capacity (V/C) ratios. Individual cycle failures occur frequently.	55.1 to 80.0
F	This level of delay is considered unacceptable by most drivers. This condition often occurs with oversaturation, that is, when arrival flow rates exceed the capacity of the intersection. Poor progression and long cycle lengths may also be major contributing causes of such delay levels.	greater than 80.0

Source: Transportation Research Board, 2000 *Highway Capacity Manual* (Washington, D.C., 2000) p10-16.

Intersection Operations

The analysis of intersection level of service is often supplemented with an analysis of intersection operations for selected intersections where the project would add a significant number of left-turning vehicles. The operations analysis is based on vehicle queuing for high-demand turning-movements at intersections. Vehicle queues are estimated using a Poisson probability distribution, which estimates the probability of "n" vehicles for a vehicle movement using the following formula:

$$P(x=n) = \frac{\lambda^n e^{-(\lambda)}}{n!}$$

where:

P(x=n) = probability of "n" vehicles in queue per lane

n = number of vehicles in the queue per lane

λ = Avg. # of vehicles in the queue per lane (vehicles per hr per lane/signal cycles per hr)

The basis of the analysis is as follows: (1) the Poisson probability distribution is used to estimate the 95th percentile maximum number of queued vehicles per signal cycle for a particular movement; (2) the estimated maximum number of vehicles in the queue is translated into a queue length, assuming 25 feet per vehicle; and (3) the estimated maximum queue length is compared to the existing or planned available storage capacity for the movement. This analysis thus provides a basis for estimating future left-turn storage requirements at signalized intersections.

The 95th percentile queue length value indicates that during the peak hour, a queue of this length or less would occur on 95 percent of the signal cycles. Or, a queue length larger than the 95th percentile queue would only occur on 5 percent of the signal cycles (about 3 cycles during the peak hour for a signal with a 60-second cycle length). Therefore, left-turn storage pocket designs based on the 95th percentile queue length would ensure that storage space would be exceeded only 5 percent of the time. The 95th percentile queue length is also known as the "design queue length."

Unsignalized Intersections

One of the study intersections is unsignalized: Vallco Parkway and Finch Avenue. Finch Avenue is currently closed to vehicular access. It is being reconstructed as a private street between Stevens Creek Boulevard and Vallco Parkway, through the Main Street development. The intersection of Vallco Parkway and Finch Avenue will be signalized with the completion of the Main Street development. Since this intersection operated as an unsignalized intersection with a stop control on Finch Avenue when the traffic counts were conducted, this intersection was analyzed as an unsignalized intersection under existing conditions. Under background and cumulative conditions, this intersection was analyzed as a signalized intersection. Table 3 shows the level of service definitions for unsignalized intersections.

Table 3
Unsignalized Intersection Level of Service Definitions Based on Control Delay

Level of Service	Description	Average Delay Per Vehicle (Sec.)
A	Little or no traffic delay	10.0 or less
B	Short traffic delays	10.1 to 15.0
C	Average traffic delays	15.1 to 25.0
D	Long traffic delays	25.1 to 35.0
E	Very long traffic delays	35.1 to 50.0
F	Extreme traffic delays	greater than 50.0

Source: Transportation Research Board, *2000 Highway Capacity Manual* (Washington, D.C., 2000) p17-2.

Report Organization

The remainder of this report is divided into seven chapters. Chapter 2 describes existing conditions including the existing roadway network, transit service, and existing bicycle and pedestrian facilities. Chapter 3 presents the intersection operations under existing plus project conditions and describes the method used to estimate project traffic. Chapter 4 presents the intersection operations under background conditions. Chapter 5 presents the intersection operations under background plus project conditions and describes the project's impact on the near-term transportation system. Chapter 6 describes non-level of service operational issues associated with the proposed project. Chapter 7 describes cumulative traffic conditions. Chapter 8 presents the conclusions of the traffic study.

2. Existing Conditions

This chapter describes the existing conditions for all of the major transportation facilities in the vicinity of the site, including the roadway network, transit service, and bicycle and pedestrian facilities. Also included are the existing levels of service of the key intersections in the study area.

Existing Roadway Network

Regional access to the project site is provided by I-280 and local access is provided by Wolfe Road, Vallco Parkway, Stevens Creek Boulevard, Tantau Avenue and Perimeter Road. These facilities and other roadways in the vicinity of the project site are described below.

I-280 is an eight-lane freeway (three mixed-flow lanes and one HOV lane in each direction) and is located immediately north of the project site. I-280 is aligned in an east-west orientation within the project vicinity and provides regional freeway access between the cities of San Francisco and San Jose. Cupertino access to/from I-280 is provided via interchanges with De Anza Boulevard, Wolfe Road, Stevens Creek Boulevard, and Lawrence Expressway. I-280 provides connections to SR 85 approximately 2.5 miles west of the Wolfe Road interchange and to I-880 approximately 4.5 miles east of the Wolfe Road interchange.

Homestead Road is a four-lane, east-west roadway that extends from Foothill Expressway in the west to Lafayette Street in the east. Homestead Road is located to the north of the project site and provides connections to Wolfe Road and Tantau Avenue.

Wolfe Road is a four-to-six-lane north-south roadway and is located immediately west of the project site. It extends north to the City of Sunnyvale and south to the City of Saratoga. North of Stevens Creek Boulevard the roadway is called Wolfe Road and south of Stevens Creek Boulevard it is called Miller Avenue. Wolfe Road has an interchange with I-280. Wolfe Road provides access to the project site via a grade-separated interchange with Perimeter Road.

Tantau Avenue is a two-lane to four-lane, north-south roadway that extends from Bollinger Road in the south to Homestead Road in the north. Tantau Avenue is located to the east of the project site and provides connections to Stevens Creek Boulevard and to Vallco Parkway. Through traffic is not allowed on Tantau Avenue at Stevens Creek Boulevard.

Vallco Parkway is a short (less than 0.5 mile) six-lane, east-west roadway that provides a connection between Wolfe Road and Tantau Avenue. Entitled development projects, including JC Penney, Rose bowl, and Main Street, are located along Vallco Parkway. The lane configuration of Vallco Parkway will be modified in conjunction with these development projects to four travel lanes with some on-street parking. The road currently has one signalized intersection at Perimeter Road. With the new development projects, two additional traffic lights will be added: one at Finch Avenue (Main Street) and the other at the new entrance to the Main Street garage between Finch Avenue and Tantau Avenue.



Perimeter Road is a two-lane roadway connecting Stevens Creek Boulevard, Vallco Parkway and Wolfe Road. Perimeter Road runs under Wolfe Road, and connects via ramps to and from Wolfe Road. Access to the project site is provided via two driveways on Perimeter Road.

Finch Avenue is a two-lane north-south roadway that connects Vallco Parkway on the north and Phil Lane on the south. Finch Avenue is currently being reconstructed as part of the Main Street project. It will be reconstructed as a private street between Stevens Creek Boulevard and Vallco Parkway through the Main Street development, and its intersection with Vallco Parkway will be signalized. South of Stevens Creek Boulevard, Finch Avenue provides access to Cupertino High School.

Stevens Creek Boulevard is an east-west six-lane divided arterial that connects western Cupertino to downtown San Jose (via West San Carlos Street). Stevens Creek Boulevard provides access to SR 85, I-280 and Lawrence Expressway via interchanges.

Existing Pedestrian and Bicycle Facilities

Pedestrian Facilities

Pedestrian facilities consist of sidewalks along most of the streets in the immediate vicinity of the project site and crosswalks with pedestrian signal heads at all of the signalized intersections. Sidewalks are provided on both sides of Homestead Road, Wolfe Road, Stevens Creek Boulevard and Vallco Parkway. However, there are no sidewalks on Perimeter Road or on the connecting ramps to Wolfe Road in the immediate vicinity of the project site.

Bicycle Facilities

Bicycle facilities include bike paths, bike lanes, and bike routes. Bike paths (Class I facilities) are pathways, separate from roadways that are designated for use by bicycles. Often, these pathways also allow pedestrian access. Bike lanes (Class II facilities) are lanes on roadways designated for use by bicycles with special lane markings, pavement legends, and signage. Bike routes (Class III) are existing right-of-ways that accommodate bicycles but are not separate from the existing travel lanes. Routes are typically designated only with signs. Existing bicycle facilities within the study area are shown on Figure 3.

Within the immediate surroundings of the project site, Homestead Road Wolfe Road, Stevens Creek Boulevard, Tantau Avenue north of Stevens Creek, Vallco Parkway between Wolfe and Tantau, and Pruneridge Avenue between Wolfe and Tantau all have bike lanes. Miller Avenue and Tantau Avenue south of Stevens Creek both are identified as bike routes.

Existing Transit Services

Existing transit services in the study area are provided by the Santa Clara Valley Transportation Authority (VTA) and are described below. VTA operates bus service in Cupertino. The local bus lines that serve the project study area are shown on Figure 4.

VTA Bus Service

The VTA bus lines that operate within the study area are listed in Table 4, including their terminus points and commute hour headways.

Table 4
Existing Weekday VTA Bus Service

Bus Route	Route Description	Weekday Hours of Operation	Headway During Commute Periods
Local Route 23	Alum Rock Transit Center to De Anza College	5:20 AM - 1:00 AM	12 min.
Local Route 26	Eastridge Transit Center to Sunnyvale/Lockheed Martin Transit Center	5:20 AM - 11:50 PM	30 min.
Local Route 81	San Jose State University to Vallco Shopping Mall	6:25 AM - 8:50 PM	30 min.
Express Route 101	Camden & Hwy 85 to Palo Alto	6:50-7:50 AM/ 4:50-6:00 PM	2 Trips Each Direction Daily
Express Route 182	IBM/Bailey Avenue to Palo Alto	5:50 AM/7:50 PM 5:35-6:15 PM	1 Trip Each Direction Daily

Source: VTA, May 2014.

Route 23 is a local bus route that provides service between East San Jose and the De Anza College via Stevens Creek Boulevard near the site. Weekday hours of operation are from 5:20 am to 1:00 am with 12 to 30 minute headways. On weekends, this route operates on 15 to 30 minute headways between 6:15 am and 1:00 am.

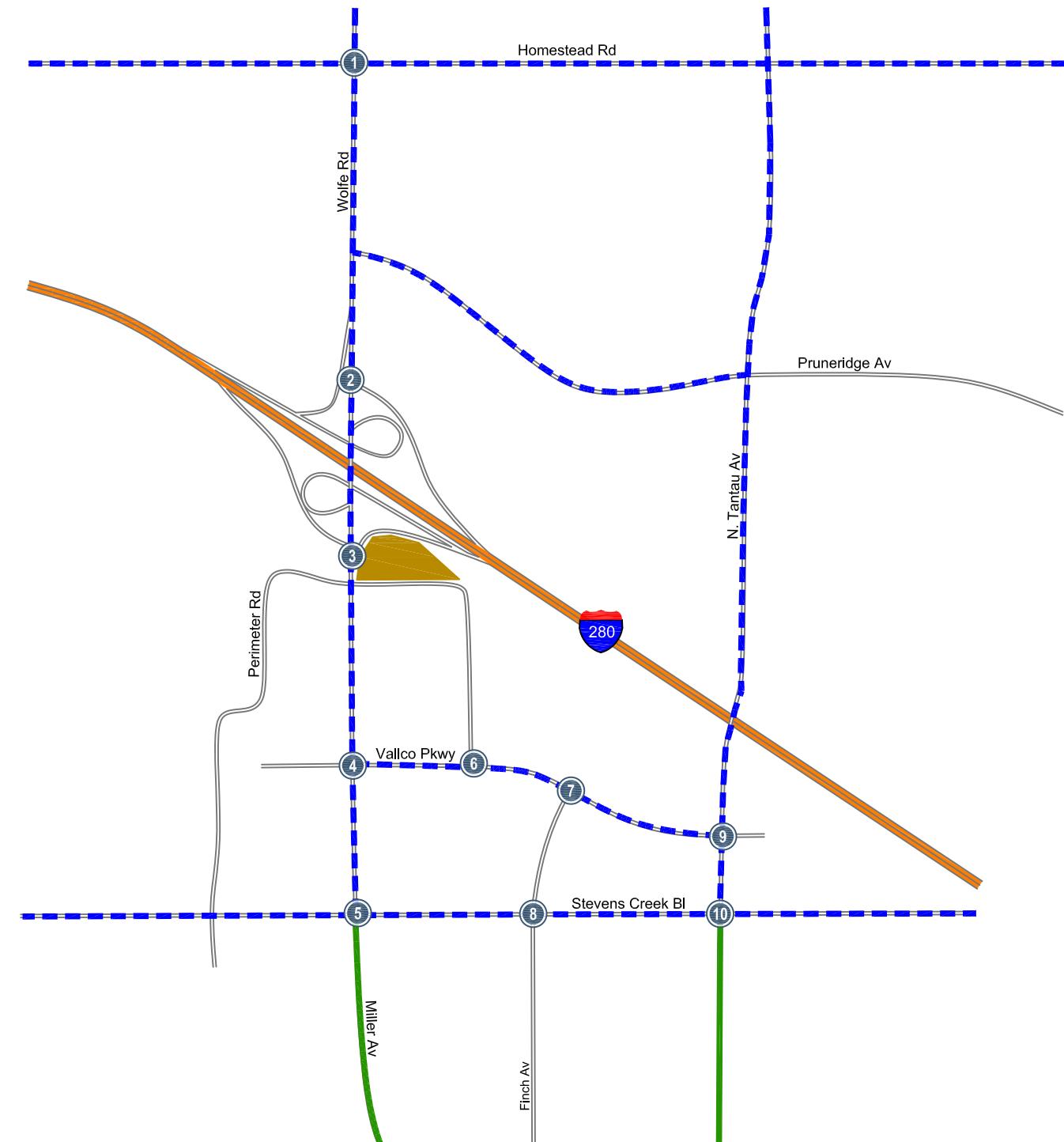
Route 26 is a local bus route that provides service between East San Jose and Sunnyvale Lockheed Martin LRT Station via Wolfe Road in the vicinity of the project site. Weekday hours of operation are from 5:20 am to 11:50 pm with a 30-minute headway. On weekends, this route operates on 30 to 60 minute headways between 6:30 am and 10:50 pm.

Route 81 is a local bus route that provides service between San Jose State University and Vallco Shopping Mall via Wolfe Road in the vicinity of the project site. Weekday hours of operation are from 6:25 am to 8:50 pm with 30 to 60 minute headways. On weekends, this route operates on a 60-minute headway between 9:30 am and 4:50 pm from the Santa Clara Transit Center to Vallco Shopping Mall.

Route 101 is an express bus route between the Park-n-Ride lot at Camden Avenue/State Route 85 and Palo Alto. This route operates twice in the northbound direction between 6:50 am to 7:50 am and twice in the southbound direction between 4:50 pm and 6:00 pm on Wolfe Road and I-280 near the project site. This route does not operate on weekends.

Route 182 is an express bus route between the IBM at Bailey Avenue and Palo Alto. This route operates once in the northbound direction at 5:50 am and once in the southbound direction at 7:50 pm. This route does not operate on weekends. Route 182 operates on Wolfe Road and I-280 near the project site.

The closest bus stops for routes 26, 81, and 101 are located on Wolfe Road at Vallco Mall, approximately 850 feet from the project site. The bus stops for route 182 are located on Vallco Parkway, approximately ¼ mile from the project site. The closest stops for route 23 are on Stevens Creek Boulevard, about one-half mile from the site. However, no sidewalks exist between the project site and any of the bus stops.



LEGEND

= Project Site Location

= Study Intersection

= On-Street Bike Lanes (Class II)

= On-Street Bike Routes (Class III)

Figure 3
Existing Bicycle Facilities

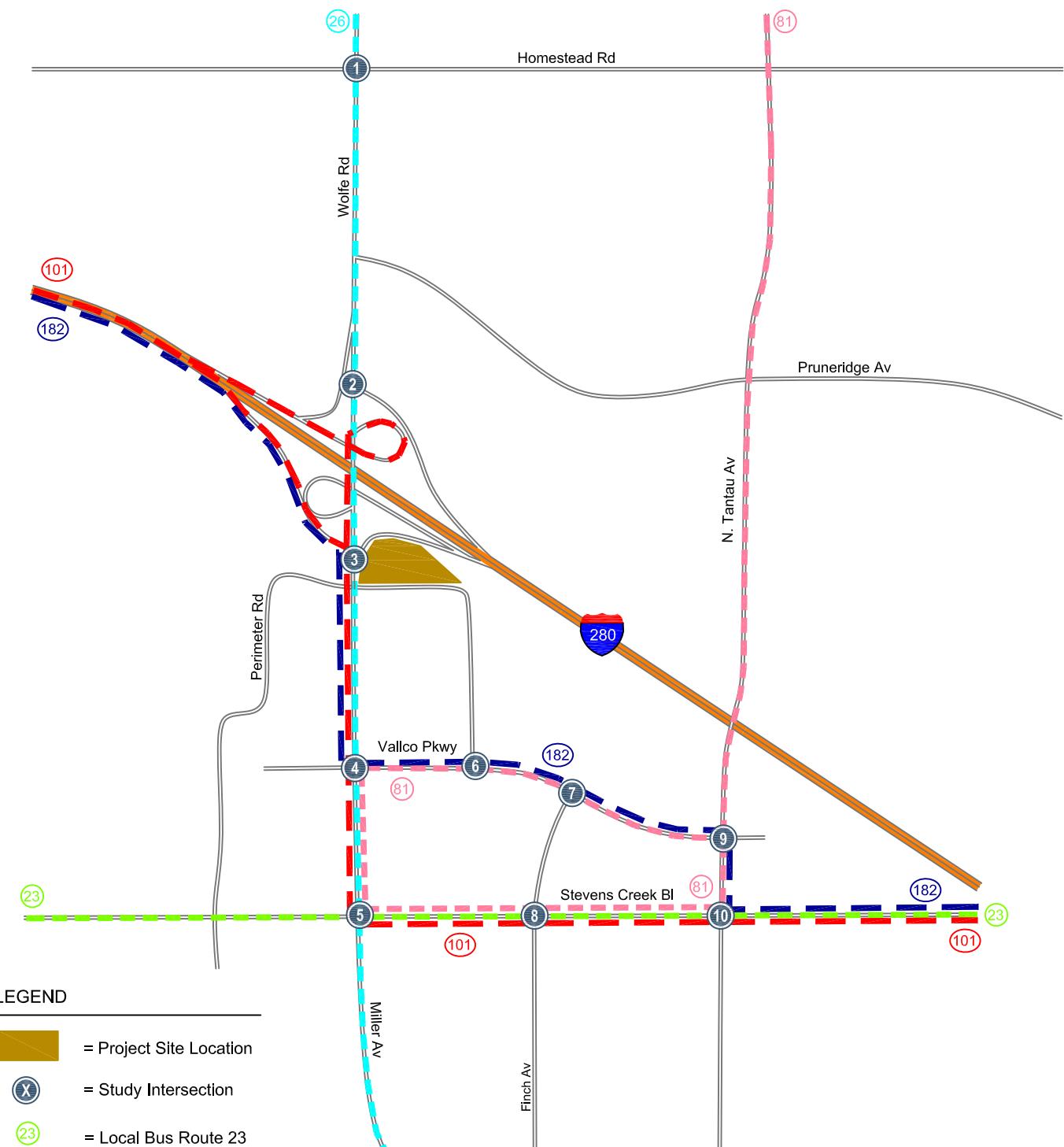


Figure 4
Existing Transit Facilities

Existing Intersection Lane Configurations

The existing lane configurations at the study intersections were confirmed by observations in the field and are shown on Figure 5.

At the Wolfe Road/Vallco Parkway intersection, since the release date of the Apple Campus 2 EIR, the City of Cupertino has modified the westbound through lane to a shared through/right-turn lane for a total of two left-turn lanes, one shared through/right-turn lane, and one exclusive right-turn lane. Therefore, the lane configuration at the intersection of Wolfe Road/Vallco Parkway has been modified from the Apple Campus 2 EIR.

Existing Traffic Volumes

Existing traffic volumes were obtained from the City of Cupertino (consistent with the Apple Campus 2 EIR) for all study intersections except for the intersections of Vallco Parkway/Perimeter Road and Vallco Parkway/Finch Avenue. New counts were conducted at these two intersections in May 2014. For the three CMP-designated intersections, PM peak hour counts were obtained from the CMP traffic count database.

Existing peak-hour traffic volumes are shown on Figure 6. New count data are included in Appendix A.

Existing Intersection Levels of Service

Intersection levels of service were evaluated against City of Cupertino and CMP standards. The results of the intersection level of service analysis under existing conditions are summarized in Table 5.

City of Cupertino Intersection Analysis

The results of the level of service analysis show that all study intersections currently operate at an acceptable level of service (LOS D or better) during both the AM and PM peak hours of traffic.

CMP Intersection Analysis

The results of the level of service analysis show that all of the study CMP intersections currently operate at an acceptable level of service (LOS E or better) during the AM and PM peak hours of traffic.

The intersection level of service calculation sheets are included in Appendix B.

Hyatt House Hotel, Cupertino

1 Homestead Rd	2	3 I-280 SB Off-Ramp	4 Valco Pkwy
Wolfe Rd		I-280 NB Off-Ramp	Wolfe Rd
	Wolfe Rd		I-280 SB On-Ramp
Stevens Creek BI	6	7 Valco Driveway	8 Stevens Creek BI
Miller Av	Valco Pkwy	Valco Pkwy	Finch Av
	Perimeter Rd		Finch Av



LEGEND

- [Yellow Polygon] = Project Site Location
- [Blue Circle with X] = Study Intersection
- [Left Arrow] = Lane Configuration under Existing Conditions
- [Pink Arrow] = Lane Configuration under Background Conditions
- [Green Arrow] = Lane Configuration under Cumulative Conditions

9 Valco Pkwy	10 Stevens Creek BI
Tantau Av	Tantau Av

Figure 5
Lane Configurations

Hyatt House Hotel, Cupertino

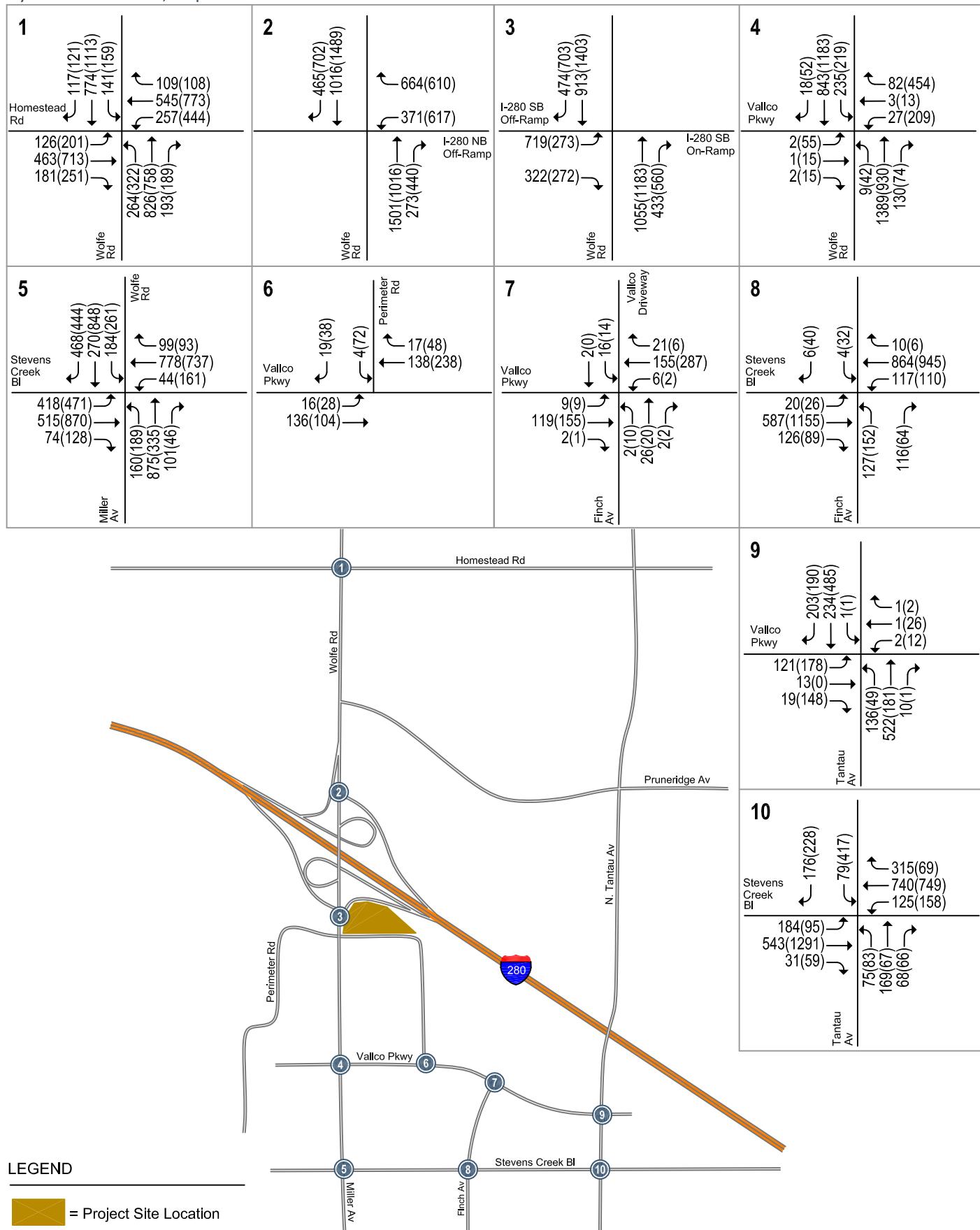


Figure 6
Existing Traffic Volumes

Table 5
Existing Intersection Levels of Service

Study Number	Intersection	Peak Hour	Count Date	Avg. Delay (sec)	LOS
1	Wolfe Rd & Homestead Rd	AM	05/05/11	34.0	C
		PM	05/05/11	39.2	D
2	Wolfe Rd & I-280 NB Ramp*	AM	05/05/11	12.8	B
		PM	05/05/11	13.4	B
3	Wolfe Rd & I-280 SB Ramp*	AM	05/05/11	10.6	B
		PM	05/05/11	6.5	A
4	Wolfe Rd & Vallco Pkwy	AM	05/04/11	18.1	B
		PM	05/04/11	29.8	C
5	Wolfe Rd & Stevens Creek Blvd*	AM	05/05/11	37.4	D
		PM	05/05/11	38.3	D
6	Perimeter Rd & Vallco Pkwy	AM	05/15/14	6.1	A
		PM	05/15/14	11.1	B
7	Finch Ave & Vallco Pkwy ¹²	AM	05/13/14	9.4	B
		PM	05/13/14	1.3	B
8	Finch Ave & Stevens Creek Blvd	AM	05/12/11	23.5	C
		PM	05/12/11	23.5	C
9	N Tantau Ave & Vallco Pkwy	AM	06/01/11	24.1	C
		PM	06/01/11	27.8	C
10	N Tantau Ave & Stevens Creek Blvd	AM	05/12/11	38.9	D
		PM	05/12/11	38.8	D

Notes:

* denotes a CMP intersection

¹ This intersection was analyzed as an unsignalized intersection under existing conditions and as a signalized intersection under background and cumulative conditions.

² LOS for unsignalized intersection represents worst movement LOS

3.

Existing Plus Project Conditions

This chapter describes existing plus project traffic conditions, including the method by which project traffic is estimated. Existing plus project traffic conditions could potentially occur if the project were to be occupied prior to the other approved projects in the area. It is unlikely that this traffic condition would occur, since other approved projects expected to add traffic to the study area would likely be built and occupied during the time the project is going through the development review process.

Transportation Network Under Existing Plus Project Conditions

It is assumed in this analysis that the transportation network under existing plus project conditions would be the same as the existing transportation network.

Project Trip Estimates

The magnitude of traffic produced by a new development and the locations where that traffic would appear are estimated using a three-step process: (1) trip generation, (2) trip distribution, and (3) trip assignment. In determining project trip generation, the magnitude of traffic entering and exiting the site is estimated for the AM and PM peak hours. As part of the project trip distribution, an estimate is made of the directions to and from which the project trips would travel. In the project trip assignment, the project trips are assigned to specific streets. These procedures are described further in the following sections.

Trip Generation

Through empirical research, data have been collected that quantify the amount of traffic produced by common land uses. Thus, for the most common land uses there are standard trip generation rates that can be applied to help predict the future traffic increases that would result from a new development. The magnitude of traffic added to the roadway system by a particular development is estimated by multiplying the applicable trip generation rates by the size of the development.

The Institute of Transportation Engineers (ITE) *Trip Generation Manual, 9th Edition* publishes trip generation rates based on numerous counts of existing development of the same land use type. The rates published by ITE for Hotel (ITE land use code 310) were used for this project. The Hotel category includes trips generated by typical hotel amenities such as restaurants, bars, and meetings places. No trip credits were taken because the existing site is currently being used as a parking lot. Based on the ITE trip generation rates, the proposed project would generate a total of 1,209 daily trips, with 78 trips occurring during the AM peak hour and 89 trips occurring during the PM peak hour period. Table 6 shows the project trip generation estimates.

No reduction was assumed in trip generation estimates attributed to the shuttle services such as airport shuttles and limousine services because the ITE rates are based on surveys conducted at hotels with similar services.

Trip Distribution Pattern

The trip distribution pattern for the project was developed based on existing travel patterns on the surrounding roadway system and the locations of complementary land uses. The traffic volumes on the nearby street and freeway network were examined and the distribution percentages were calculated in proportion to those volumes. Figure 7 shows the project trip distribution pattern.

Trip Assignment

The peak hour vehicle trips generated by the project were assigned to the roadway network in accordance with the trip distribution pattern. Figure 8 shows the project trip assignment.

Table 6
Project Trip Generation Estimates

Land Use	Size	Daily Rate ¹	Daily Trips	AM Peak Hour				PM Peak Hour				Total Trips
				Peak-Hour Rate ¹	In	Out	Total Trips	Peak-Hour Rate ¹	In	Out	Total Trips	
<u>Proposed</u>												
Hotel ¹	148 rooms	8.17	1,209	0.53	46	32	78	0.60	45	43	89	
Notes:												
1. ITE trip rates for Hotel (Land Use #310) used.												
Source: Institute of Transportation Engineers, <i>Trip Generation</i> , 9th Edition.												

Existing Plus Project Traffic Volumes

The project trips were added to existing traffic volumes to obtain existing plus project traffic volumes (see Figure 9).

Intersection Levels of Service Under Existing Plus Project Conditions

Intersection levels of service were evaluated against City of Cupertino and CMP standards. The results of the intersection level of service analysis under existing plus project conditions are summarized in Table 7.

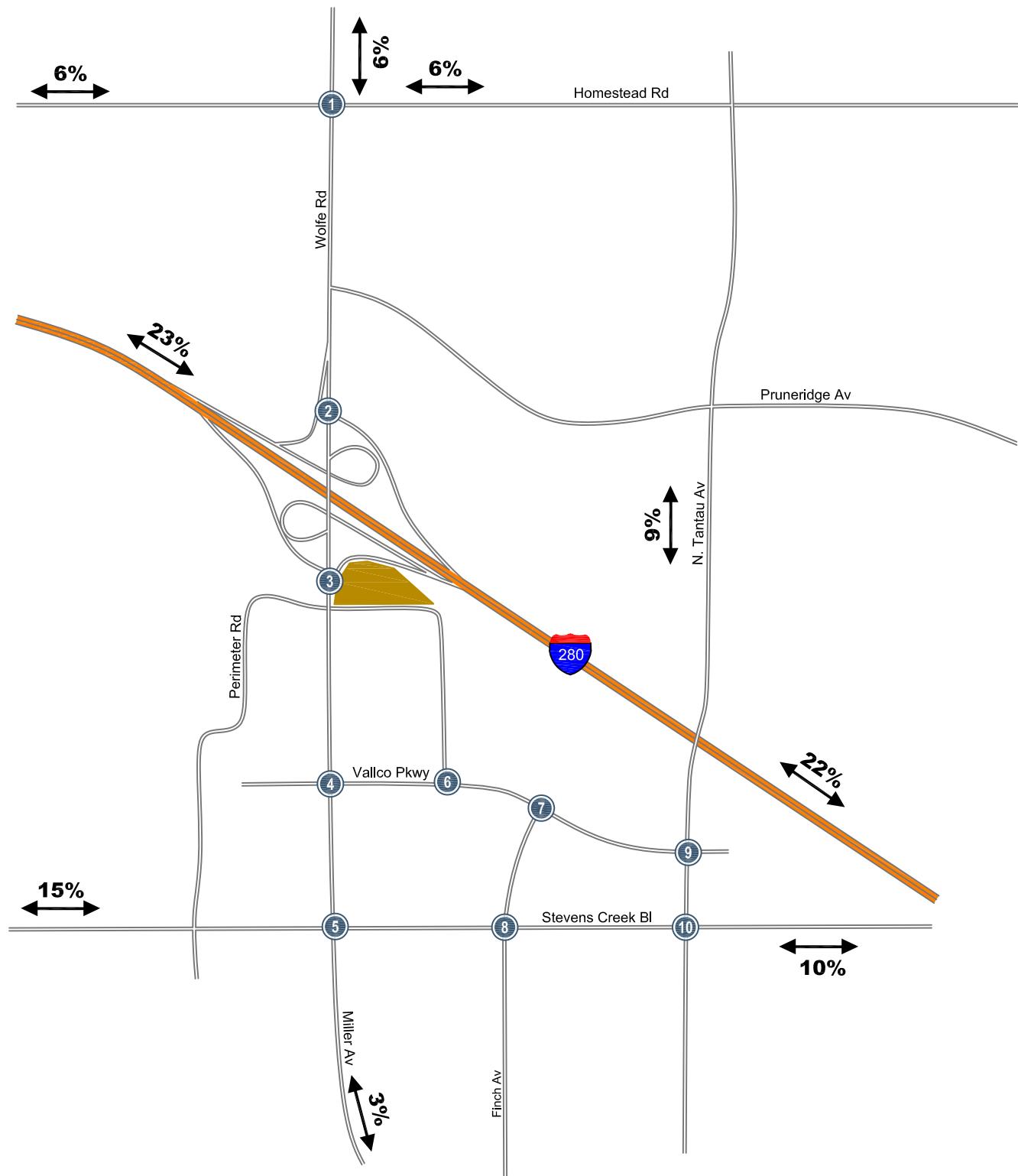
City of Cupertino Intersection Analysis

The results of the level of service analysis under existing plus project conditions show that all study intersections would operate at an acceptable level of service (LOS D or better) during both the AM and PM peak hours of traffic.

CMP Intersection Analysis

The results of the level of service analysis under existing plus project conditions show that all of the study CMP intersections would operate at an acceptable level of service (LOS E or better) during the AM and PM peak hours of traffic.

The intersection level of service calculation sheets are included in Appendix B.



LEGEND

= Project Site Location

= Study Intersection

Figure 7
Project Trip Distribution

Hyatt House Hotel, Cupertino

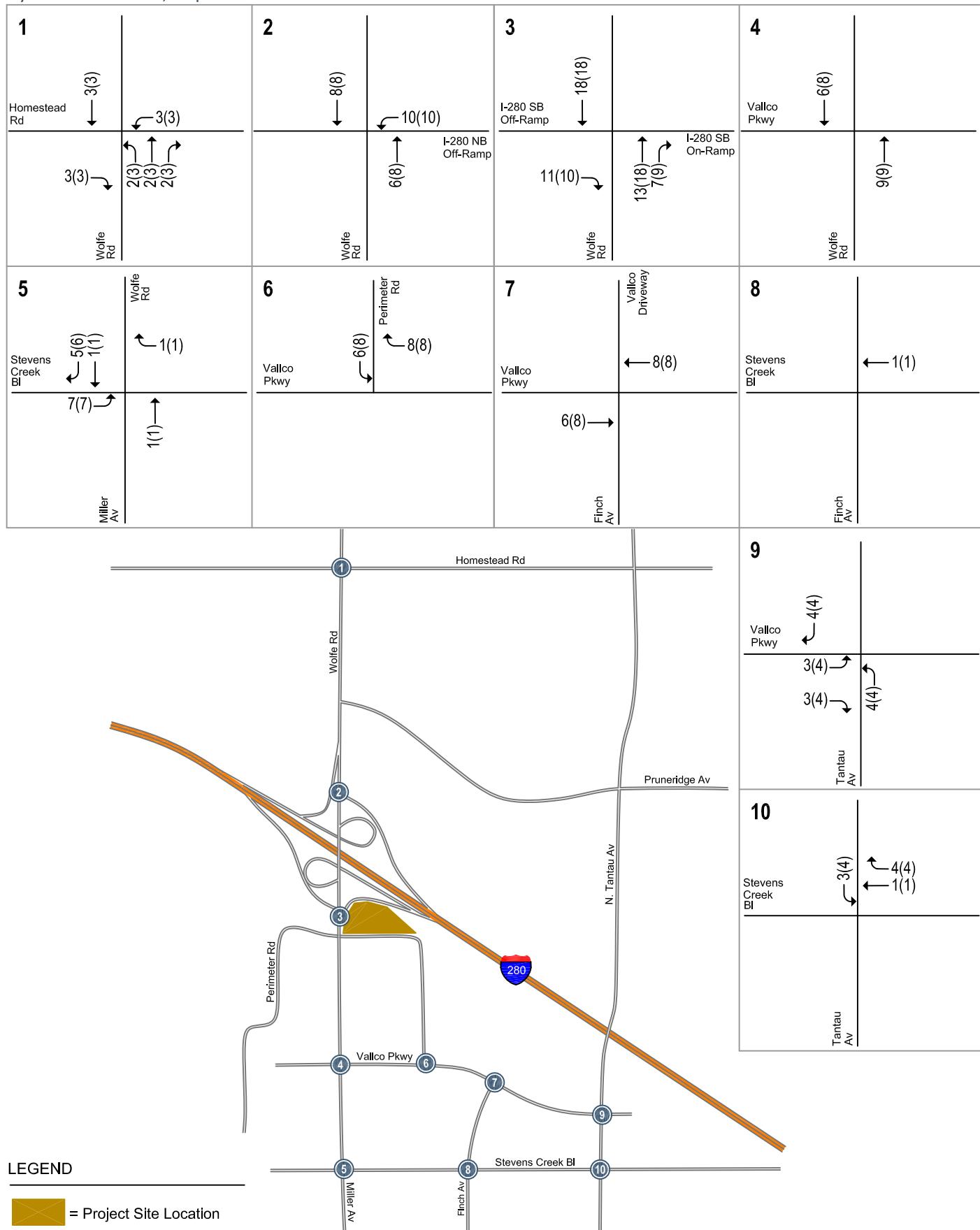


Figure 8
Project Trip Assignment

Hyatt House Hotel, Cupertino

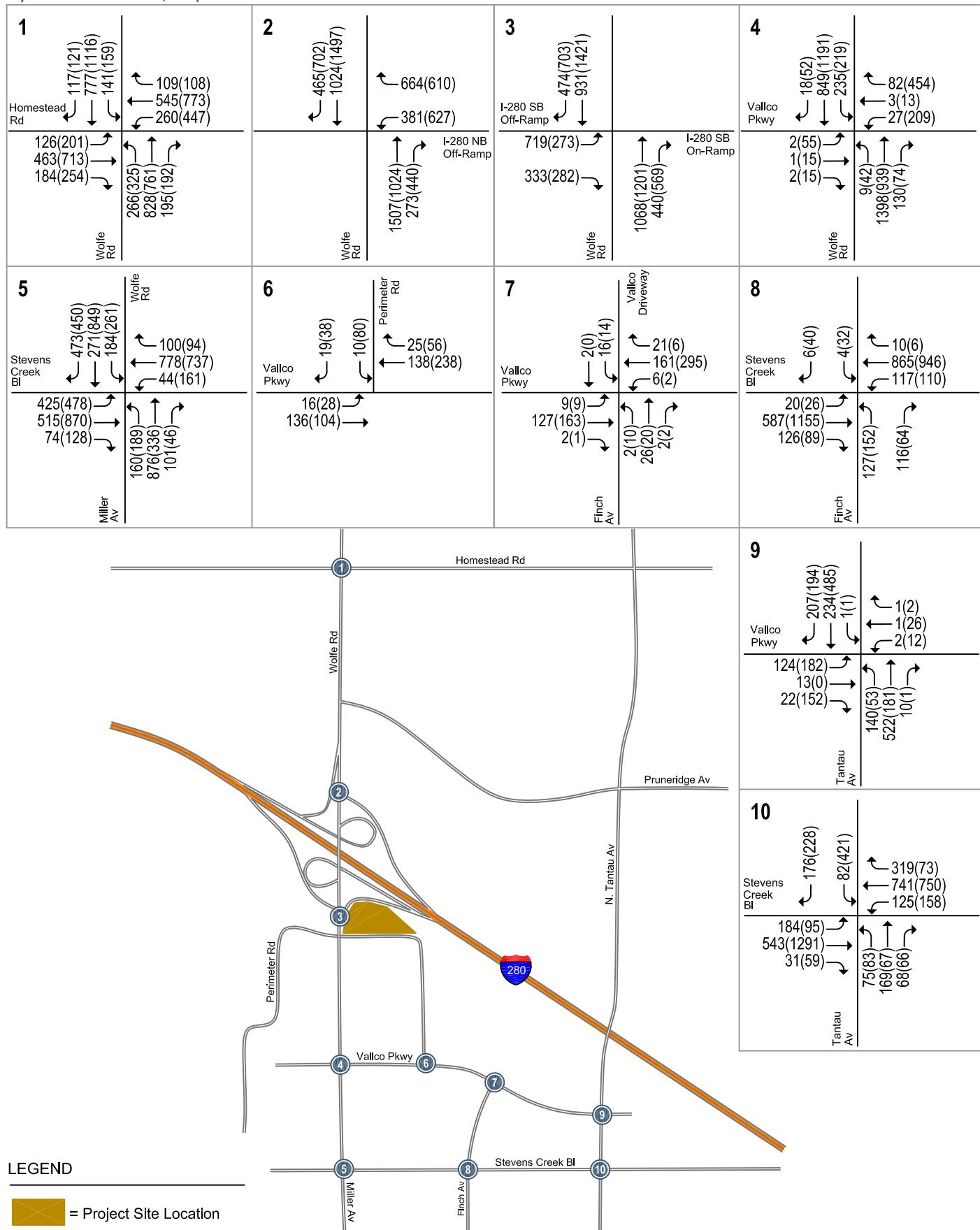


Figure 9
Existing Plus Project Traffic Volumes

Table 7
Existing Plus Project Intersection Levels of Service

Study Number	Intersection	Peak Hour	Count Date	Existing		Existing + Project	
				Avg. Delay (sec)	LOS	Avg. Delay (sec)	LOS
1	Wolfe Rd & Homestead Rd	AM	05/05/11	34.0	C	34.0	C
		PM	05/05/11	39.2	D	39.1	D
2	Wolfe Rd & I-280 NB Ramp*	AM	05/05/11	12.8	B	12.7	B
		PM	05/05/11	13.4	B	13.4	B
3	Wolfe Rd & I-280 SB Ramp*	AM	05/05/11	10.6	B	10.6	B
		PM	05/05/11	6.5	A	6.5	A
4	Wolfe Rd & Vallco Pkwy	AM	05/04/11	18.1	B	18.1	B
		PM	05/04/11	29.8	C	29.9	C
5	Wolfe Rd & Stevens Creek Blvd*	AM	05/05/11	37.4	D	37.4	D
		PM	05/05/11	38.3	D	38.3	D
6	Perimeter Rd & Vallco Pkwy	AM	05/15/14	6.1	A	7.0	A
		PM	05/15/14	11.1	B	11.5	B
7	Finch Ave & Vallco Pkwy ¹²	AM	05/13/14	9.4	B	9.5	B
		PM	05/13/14	1.3	B	1.3	B
8	Finch Ave & Stevens Creek Blvd	AM	05/12/11	23.5	C	23.6	C
		PM	05/12/11	23.5	C	23.6	C
9	N Tantau Ave & Vallco Pkwy	AM	06/01/11	24.1	C	24.5	C
		PM	06/01/11	27.8	C	28.2	C
10	N Tantau Ave & Stevens Creek Blvd	AM	05/12/11	38.9	D	38.7	D
		PM	05/12/11	38.8	D	38.6	D

Notes:

* denotes a CMP intersection

¹ This intersection was analyzed as an unsignalized intersection under existing conditions and as a signalized intersection under background and cumulative conditions.

² LOS for unsignalized intersection represents worst movement LOS

4. **Background Conditions**

This chapter presents background traffic conditions, which are defined as conditions just prior to completion of the proposed project. Traffic volumes for background conditions comprise volumes from existing traffic counts plus traffic generated by other approved developments in the vicinity of the site. This chapter describes the procedure used to determine background traffic volumes and the resulting traffic conditions. The background scenario predicts a realistic traffic condition that would occur as approved development gets built and occupied.

Background Traffic Volumes

Background peak hour traffic volumes were estimated by adding to existing peak hour volumes the estimated traffic from approved but not yet constructed developments. The added traffic from approved but not yet constructed developments was obtained from the Apple Campus 2 EIR. All approved projects assumed in the Apple Campus 2 EIR, including the Apple Campus, were included under background conditions. Background traffic volumes are shown graphically on Figure 10.

Background Transportation Network

The following transportation improvements were assumed under background conditions.

1. Valco Parkway/Finch Road intersection – Currently Finch Road is closed to traffic and being reconstructed as a private street providing access to the Main Street development. This intersection will be signalized with the completion of the Main Street development. The Main Street development will also construct a new signalized intersection on Valco Parkway between Finch Road and Tantau Avenue that will provide access to the parking garage within the Main Street development.
2. Wolfe Road/I-280 NB Ramps intersection – The Apple Campus 2 project will add a third through lane to northbound Wolfe Road from just after the northbound I-280 loop on-ramp to Pruneridge Avenue (increase from two northbound through lanes) and widen the northbound off-ramp to four lanes (two westbound left-turn lanes and two westbound right-turn lanes).
3. Valco Parkway/Tantau Avenue – The Apple Campus 2 project will construct an exclusive northbound through lane for a total of one left-turn lane, one through lane, and one shared through/right-turn lane. An additional receiving lane on the north side of the intersection would also be constructed. The exclusive eastbound through lane will be converted to a shared through/left-turn lane. These improvements will require the provision of two northbound through receiving lanes on Tantau Avenue. A southbound right-turn pocket (approximately 250 feet in length) will be constructed, and the existing shared southbound through/right-turn lane will be converted to an exclusive southbound through lane.

4. Stevens Creek Boulevard/Tantau Avenue intersection – The Apple Campus 2 project will add a southbound right-turn pocket (approximately 100 feet in length) and convert the existing shared southbound left-turn/right-turn lane to an exclusive southbound left-turn lane.

Intersection Levels of Service Under Background Conditions

Intersection levels of service were evaluated against City of Cupertino and CMP standards. The results of the intersection level of service analysis under background conditions are summarized in Table 8.

City of Cupertino Intersection Analysis

The results of the level of service analysis under background conditions show that all of the study intersections would operate at an acceptable level of service (LOS D or better) during both the AM and PM peak hours of traffic.

CMP Intersection Analysis

The results of the level of service analysis under background conditions show that all of the study CMP intersections would operate at an acceptable level of service (LOS E or better) during the AM and PM peak hours of traffic.

The intersection level of service calculation sheets are included in Appendix B.

Hyatt House Hotel, Cupertino

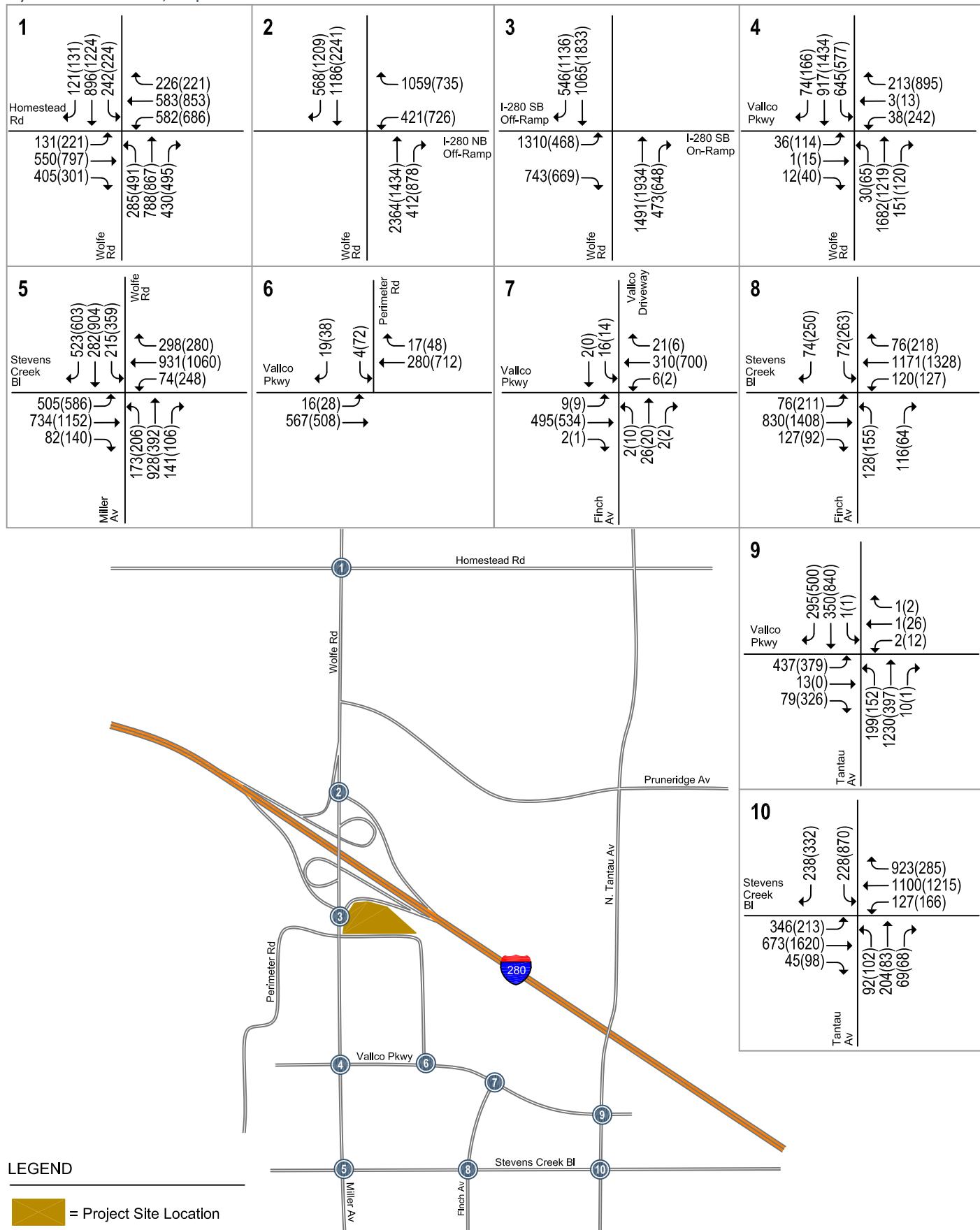


Figure 10
Background Traffic Volumes

Table 8
Background Intersection Levels of Service

Study Number	Intersection	Peak Hour	Existing		Background	
			Avg. Delay (sec)	LOS	Avg. Delay (sec)	LOS
1	Wolfe Rd & Homestead Rd	AM	34.0	C	40.9	D
		PM	39.2	D	50.5	D
2	Wolfe Rd & I-280 NB Ramp*	AM	12.8	B	18.0	B
		PM	13.4	B	26.8	C
3	Wolfe Rd & I-280 SB Ramp*	AM	10.6	B	17.9	B
		PM	6.5	A	11.2	B
4	Wolfe Rd & Vallco Pkwy	AM	18.1	B	27.3	C
		PM	29.8	C	38.8	D
5	Wolfe Rd & Stevens Creek Blvd*	AM	37.4	D	39.5	D
		PM	38.3	D	43.8	D
6	Perimeter Rd & Vallco Pkwy	AM	6.1	A	4.1	A
		PM	11.1	B	6.8	A
7	Finch Ave & Vallco Pkwy ¹	AM	9.4	B	25.7	C
		PM	1.3	B	37.9	D
8	Finch Ave & Stevens Creek Blvd	AM	23.5	C	23.8	C
		PM	23.5	C	31.9	C
9	N Tantau Ave & Vallco Pkwy	AM	24.1	C	28.4	C
		PM	27.8	C	34.0	C
10	N Tantau Ave & Stevens Creek Blvd	AM	38.9	D	43.2	D
		PM	38.8	D	46.8	D

Notes:

* denotes a CMP intersection

¹ This intersection was analyzed as an unsignalized intersection under existing conditions and as a signalized intersection under background and cumulative conditions.

5.

Background Plus Project Conditions

This chapter describes near-term traffic conditions that most likely would occur when the project is complete. It includes a description of the significance criteria used to establish what constitutes a project impact, the method by which project traffic is estimated, and any impacts caused by the project. Background plus project conditions were evaluated relative to background conditions in order to determine potential project impacts. This traffic scenario represents a more congested traffic condition than the existing plus project scenario, since it includes traffic generated by approved but not yet built projects in the area.

Significant Impact Criteria

Significance criteria are used to establish what constitutes an impact. For this analysis, the criteria used to determine significant impacts on signalized intersections are based on City of Cupertino Level of Service standards.

City of Cupertino Definition of Significant Intersection Impacts

The project is said to create a significant adverse impact on traffic conditions at a signalized intersection in the City of Cupertino if for either peak hour:

1. The level of service at the intersection degrades from an acceptable LOS D or better under background conditions to an unacceptable LOS E or F under background plus project conditions, or
2. The level of service at the intersection is an unacceptable LOS E or F under background conditions and the addition of project trips causes both the critical-movement delay at the intersection to increase by four (4) or more seconds *and* the volume-to-capacity ratio (V/C) to increase by one percent (.01) or more.

An exception to rule #2 above applies when the addition of project traffic reduces the amount of average delay for critical movements (i.e., the change in average stopped delay for critical movements is negative). In this case, the threshold of significance is an increase in the critical V/C value by .01 or more.

A significant impact by City of Cupertino standards is said to be satisfactorily mitigated when measures are implemented that would restore intersection level of service to background conditions or better.

CMP Definition of Significant Intersection Impacts

The definition of a significant impact at a CMP intersection is the same as for the City of Cupertino, except that the CMP standard for acceptable level of service at a CMP intersection is LOS E or better. Thus, a CMP intersection that operates at LOS F would fail to meet the CMP level of service standard.

Transportation Network Under Background Plus Project Conditions

It is assumed in this analysis that the transportation network under background plus project conditions would be the same as the background transportation network.

Project Trip Estimates

As previously described in Chapter 3 (see Table 6), the proposed project would generate 1,209 new daily vehicle trips, with 79 new trips occurring during the AM peak hour and 89 new trips occurring during the PM peak hour.

Background Plus Project Traffic Volumes

The peak hour trips generated by the project were added to background traffic volumes to obtain background plus project traffic volumes (see Figure 11). The project trips were assigned to the roadway system in accordance with the trip distribution patterns discussed in Chapter 3.

Intersection LOS Under Background Plus Project Conditions

Intersection levels of service were evaluated against City of Cupertino and CMP standards. The results of the intersection level of service analysis under background plus project conditions are summarized in Table 9.

City of Cupertino Intersection Analysis

The results of the level of service analysis under background plus project conditions show that all of the study intersections would operate at an acceptable level of service (LOS D or better) during both the AM and PM peak hours of traffic.

CMP Intersection Analysis

The results of the level of service analysis under background plus project conditions show that all of the study CMP intersections would operate at an acceptable level of service (LOS E or better) during the AM and PM peak hours of traffic.

The intersection level of service calculation sheets are included in Appendix B.

Hyatt House Hotel, Cupertino

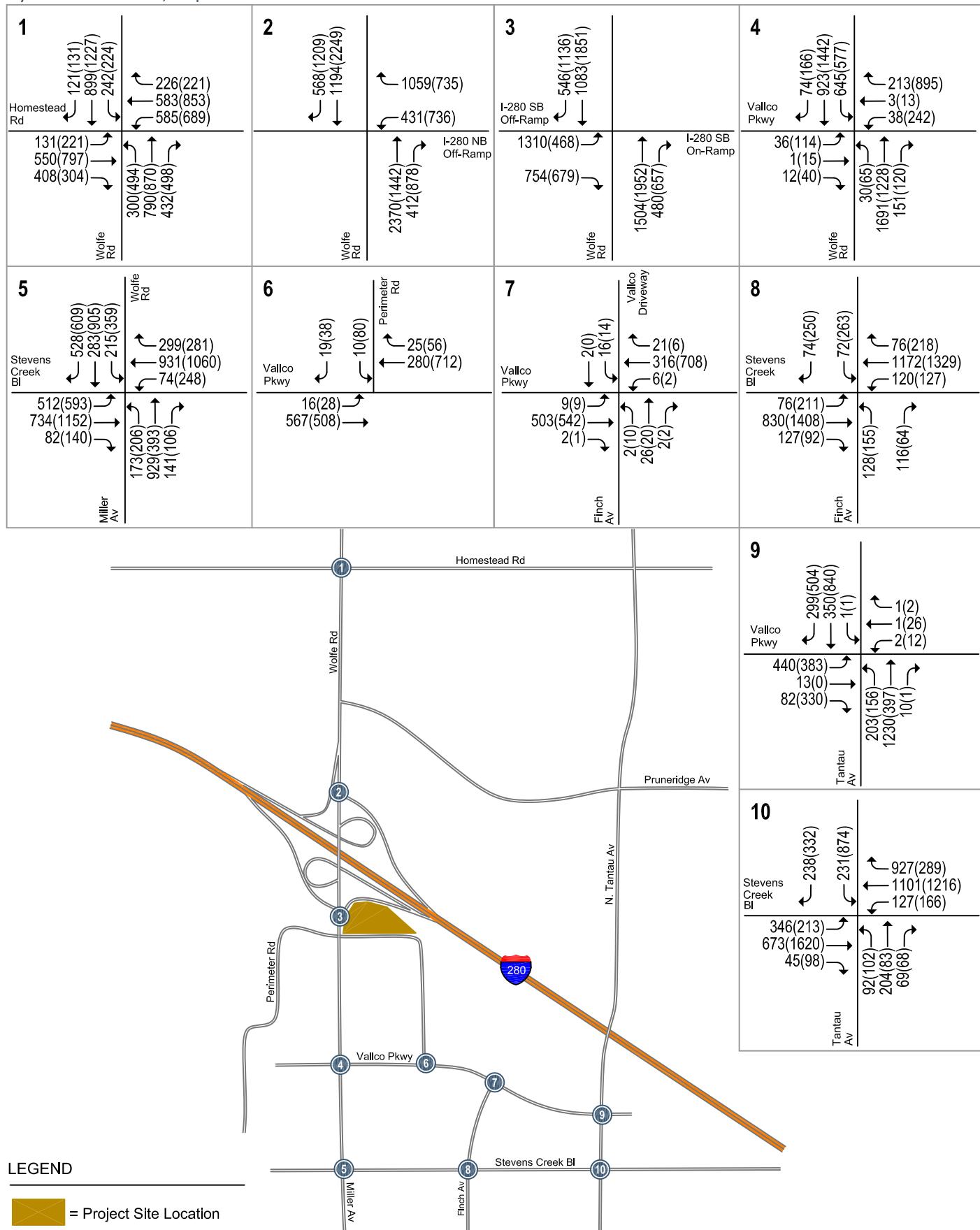


Figure 11
Background Plus Project Traffic Volumes

Table 9
Background Plus Project Intersection Levels of Service

Study Number	Intersection	Peak Hour	Background		Background Plus Project		
			Avg. Delay (sec)	LOS	Avg. Delay (sec)	LOS	Incr. In Crit. Delay (sec)
1	Wolfe Rd & Homestead Rd	AM	40.9	D	41.2	D	0.4 0.004
		PM	50.5	D	50.8	D	0.4 0.003
2	Wolfe Rd & I-280 NB Ramp*	AM	18.0	B	18.0	B	0.1 0.001
		PM	26.8	C	27.6	C	0.7 0.003
3	Wolfe Rd & I-280 SB Ramp*	AM	17.9	B	18.1	B	0.4 0.004
		PM	11.2	B	11.4	B	0.3 0.009
4	Wolfe Rd & Vallco Pkwy	AM	27.3	C	27.3	C	0.0 0.002
		PM	38.8	D	38.9	D	0.1 0.002
5	Wolfe Rd & Stevens Creek Blvd*	AM	39.5	D	39.6	D	0.1 0.003
		PM	43.8	D	44.0	D	0.3 0.003
6	Perimeter Rd & Vallco Pkwy	AM	4.1	A	4.3	A	0.2 0.007
		PM	6.8	A	7.1	A	0.3 0.008
7	Finch Ave & Vallco Pkwy ¹	AM	25.7	C	25.9	C	0.2 0.008
		PM	37.9	D	39.0	D	1.1 0.009
8	Finch Ave & Stevens Creek Blvd	AM	23.8	C	23.8	C	0.0 0.000
		PM	31.9	C	31.9	C	0.0 0.000
9	N Tantau Ave & Vallco Pkwy	AM	28.4	C	28.5	C	0.1 0.001
		PM	34.0	C	34.3	C	0.4 0.008
10	N Tantau Ave & Stevens Creek Blvd	AM	43.2	D	43.2	D	0.0 0.000
		PM	46.8	D	47.0	D	0.2 0.001

Notes:
* denotes CMP intersection
¹ This intersection was analyzed as an unsignalized intersection under existing conditions and as a signalized intersection under background and cumulative conditions.
Bold indicates a significant impact.

6. **Other Transportation Issues**

This chapter presents an analysis of other transportation issues associated with the project, including:

- Site access and circulation
- Sight distance at project driveways
- Potential project impacts to transit, bicycle, and pedestrian facilities, including the provision of a new pathway on the west side of the project site
- Parking

Unlike the level of service impact methodology, which is adopted by the City Council, the analyses in this chapter are based on professional judgment in accordance with the standards and methods employed by the traffic engineering community.

Vehicular Access and On-Site Circulation

A site circulation and access review was conducted to determine the adequacy of the proposed site plan in accordance with generally accepted traffic engineering standards. The site plan prepared by Gene Fong Associates (July 30, 2014) was used for this purpose. The site plan is shown on Figure 2.

Vehicular access to the project site would be provided via two entry points along Perimeter Road, along the southern boundary of the project site. Both the entrances would be full access driveways. The western driveway on Perimeter Road would provide direct access to the main entrance and drop-off area that would be located on the south side of the hotel, facing Perimeter Road. The intersection of this driveway and Perimeter Road would be controlled by an all-way stop sign. The second driveway to the east would provide access to the parking aisle that circulates around the hotel building providing access to 73 surface parking stalls and 83 underground parking spaces. This driveway would have a stop control for the outbound approach. The underground parking garage would be accessible by the hotel's two elevator shafts and one of the two staircases.

Sight Distance at Project Driveway

The project driveways should be free and clear of any obstructions to optimize sight distance, thereby ensuring that exiting vehicles can see pedestrians on the sidewalk and vehicles traveling on Perimeter Road. Landscaping at the project driveways should not conflict with a driver's ability to locate a gap in traffic. Adequate sight distance (sight distance triangles) should be provided at all project driveways in accordance with Caltrans standards. Sight distance triangles should be measured approximately 10 feet back from the traveled way. The sight distance for outbound vehicles exiting the first driveway is limited due to the downward slope on Perimeter Road, west of the project site as Perimeter Road runs beneath Wolfe Road. However, since the intersection of this driveway at Perimeter Road will be controlled by an all-way stop sign, vehicles will be able to exit safely onto Perimeter Road. The second driveway to the east will have adequate sight distance for exiting vehicles to safely locate a gap on Perimeter Road. Also, because of low traffic volumes on Perimeter Road, vehicles exiting the hotel will be able to easily find gaps in traffic on Perimeter Road.

Transit Services

The closest bus stop is located on Wolfe Road at Vallco Mall about 850 feet walking distance from the site. This bus stop serves Routes 26, 81, 101. The proposed hotel is not expected to generate many new transit riders. If the hotel generates any transit riders, these new riders could be easily accommodated by the current available ridership capacity of the existing bus service in the study area.

Shuttle Services

The project would include the use of an airport shuttle service for hotel guests to provide access to the San Jose International Airport, running approximately 6 times per day. In addition, the project would provide a local shuttle for hotel guests (limited to within a 5-mile radius of the Project site) that would run approximately 2 times per day.

Pedestrian Facilities

Pedestrian facilities typically consist of sidewalks and crosswalks, including crosswalks with pedestrian signal heads at signalized intersections. Sidewalks are provided on both sides of Homestead Road, Wolfe Road, Stevens Creek Boulevard and Vallco Parkway. However, there are currently no sidewalks on Perimeter Road or on the connecting ramps to Wolfe Road. Thus, there are no good existing pedestrian connections from the site to Wolfe Road, Vallco Parkway, or Stevens Creek Boulevard.

As shown on the site plan dated 7/30/2014, the hotel project will provide a sidewalk on the north side of Perimeter Road and two crosswalks across Perimeter Road, connecting the site to the rest of the Vallco Shopping District. One of the crosswalks will be located at the main entrance driveway, where there will be 4-way stop control. The other crosswalk will be west of the second driveway that leads to parking spaces. Also, the 7/30/2014 site plan shows a pedestrian connections from the hotel to the new sidewalk on Perimeter Road near where the crosswalks will be placed..

Bicycle Facilities

The site is well-served by existing and planned bicycle facilities. The one street in the vicinity without bicycle lanes is Perimeter Road, and it has low traffic volume and speed so it is conducive to bicycle travel. The project will provide a new pathway along the west side of the project site. This path will connect to a future trail along the south side of I-280. Although the trail on the south side of I-280 has not yet been fully studied, the pathway connection provided by the project will significantly enhance bicycle and pedestrian connectivity if it is ever implemented.

In accordance with Cupertino requirements, the project needs to provide bicycle parking equivalent to 5% of the vehicle parking. Based on 156 spaces, 8 bicycle spaces will be required. The current site plan shows that 10 bicycle spaces will be provided in front of the drop-off area along Perimeter Road, which exceeds the City's requirement.

Parking

City Parking Codes

The City of Cupertino Parking Code requirement is 1.0 space per room and 1.0 space per employee. The proposed hotel would have 148 rooms and 9 employees per shift. This would require 157 parking spaces by code. The proposed project also includes a full service restaurant (43 seats, including the seats on the outdoor dining terrace), a bar (9 seats) and 2480 square feet of meeting rooms. The City of Cupertino's parking requirement for restaurants is one stall for every four seats, plus one for each employee on shift. The requirement for a bar area is one stall for every three seats, plus one for each employee on a shift. Since the proposed dining and bar areas would have 3 employees per shift, the City parking code would require a total of 17 additional parking spaces for the restaurant and bar. For meeting rooms, the requirement is one space per 250 square feet, which would be an additional 10 spaces. Therefore, 184 parking spaces would be required based on the City parking code. The project proposes to provide 156 parking spaces.

In order to determine whether the proposed parking would be sufficient to serve the project, parking surveys were conducted at three similar hotels in Cupertino during the period of peak parking demand. The three hotels that were surveyed include: Aloft (located at 10165 North De Anza Boulevard), Cupertino Inn (located at 10889 N De Anza Boulevard) and Hilton Garden Inn (located at 10741 N Wolfe Road). The Cupertino Inn has 124 guest rooms, a small bar area, and some small meeting rooms. The Hilton Garden Inn has 164 guest rooms, a small bar and restaurant area, and three small meeting rooms. The Aloft Cupertino has 138 guest rooms and also includes a restaurant, bar, lounge and conference rooms. These three comparable hotels were chosen based on the following considerations:

- Located in the City of Cupertino
- Similar land uses (business hotel)
- Proximity to major arterials providing freeway access (e.g., De Anza Boulevard and Wolfe Road)
- Proximity to the Heart of the City Specific Plan area

Parking Survey Results

The counts were conducted on Wednesday June 11, 2014 and Saturday June 14, 2014 from 6:00 PM to 10:00 PM. These are the hours when hotel parking is at its highest level. Parking rates were calculated for weekday and weekend. The results of the parking surveys are shown in Table 10.

The results indicate that weekday parking rates (Occupied spaces/Occupied room) varied from 0.32 to 0.59. The Cupertino Inn had a maximum parking demand of 0.47 spaces per occupied room, the Aloft Cupertino had a maximum parking demand of 0.55 spaces per room, and the Hilton Garden Inn had a maximum demand of 0.73 spaces per room. The average for these three hotels was 0.58 spaces per room. This number is lower than the 85th percentile parking ratio of 0.71 spaces per room that is published in the Institute of Transportation Engineers (ITE) Parking Generation Manual.



The weekend survey results indicated a higher parking rate for the Cupertino Inn and the Hilton Garden Inn and a lower rate at the Aloft Cupertino. The Cupertino Inn had a maximum parking demand of 0.77 spaces per occupied room, the Aloft Cupertino had a maximum parking demand of 0.40 spaces per room, and the Hilton Garden Inn had a maximum demand of 0.81 spaces per room with 100% occupancy, which is the highest rate among all the three hotels during either the weekday or weekend. The maximum demand of 0.81 includes the demand for parking related to the restaurant and bar area seating and employee staffing.

The proposed Hyatt House hotel includes a full service restaurant, bar, and meeting space, which is quite similar to the Hilton Garden Inn. The proposed hotel would provide 156 parking spaces, a rate of 1.05 parking spaces per guestroom, which is higher than the highest parking rate of 0.81 from the survey. Therefore, it could be concluded that the proposed 156 parking spaces would be adequate.

The site plan indicates that there will be four tandem parking stalls, providing eight of the parking spaces provided in the garage. The applicant has indicated that those spaces would be only for employee use. Tandem parking has proved to be a feasible solution in similar circumstances, as long as the management develops a realistic plan for moving employee cars when necessary in order to allow employees to leave when their shift is over. The hotel management will need to develop such a plan for the tandem spaces, taking into account the start and end times of employees' shifts in order to minimize the amount of vehicle re-parking that needs to occur.

Table 10
Parking Survey Results

Time	Parked Cars					
	Cupertino Inn (Total 126 Rooms)		Aloft Hotel (Total 123 Rooms)		Hilton Garden Inn (Total 164 Rooms)	
	Wednesday June 11th	Saturday June 14th	Wednesday June 11th	Saturday June 14th	Wednesday June 11th	Saturday June 14th
6:00 PM	40	62	31	36	58	77
6:30 PM	36	68	30	34	49	69
7:00 PM	37	64	35	33	54	78
7:30 PM	35	64	31	39	59	85
8:00 PM	38	69	33	40	71	91
8:30 PM	40	70	32	42	82	97
9:00 PM	44	72	42	43	99	112
9:30 PM	52	76	56	47	107	122
10:00 PM	59	87	68	49	119	133
<hr/>						
Occupied Rooms	126	113	123	121	164	164
Highest Parking Ratio	0.47	0.77	0.55	0.40	0.73	0.81

7. Cumulative Conditions

This chapter presents a summary of the traffic conditions that would occur under cumulative conditions. Cumulative conditions typically include specific development projects that are being reviewed (pending projects) but are not yet approved. A significant cumulative traffic impact is identified by comparing project traffic against baseline cumulative conditions. This cumulative traffic scenario is evaluated in order to fulfill CMP and California Environmental Quality Act (CEQA) requirements.

Cumulative Traffic Volumes

Cumulative traffic conditions were calculated by adding the trips generated by pending developments to background traffic volumes. Traffic volumes for the baseline cumulative conditions were based on the traffic volumes assumed for the cumulative plus project conditions in the Apple Campus 2 EIR. The peak hour baseline cumulative traffic volumes are shown graphically on Figure 13. Project traffic was then added to the baseline cumulative conditions to evaluate project impacts under cumulative conditions. Cumulative plus project peak hour traffic volumes are shown on Figure 14.

Cumulative Transportation Network

The roadway network under cumulative conditions was assumed to include all the proposed improvements under background conditions as well as the following additional improvement:

1. Wolfe Road/Vallco Parkway intersection – The Apple Campus 2 project will modify the traffic signal operations to provide an overlap phase for the westbound right-turn movement. Southbound U-turns will be prohibited. The westbound approach will be modified to provide one left-turn lane, one shared left-turn/through lane, and two right-turn lanes.

Cumulative Intersection Level of Service Analysis

Intersection levels of service were evaluated against City of Cupertino and CMP standards. The results of the intersection level of service analysis under cumulative conditions are summarized in Table 11.

City of Cupertino Intersection Analysis

The results of the level of service analysis under cumulative conditions show that all of the study intersections would operate at an acceptable level of service (LOS D or better) during both the AM and PM peak hours of traffic.

CMP Intersection Analysis

The results of the level of service analysis under cumulative conditions show that all of the study CMP intersections would operate at an acceptable level of service (LOS E or better) during the AM and PM peak hours of traffic.

The intersection level of service calculation sheets are included in Appendix B.

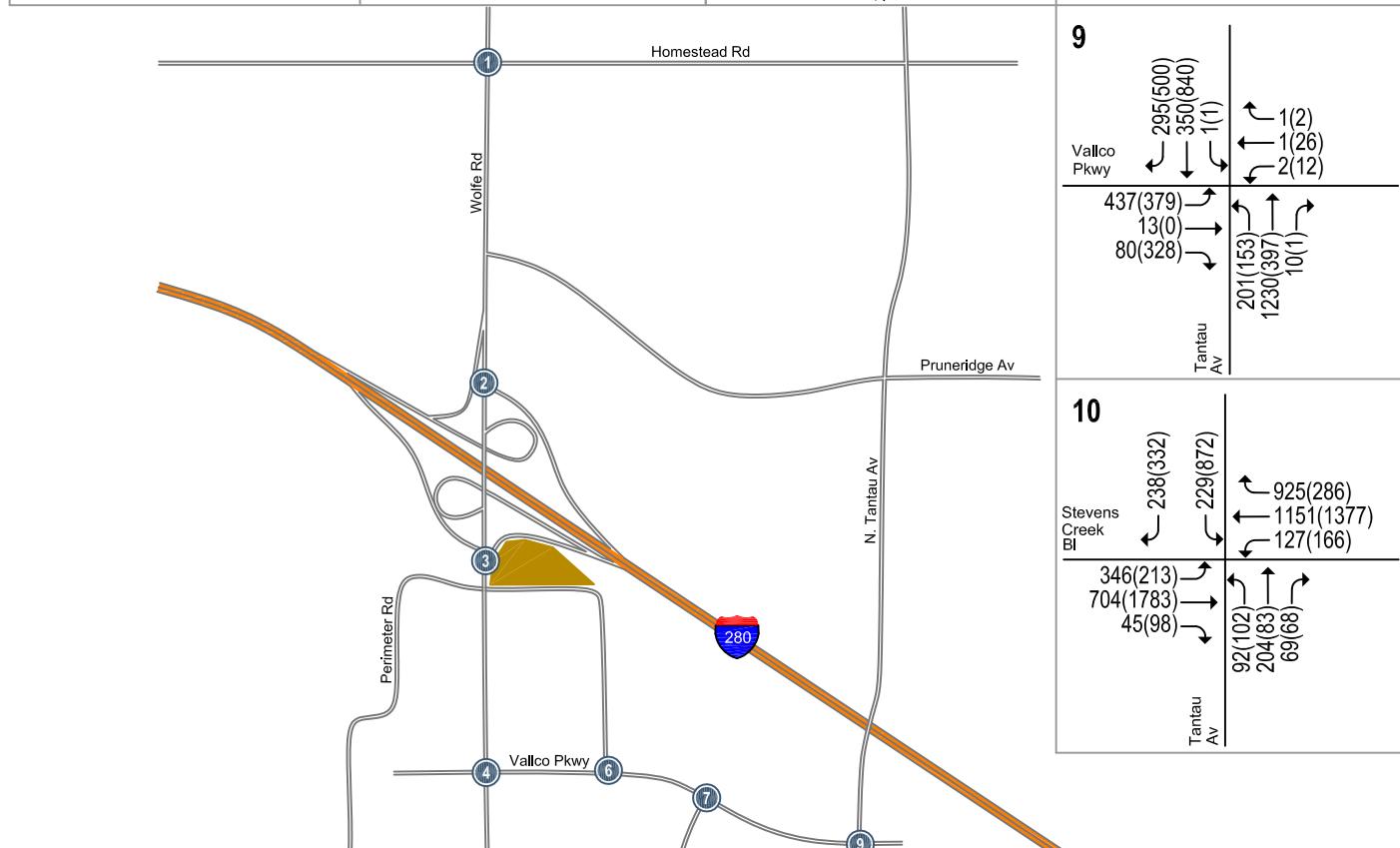
Table 11
Cumulative Intersection Levels of Service

Study Number	Intersection	Peak Hour	Cumulative		Cumulative Plus Project		
			Avg. Delay (sec)	LOS	Avg. Delay (sec)	LOS	Incr. In Crit. Delay (sec)
1	Wolfe Rd & Homestead Rd	AM	39.9	D	40.0	D	0.3
		PM	51.6	D	51.9	D	0.5
2	Wolfe Rd & I-280 NB Ramp*	AM	18.1	B	18.1	B	0.1
		PM	27.6	C	27.9	C	0.2
3	Wolfe Rd & I-280 SB Ramp*	AM	19.2	B	19.4	B	0.5
		PM	29.3	C	30.9	C	2.8
4	Wolfe Rd & Vallco Pkwy	AM	28.0	C	28.0	C	0.0
		PM	35.0	C	35.1	D	0.1
5	Wolfe Rd & Stevens Creek Blvd*	AM	39.9	D	40.0	D	0.2
		PM	49.2	D	49.6	D	0.6
6	Perimeter Rd & Vallco Pkwy	AM	4.1	A	4.2	A	0.2
		PM	6.0	A	6.1	A	0.1
7	Finch Ave & Vallco Pkwy ¹	AM	25.8	C	26.0	C	0.2
		PM	38.1	D	39.2	D	1.1
8	Finch Ave & Stevens Creek Blvd	AM	26.3	C	26.3	C	0.0
		PM	44.5	D	44.5	D	0.0
9	N Tantau Ave & Vallco Pkwy	AM	27.2	C	27.3	C	0.1
		PM	33.4	C	33.7	C	0.4
10	N Tantau Ave & Stevens Creek Blvd	AM	43.4	D	43.4	D	0.0
		PM	49.4	D	49.6	D	0.3

<u>Notes:</u>
* denotes CMP intersection
¹ This intersection was analyzed as an unsignalized intersection under existing conditions and as a signalized
Bold indicates a significant impact.

Hyatt House Hotel, Cupertino

1	2	3	4
Homestead Rd 121(131) 903(1248) 242(224) ↓ 226(221) 583(853) 584(694)	Wolfe Rd 568(1209) 1198(2281) ↓ 1059(735) 421(726)	I-280 SB Off-Ramp 1310(468) 840(991)	Valco Pkwy 74(166) 917(1434) 790(1059)
Wolfe Rd 131(221) 612(797) 345(309) ↓ 287(499) 793(892) 432(503)	Wolfe Rd 2376(1474) 474(1200)	I-280 NB Off-Ramp 1560(2296) 496(769)	278(1233) 3(13) 53(323)



LEGEND

= Project Site Location

= Study Intersection

XX(XX) = AM(PM) Peak-Hour Traffic Volumes

Hyatt House Hotel, Cupertino

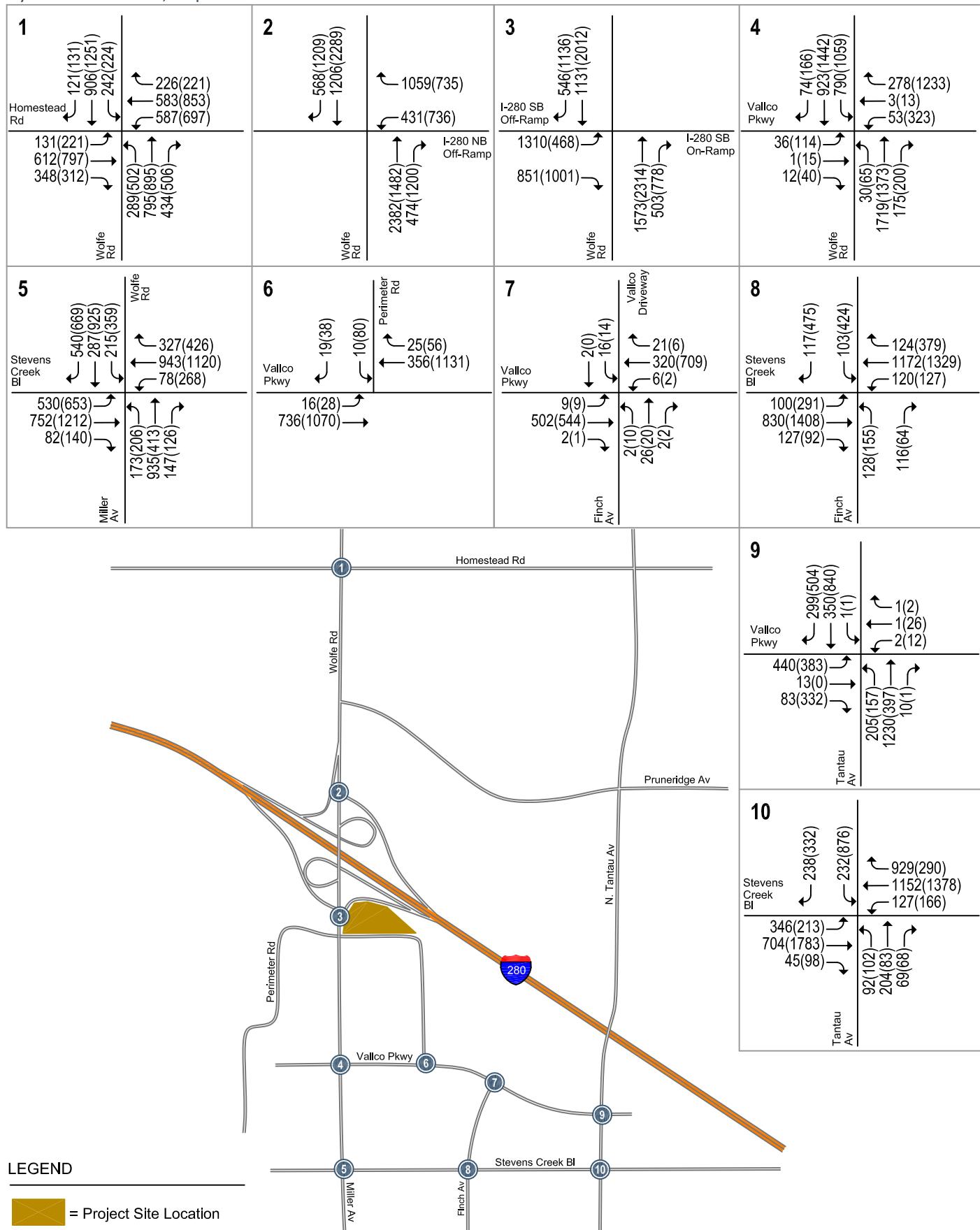


Figure 13
Cumulative Plus Project Traffic Volumes

8. **Conclusions**

The impacts of the project were evaluated following the standards and methodologies set forth by the City of Cupertino, and the Valley Transportation Agency (VTA). The VTA administers the Santa Clara County Congestion Management Program (CMP). Since the project would not generate more than 100 peak hour gross vehicle trips, a CMP analysis was not conducted. The study determined the traffic impacts of the project on nine (9) signalized intersections, and one (1) unsignalized intersection in the vicinity of the project site during the weekday AM and PM peak periods of traffic. Project impacts on other transportation facilities, such as bicycle facilities and transit services, were determined on the basis of engineering judgment.

Intersection Levels of Service

The results of the intersection level of service analysis show that, measured against the City of Cupertino and the Valley Transportation Agency (VTA) standards, all of the study intersections are expected to operate at acceptable levels of service under all conditions. The project would not cause a significant impact at any of the study intersections.

Other Transportation Issues

There are currently no sidewalks on Perimeter Road or on the connecting ramps to Wolfe Road. Thus, there are no good pedestrian connections from the hotel site to Wolfe Road, Vallco Parkway, or Stevens Creek Boulevard. . As shown on the site plan dated 7/30/2014, the hotel project will provide a sidewalk on the north side of Perimeter Road and two crosswalks across Perimeter Road, connecting the site to the rest of the Vallco Shopping District. One of the crosswalks will be located at the main entrance driveway, where there will be 4-way stop control. The other crosswalk will be west of the second driveway that leads to parking spaces. Also, the 7/30/2014 site plan shows pedestrian connections from the hotel to the new sidewalk on Perimeter Road near where the crosswalks will be placed..

The project will also provide a new pathway along the west side of the project site. This path will connect to a future trail along the south side of I-280. Although the trail on the south side of I-280 has not yet been fully studied, the pathway connection provided by the project will significantly enhance bicycle and pedestrian connectivity if it is ever implemented.

Parking

The project will provide 156 parking spaces which is a rate of 1.05 parking spaces per room. Hexagon surveyed parking utilization at three similar hotels in Cupertino, and the highest ratio was found to be 0.81 parking spaces per occupied room. The proposed 156 parking spaces would be adequate based on the highest surveyed peak hour parking demand.



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Hyatt House Hotel

Draft Transportation Impact Analysis **Technical Appendices**

Appendix A

New Traffic Counts

AM Peak-Hour Volume Count Worksheet

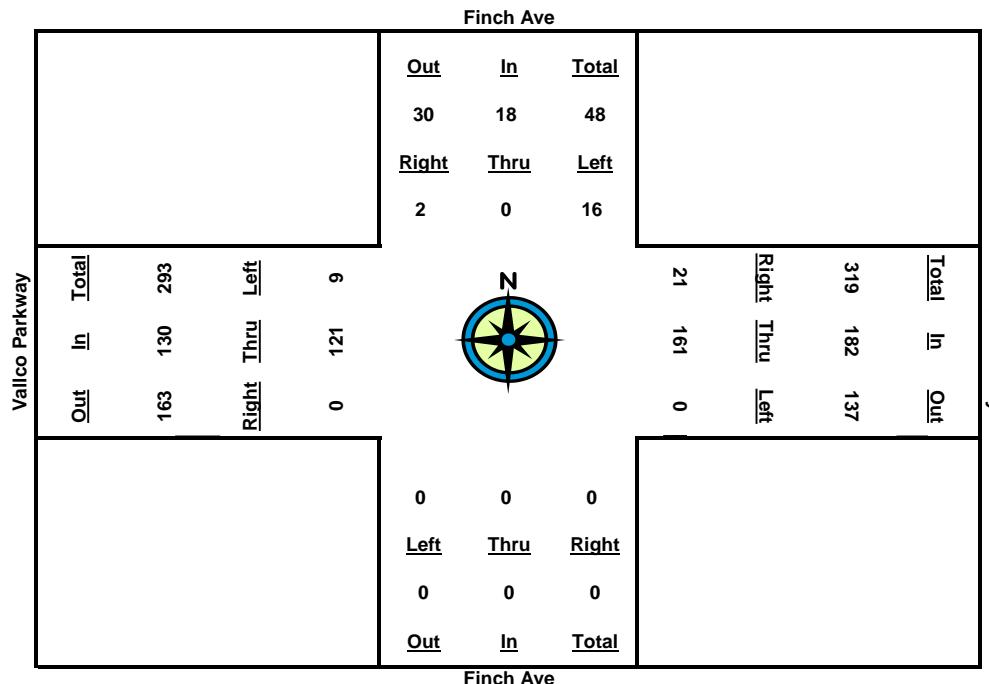
Date: 5/13/14
 Counter: Patti
 Intersection Name: Finch Ave and Valco Parkway
 Weather: Clear Cupertino

AUTO-CENSUS
Traffic Monitoring and Analysis
 870 Castlewood Dr. #1
 Los Gatos, CA 95032
 Phone 408-826-9673 Fax 408-877-1625

Start Time	Finch Ave				Valco Parkway				Finch Ave				Valco Parkway			
	North Approach				East Approach				South Approach				West Approach			
	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total
7:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15	0	0	5	5	2	18	0	20	0	0	0	0	0	18	5	23
7:30	0	0	6	6	4	56	0	60	0	0	0	0	0	49	5	54
7:45	0	0	9	9	12	111	0	123	0	0	0	0	0	87	10	97
8:00	0	0	17	17	17	142	0	159	0	0	0	0	0	111	10	121
8:15	1	0	21	22	20	169	0	189	0	0	0	0	0	137	11	148
8:30	2	0	22	24	25	217	0	242	0	0	0	0	0	170	14	184
8:45	2	0	29	31	27	256	0	283	0	0	0	0	0	201	17	218
9:00	3	0	33	36	33	285	0	318	0	0	0	0	0	242	20	262

Peak Hour	Right	Thru	Left	Total	PK Hour												
7:00 - 8:00	0	0	17	17	17	142	0	159	0	0	0	0	0	111	10	121	297
7:15 - 8:15	1	0	16	17	18	151	0	169	0	0	0	0	0	119	6	125	311
7:30 - 8:30	2	0	16	18	21	161	0	182	0	0	0	0	0	121	9	130	330
7:45 - 8:45	2	0	20	22	15	145	0	160	0	0	0	0	0	114	7	121	303
8:00 - 9:00	3	0	16	19	16	143	0	159	0	0	0	0	0	131	10	141	319

Cut and Paste	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
	0	0	0	16	0	2	9	121	0	0	161	21



PM Peak-Hour Volume Count Worksheet

Date: 5/13/14
 Counter: Patti
 Intersection Name: Finch Ave and Vallco Parkway
 Weather: Clear Cupertino

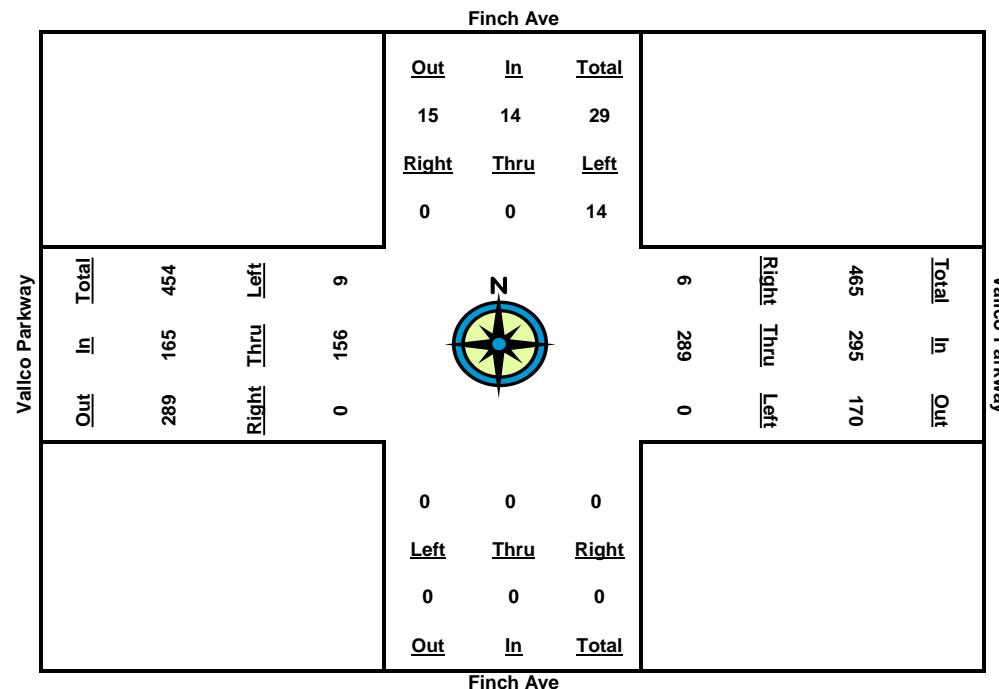
AUTO-CENSUS

Traffic Monitoring and Analysis
 870 Castlewood Dr. #1
 Los Gatos, CA 95032
 Phone 408-826-9673 Fax 408-877-1625

Start Time	Finch Ave				Vallco Parkway				Finch Ave				Vallco Parkway			
	North Approach				East Approach				South Approach				West Approach			
	Right	Thru	Left	Total												
4:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15	2	0	4	6	3	30	0	33	0	0	0	0	0	33	5	38
4:30	2	0	6	8	7	65	0	72	0	0	0	0	0	66	5	71
4:45	4	0	8	12	9	108	0	117	0	0	0	0	0	100	8	108
5:00	4	0	11	15	11	153	0	164	0	0	0	0	0	118	8	126
5:15	4	0	17	21	12	224	0	236	0	0	0	0	0	148	11	159
5:30	4	0	23	27	17	300	0	317	0	0	0	0	0	180	11	191
5:45	4	0	25	29	17	378	0	395	0	0	0	0	0	221	16	237
6:00	4	0	25	29	17	442	0	459	0	0	0	0	0	274	17	291

Peak Hour	Right	Thru	Left	Total	PK Hour												
4:00 - 5:00	4	0	11	15	11	153	0	164	0	0	0	0	0	118	8	126	305
4:15 - 5:15	2	0	13	15	9	194	0	203	0	0	0	0	0	115	6	121	339
4:30 - 5:30	2	0	17	19	10	235	0	245	0	0	0	0	0	114	6	120	384
4:45 - 5:45	0	0	17	17	8	270	0	278	0	0	0	0	0	121	8	129	424
5:00 - 6:00	0	0	14	14	6	289	0	295	0	0	0	0	0	156	9	165	474
Peak Volumes:	0	0	14	14	6	289	0	295	0	0	0	0	0	156	9	165	474

Cut and Paste	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Total	In	Out	Total
	0	0	0	14	0	0	9	156	0	0	289	6	0	0	0	14



AM Peak-Hour Volume Count Worksheet

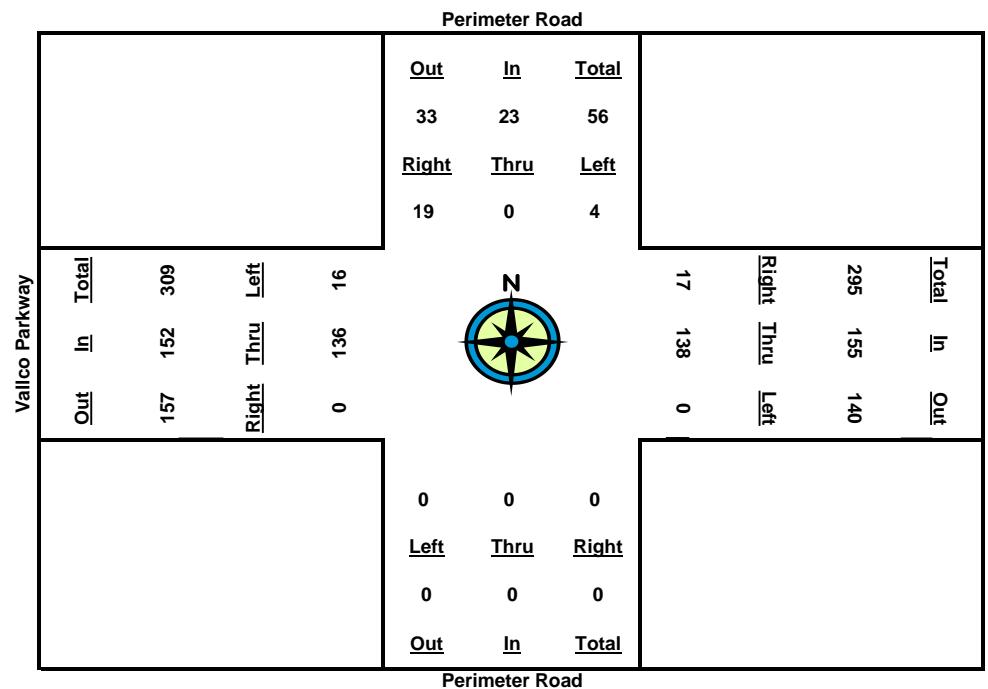
Date: 5/15/14
 Counter: Kevin and An
 Intersection Name: Perimeter Road and Valco Parkway
 Weather: Clear San Jose

AUTO-CENSUS
Traffic Monitoring and Analysis
 870 Castlewood Dr. #1
 Los Gatos, CA 95032
 Phone 408-826-9673 Fax 408-877-1625

Start Time	Perimeter Road				Valco Parkway				Perimeter Road				Valco Parkway			
	North Approach				East Approach				South Approach				West Approach			
	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total
7:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15	1	0	0	1	0	7	0	7	0	0	0	0	0	17	0	17
7:30	2	0	1	3	3	24	0	27	0	0	0	0	0	47	0	47
7:45	6	0	7	13	12	67	0	79	0	0	0	0	0	88	3	91
8:00	7	0	8	15	17	86	0	103	0	0	0	0	0	112	5	117
8:15	9	0	8	17	20	111	0	131	0	0	0	0	0	132	7	139
8:30	14	0	9	23	22	152	0	174	0	0	0	0	0	174	9	183
8:45	19	0	11	30	26	188	0	214	0	0	0	0	0	204	14	218
9:00	26	0	12	38	34	224	0	258	0	0	0	0	0	248	21	269

Peak Hour	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	PK Hour
7:00 - 8:00	7	0	8	15	17	86	0	103	0	0	0	0	0	112	5	117	235
7:15 - 8:15	8	0	8	16	20	104	0	124	0	0	0	0	0	115	7	122	262
7:30 - 8:30	12	0	8	20	19	128	0	147	0	0	0	0	0	127	9	136	303
7:45 - 8:45	13	0	4	17	14	121	0	135	0	0	0	0	0	116	11	127	279
8:00 - 9:00	19	0	4	23	17	138	0	155	0	0	0	0	0	136	16	152	330
Peak Volumes:	19	0	4	23	17	138	0	155	0	0	0	0	0	136	16	152	330

Cut and Paste	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR				
	0	0	0	4	0	19	16	136	0	0	138	17				



PM Peak-Hour Volume Count Worksheet

Date: 5/15/14

Counter: Kevin and An

Intersection Name: Perimeter Road and Valco Parkway

Weather: Clear Cupertino

AUTO-CENSUS

Traffic Monitoring and Analysis

870 Castlewood Dr. #1

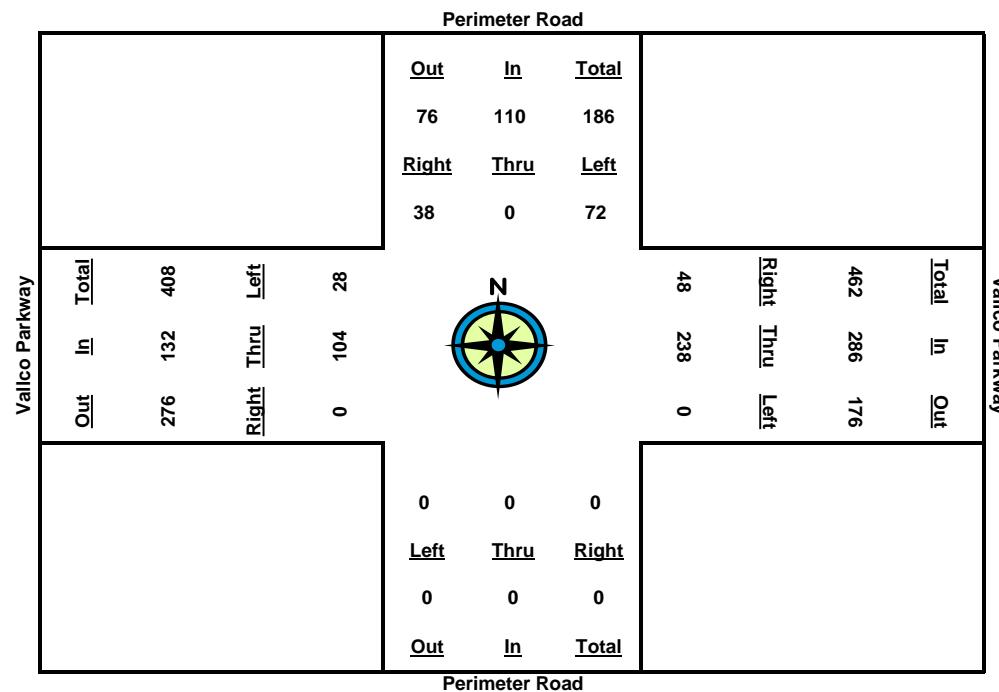
Los Gatos, CA 95032

Phone 408-826-9673 Fax 408-877-1625

Start Time	Perimeter Road				Valco Parkway				Perimeter Road				Valco Parkway			
	North Approach				East Approach				South Approach				West Approach			
	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total
4:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15	14	0	7	21	6	52	0	58	0	0	0	0	0	13	1	14
4:30	18	0	17	35	14	81	0	95	0	0	0	0	0	31	5	36
4:45	25	0	24	49	20	119	0	139	0	0	0	0	0	47	11	58
5:00	34	0	36	70	29	162	0	191	0	0	0	0	0	67	20	87
5:15	44	0	52	96	42	227	0	269	0	0	0	0	0	96	27	123
5:30	51	0	63	114	50	294	0	344	0	0	0	0	0	113	29	142
5:45	60	0	94	154	59	348	0	407	0	0	0	0	0	141	45	186
6:00	72	0	108	180	77	400	0	477	0	0	0	0	0	171	48	219

Peak Hour	Right	Thru	Left	Total	PK Hour												
4:00 - 5:00	34	0	36	70	29	162	0	191	0	0	0	0	0	67	20	87	348
4:15 - 5:15	30	0	45	75	36	175	0	211	0	0	0	0	0	83	26	109	395
4:30 - 5:30	33	0	46	79	36	213	0	249	0	0	0	0	0	82	24	106	434
4:45 - 5:45	35	0	70	105	39	229	0	268	0	0	0	0	0	94	34	128	501
5:00 - 6:00	38	0	72	110	48	238	0	286	0	0	0	0	0	104	28	132	528
Peak Volumes:	38	0	72	110	48	238	0	286	0	0	0	0	0	104	28	132	528

Cut and Paste	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
	0	0	0	72	0	38	28	104	0	0	238	48

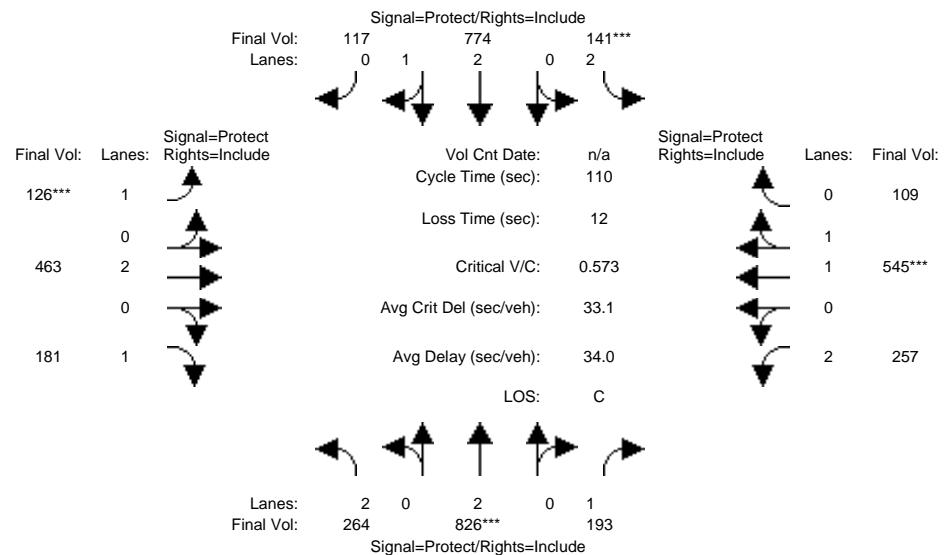


Appendix B

Intersection Level of Service Calculations

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Existing AM

Intersection #18: Wolfe Road and Homestead Road (CUP)



Street Name:		Wolfe Road				Homestead Road			
Approach:	North Bound	South Bound		East Bound		West Bound			
Movement:	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R		
Min. Green:	7 10	10 7	10 7	10 7	10 7	10 7	10 7		
Y+R:	4.0 4.0	4.0 4.0	4.0 4.0	4.0 4.0	4.0 4.0	4.0 4.0	4.0 4.0		

Volume Module:

Base Vol:	264	826	193	141	774	117	126	463	181	257	545	109
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	264	826	193	141	774	117	126	463	181	257	545	109
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	264	826	193	141	774	117	126	463	181	257	545	109
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	264	826	193	141	774	117	126	463	181	257	545	109
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	264	826	193	141	774	117	126	463	181	257	545	109
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	264	826	193	141	774	117	126	463	181	257	545	109

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.83	1.00	0.92	0.83	0.99	0.95	0.92	1.00	0.92	0.83	0.98
Lanes:	2.00	2.00	1.00	2.00	2.59	0.41	1.00	2.00	1.00	2.00	1.66
Final Sat.:	3150	3800	1750	3150	4864	735	1750	3800	1750	3150	3083

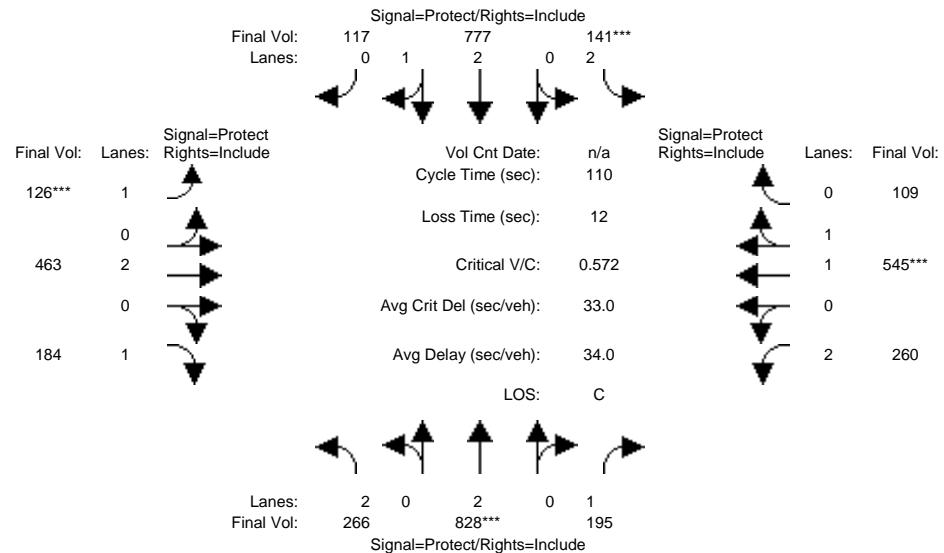
Capacity Analysis Module:

Vol/Sat:	0.08	0.22	0.11	0.04	0.16	0.16	0.07	0.12	0.10	0.08	0.18	0.18
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	17.3	41.7	41.7	8.6	32.9	32.9	13.8	28.6	28.6	19.1	33.9	33.9
Volume/Cap:	0.53	0.57	0.29	0.57	0.53	0.53	0.57	0.47	0.40	0.47	0.57	0.57
Delay/Veh:	43.7	27.7	24.1	52.2	32.4	32.4	49.0	34.7	34.2	41.5	32.7	32.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	43.7	27.7	24.1	52.2	32.4	32.4	49.0	34.7	34.2	41.5	32.7	32.7
LOS by Move:	D	C	C	D	C	C	D	C	C	D	C	C
HCM2kAvgQ:	5	11	5	3	8	8	4	6	5	5	9	9

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Ex + P AM

Intersection #18: Wolfe Road and Homestead Road (CUP)

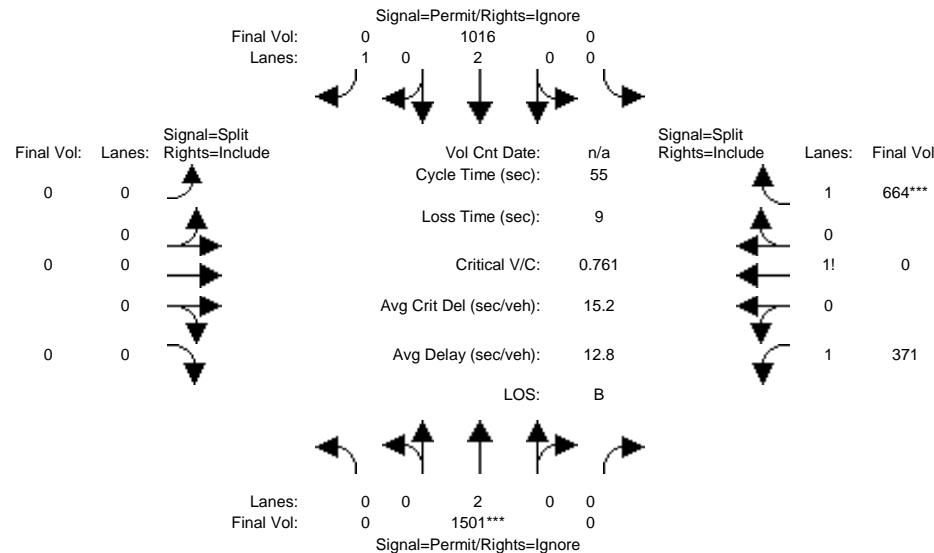


Street Name: Wolfe Road Homestead Road														
Approach:	North Bound			South Bound			East Bound			West Bound				
	L	-	T	-	R	L	-	T	-	R	L	-	T	-
Min. Green:	7 10		10 7		10 10		10 7		10 10		7 10		10 10	
Y+R:	4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0	
Volume Module:	<hr/>													
Base Vol:	264	826	193	141	774	117	126	463	181	257	545	109		
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Initial Bse:	264	826	193	141	774	117	126	463	181	257	545	109		
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0		
Hyatt:	2	2	2	0	3	0	0	0	3	3	0	0		
Initial Fut:	266	828	195	141	777	117	126	463	184	260	545	109		
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
PHF Volume:	266	828	195	141	777	117	126	463	184	260	545	109		
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0		
Reduced Vol:	266	828	195	141	777	117	126	463	184	260	545	109		
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
FinalVolume:	266	828	195	141	777	117	126	463	184	260	545	109		
Saturation Flow Module:	<hr/>													
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Adjustment:	0.83	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92		
Lanes:	2.00	2.00	1.00	2.00	2.58	0.42	1.00	2.00	1.00	2.00	1.64	0.36		
Final Sat.:	3150	3800	1750	3150	4899	738	1750	3800	1750	3150	3122	624		
Capacity Analysis Module:	<hr/>													
Vol/Sat:	0.08	0.22	0.11	0.04	0.16	0.16	0.07	0.12	0.11	0.08	0.17	0.17		
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****		
Green Time:	17.6	41.9	41.9	8.6	33.0	33.0	13.9	28.3	28.3	19.2	33.6	33.6		
Volume/Cap:	0.53	0.57	0.29	0.57	0.53	0.53	0.57	0.47	0.41	0.47	0.57	0.57		
Delay/Veh:	43.5	27.5	23.9	52.1	32.4	32.4	48.9	34.9	34.5	41.5	32.8	32.8		
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
AdjDel/Veh:	43.5	27.5	23.9	52.1	32.4	32.4	48.9	34.9	34.5	41.5	32.8	32.8		
LOS by Move:	D	C	C	D	C	C	D	C	C	D	C	C		
HCM2kAvgQ:	5	11	5	3	8	8	4	6	5	5	9	9		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Existing AM

Intersection #21: Wolfe Road and I-280 NB Ramps (CUP/CMP)



		Wolfe Road				I-280 NB Ramps			
Approach:	North Bound	South Bound		East Bound		West Bound			
Movement:	L - T - R	L - T - R		L - T - R		L - T - R			
Min. Green:	7 10	10	7 10	10	0 0	0 0	0 0	10 10	10
Y+R:	4.0 4.0	4.0	4.0 4.0	4.0	4.0 4.0	4.0 4.0	4.0 4.0	4.0 4.0	4.0

Volume Module:

Base Vol:	0	1501	273	0	1016	465	0	0	0	371	0	664
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1501	273	0	1016	465	0	0	0	371	0	664
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1501	273	0	1016	465	0	0	0	371	0	664
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1501	0	0	1016	0	0	0	0	371	0	664
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1501	0	0	1016	0	0	0	0	371	0	664
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1501	0	0	1016	0	0	0	0	371	0	664

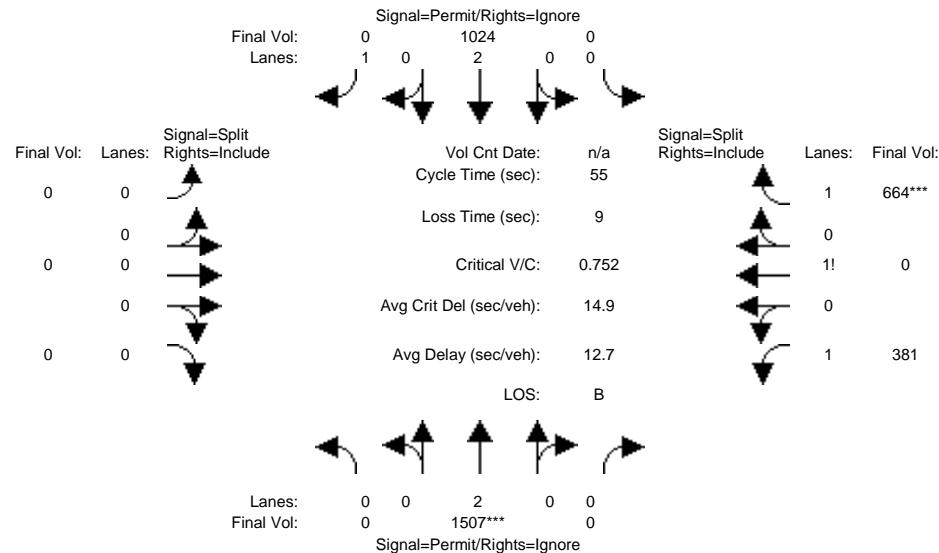
Saturation Flow Module:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.97	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	0.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	1.36	0.00	1.64
Final Sat.:	0	3700	0	0	3800	1750	0	0	0	2377	0	2873

Capacity Analysis Module:	0.00	0.41	0.00	0.00	0.27	0.00	0.00	0.00	0.00	0.16	0.00	0.23
Vol/Sat:	****	****	****	****	****	****	****	****	****	****	****	****
Crit Moves:	0.00	0.76	0.00	0.00	0.50	0.00	0.00	0.00	0.00	0.51	0.00	0.76
Green Time:	0.0	29.3	0.0	0.0	29.3	0.0	0.0	0.0	0.0	16.7	0.0	16.7
Volume/Cap:	0.00	11.9	0.0	0.0	8.4	0.0	0.0	0.0	0.0	16.0	0.0	19.9
Delay/Veh:	0.0	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	11.9	0.0	0.0	8.4	0.0	0.0	0.0	0.0	16.0	0.0	19.9
LOS by Move:	A	B	A	A	A	A	A	A	A	B	A	B
HCM2kAvgQ:	0	11	0	0	5	0	0	0	0	5	0	9

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Ex + P AM

Intersection #21: Wolfe Road and I-280 NB Ramps (CUP/CMP)



Street Name:		Wolfe Road						I-280 NB Ramps					
Approach:	North Bound	South Bound			East Bound			West Bound					
Movement:	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R
Min. Green:	7	10	10	7	10	10	0	0	0	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	1501	273	0	1016	465	0	0	0	371	0	664
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1501	273	0	1016	465	0	0	0	371	0	664
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Hyatt:	0	6	0	0	8	0	0	0	0	10	0	0
Initial Fut:	0	1507	273	0	1024	465	0	0	0	381	0	664
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1507	0	0	1024	0	0	0	0	381	0	664
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1507	0	0	1024	0	0	0	0	381	0	664
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1507	0	0	1024	0	0	0	0	381	0	664

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	0.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	1.36	0.00	1.64
Final Sat.:	0	3800	0	0	3800	1750	0	0	0	2388	0	2862

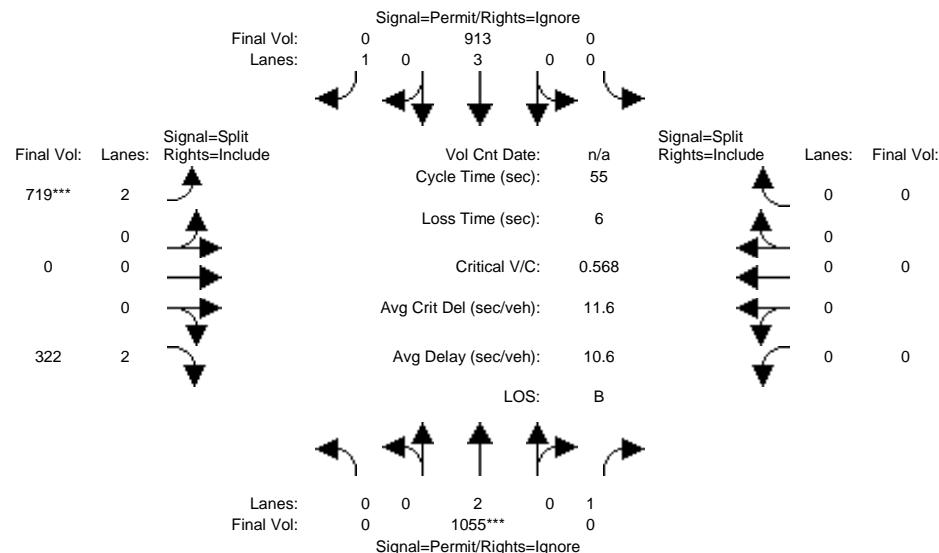
Capacity Analysis Module:

Vol/Sat:	0.00	0.40	0.00	0.00	0.27	0.00	0.00	0.00	0.00	0.16	0.00	0.23
Crit Moves:	****									****		
Green Time:	0.0	29.0	0.0	0.0	29.0	0.0	0.0	0.0	0.0	17.0	0.0	17.0
Volume/Cap:	0.00	0.75	0.00	0.00	0.51	0.00	0.00	0.00	0.00	0.52	0.00	0.75
Delay/Veh:	0.0	11.8	0.0	0.0	8.6	0.0	0.0	0.0	0.0	15.9	0.0	19.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	11.8	0.0	0.0	8.6	0.0	0.0	0.0	0.0	15.9	0.0	19.5
LOS by Move:	A	B	A	A	A	A	A	A	A	B	A	B
HCM2kAvgQ:	0	10	0	0	6	0	0	0	0	5	0	9

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Existing AM

Intersection #22: Wolfe Road and I-280 SB Ramps (CUP/CMP)



		Wolfe Road				I-280 SB Ramps			
Approach:	North Bound	South Bound		East Bound		West Bound			
Movement:	L - T - R	L - T - R		L - T - R		L - T - R			
Min. Green:	7 10	10	7 10	10	10	10 10	10	0 0	0 0
Y+R:	4.0 4.0	4.0	4.0 4.0	4.0	4.0	4.0 4.0	4.0	4.0 4.0	4.0

Volume Module:

Base Vol:	0	1055	433	0	913	474	719	0	322	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1055	433	0	913	474	719	0	322	0	0	0
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1055	433	0	913	474	719	0	322	0	0	0
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1055	0	0	913	0	719	0	322	0	0	0
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1055	0	0	913	0	719	0	322	0	0	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1055	0	0	913	0	719	0	322	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.83	0.92	1.00	0.92
Lanes:	0.00	2.00	1.00	0.00	3.00	1.00	2.00	0.00	2.00	0.00	0.00	0.00
Final Sat.:	0	3800	1750	0	5700	1750	3150	0	3150	0	0	0

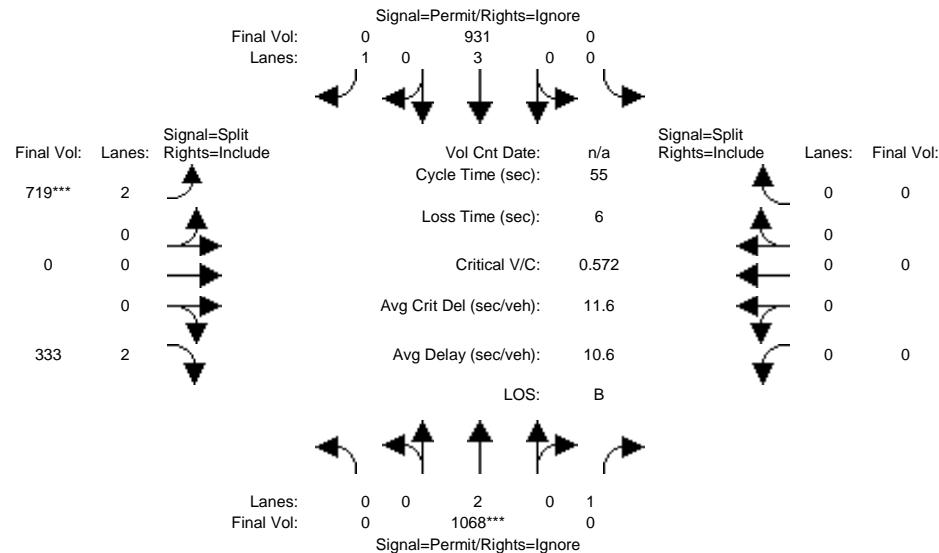
Capacity Analysis Module:

Vol/Sat:	0.00	0.28	0.00	0.00	0.16	0.00	0.23	0.00	0.10	0.00	0.00	0.00
Crit Moves:	****						****					
Green Time:	0.0	26.9	0.0	0.0	26.9	0.0	22.1	0.0	22.1	0.0	0.0	0.0
Volume/Cap:	0.00	0.57	0.00	0.00	0.33	0.00	0.57	0.00	0.25	0.00	0.00	0.00
Delay/Veh:	0.0	10.4	0.0	0.0	8.6	0.0	13.4	0.0	11.1	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	10.4	0.0	0.0	8.6	0.0	13.4	0.0	11.1	0.0	0.0	0.0
LOS by Move:	A	B	A	A	A	A	B	A	B	A	A	A
HCM2kAvgQ:	0	6	0	0	3	0	6	0	2	0	0	0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Ex + P AM

Intersection #22: Wolfe Road and I-280 SB Ramps (CUP/CMP)



Street Name:		Wolfe Road										I-280 SB Ramps					
Approach:	North Bound	South Bound				East Bound				West Bound							
Movement:	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R
Min. Green:	7	10	10	7	10	10	10	10	10	10	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	1055	433	0	913	474	719	0	322	0	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1055	433	0	913	474	719	0	322	0	0	0	0	0	0	0	0
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hyatt:	0	13	7	0	18	0	0	0	11	0	0	0	0	0	0	0	0
Initial Fut:	0	1068	440	0	931	474	719	0	333	0	0	0	0	0	0	0	0
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1068	0	0	931	0	719	0	333	0	0	0	0	0	0	0	0
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1068	0	0	931	0	719	0	333	0	0	0	0	0	0	0	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1068	0	0	931	0	719	0	333	0	0	0	0	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.83	0.92	1.00	0.92	1.00	0.92	1.00	0.92
Lanes:	0.00	2.00	1.00	0.00	3.00	1.00	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Final Sat.:	0	3800	1750	0	5700	1750	3150	0	3150	0	0	0	0	0	0	0

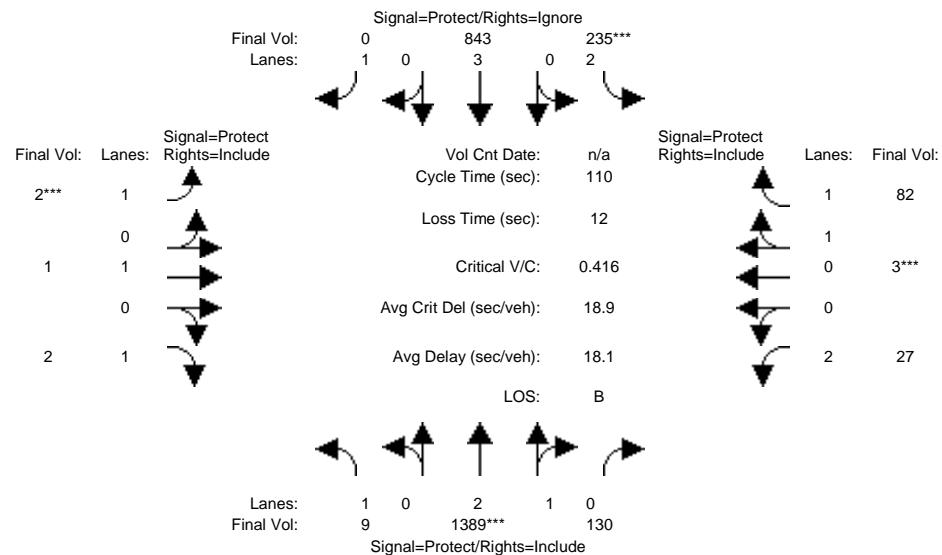
Capacity Analysis Module:

Vol/Sat:	0.00	0.28	0.00	0.00	0.16	0.00	0.23	0.00	0.11	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****						****									
Green Time:	0.0	27.0	0.0	0.0	27.0	0.0	22.0	0.0	22.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Volume/Cap:	0.00	0.57	0.00	0.00	0.33	0.00	0.57	0.00	0.26	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	0.0	10.3	0.0	0.0	8.6	0.0	13.5	0.0	11.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	10.3	0.0	0.0	8.6	0.0	13.5	0.0	11.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0
LOS by Move:	A	B	A	A	A	A	B	A	B	A	A	A	A	A	A	A
HCM2kAvgQ:	0	6	0	0	3	0	6	0	2	0	0	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Existing AM

Intersection #23: Wolfe Road and Vallco Parkway (CUP)



Street Name:	Wolfe Road				Vallco Parkway										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	10	
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	

Volume Module:

Base Vol:	9	1389	130	235	843	18	2	1	2	27	3	82			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	9	1389	130	235	843	18	2	1	2	27	3	82			
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0			
Initial Fut:	9	1389	130	235	843	18	2	1	2	27	3	82			
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Volume:	9	1389	130	235	843	0	2	1	2	27	3	82			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	9	1389	130	235	843	0	2	1	2	27	3	82			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
FinalVolume:	9	1389	130	235	843	0	2	1	2	27	3	82			

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.99	0.95	0.83	1.00	0.92	0.92	1.00	0.92	0.83	0.95	0.95
Lanes:	1.00	2.73	0.27	2.00	3.00	1.00	1.00	1.00	1.00	2.00	0.07	1.93
Final Sat.:	1750	5120	479	3150	5700	1750	1750	1900	1750	3150	127	3473

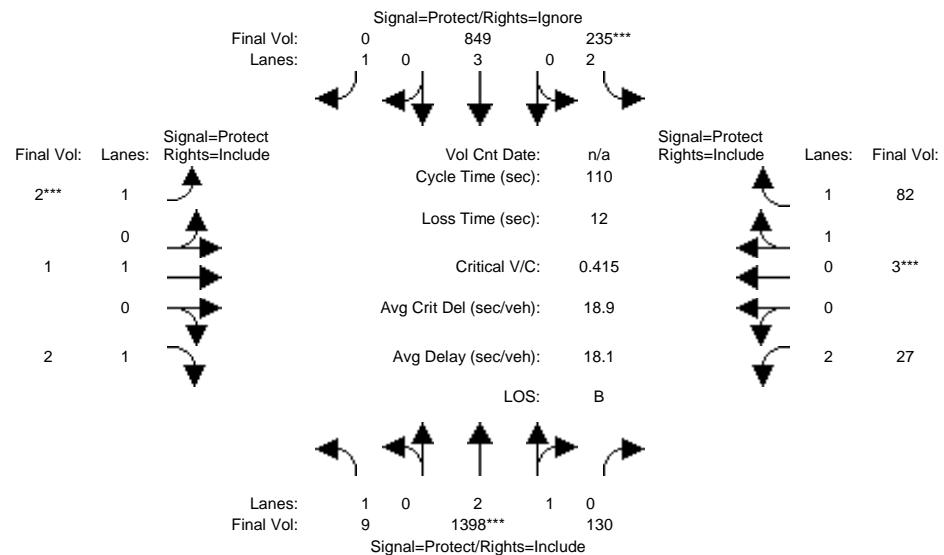
Capacity Analysis Module:

Vol/Sat:	0.01	0.27	0.27	0.07	0.15	0.00	0.00	0.00	0.00	0.01	0.02	0.02
Crit Moves:	****			****		****		****		****		
Green Time:	24.4	63.5	63.5	17.5	56.6	0.0	7.0	10.0	10.0	7.0	10.0	10.0
Volume/Cap:	0.02	0.47	0.47	0.47	0.29	0.00	0.02	0.01	0.01	0.13	0.26	0.26
Delay/Veh:	33.5	13.6	13.6	42.8	15.2	0.0	48.3	45.5	45.5	48.9	47.0	47.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	33.5	13.6	13.6	42.8	15.2	0.0	48.3	45.5	45.5	48.9	47.0	47.0
LOS by Move:	C	B	B	D	B	A	D	D	D	D	D	D
HCM2kAvgQ:	0	10	10	4	5	0	0	0	0	1	1	1

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Ex + P AM

Intersection #23: Wolfe Road and Vallco Parkway (CUP)



Street Name:	Wolfe Road				Vallco Parkway										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	7	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	9	1389	130	235	843	18	2	1	2	27	3	82				
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
Initial Bse:	9	1389	130	235	843	18	2	1	2	27	3	82				
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0				
Hyatt:	0	9	0	0	6	0	0	0	0	0	0	0				
Initial Fut:	9	1398	130	235	849	18	2	1	2	27	3	82				
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00				
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00				
PHF Volume:	9	1398	130	235	849	0	2	1	2	27	3	82				
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0				
Reduced Vol:	9	1398	130	235	849	0	2	1	2	27	3	82				
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00				
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00				
FinalVolume:	9	1398	130	235	849	0	2	1	2	27	3	82				

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92
Lanes:	1.00	2.72	0.28	2.00	3.00	1.00	1.00	1.00	1.00	2.00	0.07	1.93
Final Sat.:	1750	5177	481	3150	5700	1750	1750	1900	1750	3150	124	3386

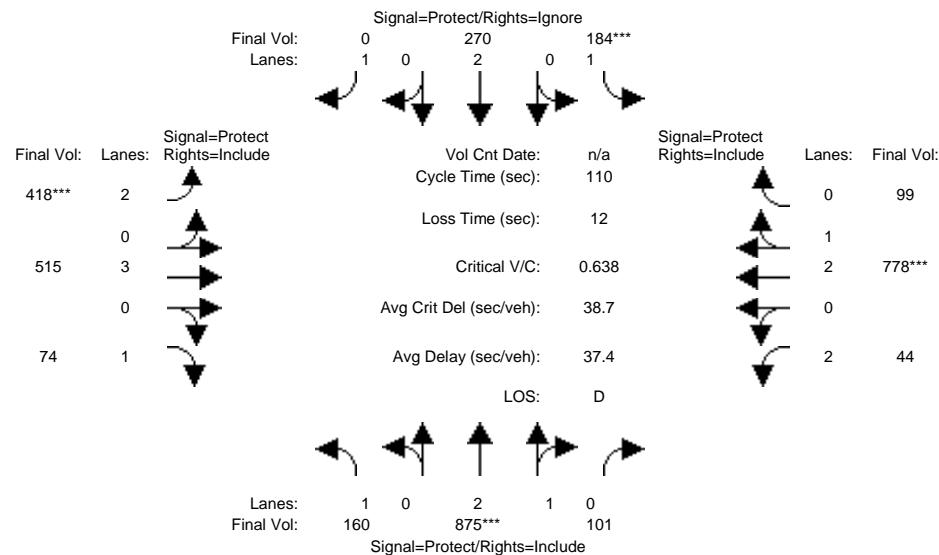
Capacity Analysis Module:

Vol/Sat:	0.01	0.27	0.27	0.07	0.15	0.00	0.00	0.00	0.00	0.01	0.02	0.02
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	24.2	63.5	63.5	17.5	56.8	0.0	7.0	10.0	10.0	7.0	10.0	10.0
Volume/Cap:	0.02	0.47	0.47	0.47	0.29	0.00	0.02	0.01	0.01	0.13	0.27	0.27
Delay/Veh:	33.6	13.6	13.6	42.7	15.2	0.0	48.3	45.5	45.5	48.9	47.0	47.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	33.6	13.6	13.6	42.7	15.2	0.0	48.3	45.5	45.5	48.9	47.0	47.0
LOS by Move:	C	B	B	D	B	A	D	D	D	D	D	D
HCM2kAvgQ:	0	10	10	4	5	0	0	0	0	1	1	1

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Existing AM

Intersection #24: Stevens Creek Blvd and Wolfe Rd/Miller Ave (CUP/CMP)



Street Name:	Wolfe Rd/Miller Ave				Stevens Creek Blvd										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	10	
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	

Volume Module:

Base Vol:	160	875	101	184	270	468	418	515	74	44	778	99			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	160	875	101	184	270	468	418	515	74	44	778	99			
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0			
Initial Fut:	160	875	101	184	270	468	418	515	74	44	778	99			
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Volume:	160	875	101	184	270	0	418	515	74	44	778	99			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	160	875	101	184	270	0	418	515	74	44	778	99			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
FinalVolume:	160	875	101	184	270	0	418	515	74	44	778	99			

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.99	0.95	0.92	1.00	0.92	0.83	1.00	0.92	0.83	0.99	0.95
Lanes:	1.00	2.68	0.32	1.00	2.00	1.00	2.00	3.00	1.00	2.00	2.65	0.35
Final Sat.:	1750	5020	579	1750	3800	1750	3150	5700	1750	3150	4967	632

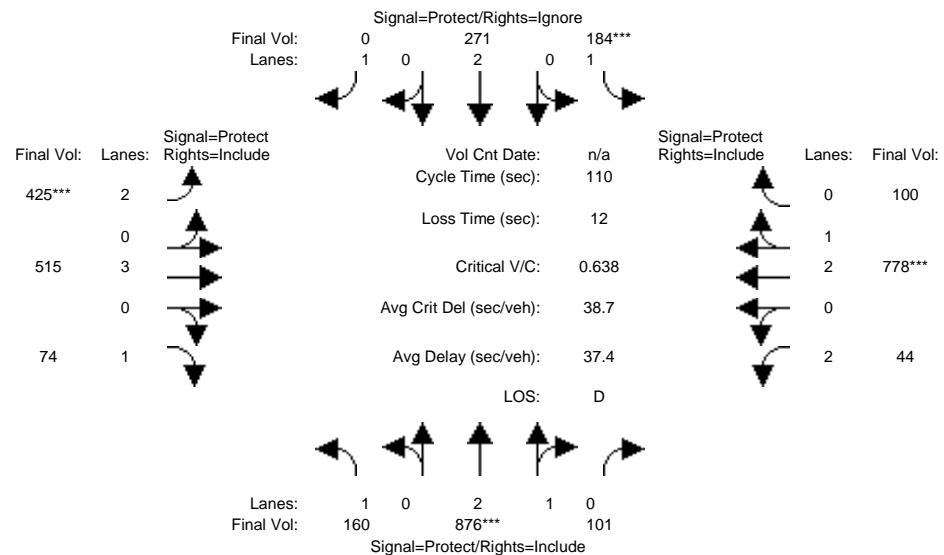
Capacity Analysis Module:

Vol/Sat:	0.09	0.17	0.17	0.11	0.07	0.00	0.13	0.09	0.04	0.01	0.16	0.16
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	24.1	30.0	30.0	18.1	24.0	0.0	22.9	29.3	29.3	20.5	27.0	27.0
Volume/Cap:	0.42	0.64	0.64	0.64	0.33	0.00	0.64	0.34	0.16	0.07	0.64	0.64
Delay/Veh:	37.6	36.1	36.1	47.6	36.4	0.0	41.9	32.7	31.1	37.0	38.2	38.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	37.6	36.1	36.1	47.6	36.4	0.0	41.9	32.7	31.1	37.0	38.2	38.2
LOS by Move:	D	D	D	D	D	A	D	C	C	D	D	D
HCM2kAvgQ:	5	11	11	6	4	0	9	5	2	1	9	9

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Ex + P AM

Intersection #24: Stevens Creek Blvd and Wolfe Rd/Miller Ave (CUP/CMP)

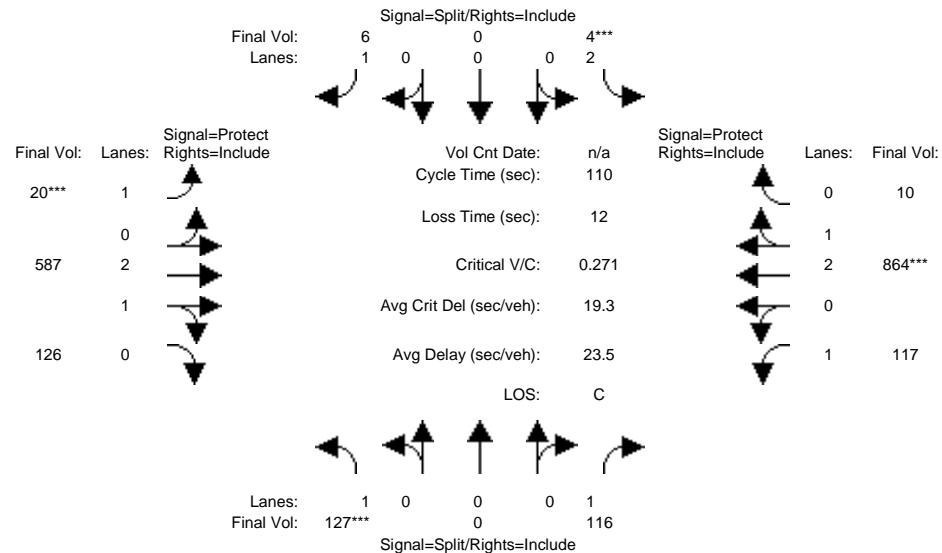


Street Name: Wolfe Rd/Miller Ave Stevens Creek Blvd															
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	7	10	10			
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0			
Volume Module:															
Base Vol:	160	875	101	184	270	468	418	515	74	44	778	99			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	160	875	101	184	270	468	418	515	74	44	778	99			
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Hyatt:	0	1	0	0	1	5	7	0	0	0	0	1			
Initial Fut:	160	876	101	184	271	473	425	515	74	44	778	100			
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Volume:	160	876	101	184	271	0	425	515	74	44	778	100			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	160	876	101	184	271	0	425	515	74	44	778	100			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
FinalVolume:	160	876	101	184	271	0	425	515	74	44	778	100			
Saturation Flow Module:															
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900			
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92	0.83	1.00	0.92			
Lanes:	1.00	2.67	0.33	1.00	2.00	1.00	2.00	3.00	1.00	2.00	2.63	0.37			
Final Sat.:	1750	5066	584	1750	3800	1750	3150	5700	1750	3150	5002	643			
Capacity Analysis Module:															
Vol/Sat:	0.09	0.17	0.17	0.11	0.07	0.00	0.13	0.09	0.04	0.01	0.16	0.16			
Crit Moves:	****	****	****	****	****		****	****		****	****				
Green Time:	24.0	29.8	29.8	18.1	23.9	0.0	23.3	29.5	29.5	20.6	26.8	26.8			
Volume/Cap:	0.42	0.64	0.64	0.64	0.33	0.00	0.64	0.34	0.16	0.07	0.64	0.64			
Delay/Veh:	37.7	36.3	36.3	47.6	36.5	0.0	41.6	32.6	31.0	36.9	38.3	38.3			
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
AdjDel/Veh:	37.7	36.3	36.3	47.6	36.5	0.0	41.6	32.6	31.0	36.9	38.3	38.3			
LOS by Move:	D	D	D	D	D	A	D	C	C	D	D	D			
HCM2kAvgQ:	5	10	10	6	4	0	9	5	2	1	9	9			

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Existing AM

Intersection #26: Stevens Creek Blvd and Finch Avenue (CUP)



Street Name:	Finch Avenue				Stevens Creek Blvd										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	10	10	10	10	10	10	7	10	10	10	7	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	127	0	116	4	0	6	20	587	126	117	864	10
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	127	0	116	4	0	6	20	587	126	117	864	10
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	127	0	116	4	0	6	20	587	126	117	864	10
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	127	0	116	4	0	6	20	587	126	117	864	10
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	127	0	116	4	0	6	20	587	126	117	864	10
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	127	0	116	4	0	6	20	587	126	117	864	10

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	0.99	0.95	0.92	0.98	0.95
Lanes:	1.00	0.00	1.00	2.00	0.00	1.00	1.00	2.45	0.55	1.00	2.96	0.04
Final Sat.:	1750	0	1750	3150	0	1750	1750	4609	989	1750	5536	64

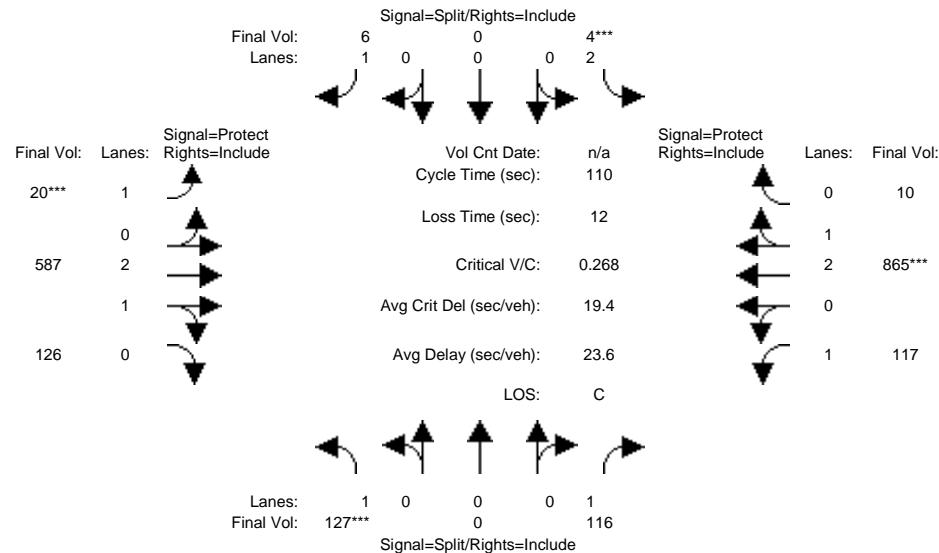
Capacity Analysis Module:

Vol/Sat:	0.07	0.00	0.07	0.00	0.00	0.00	0.01	0.13	0.13	0.07	0.16	0.16
Crit Moves:	****		****		****		****		****		****	
Green Time:	25.7	0.0	25.7	10.0	0.0	10.0	7.0	40.8	40.8	21.4	55.3	55.3
Volume/Cap:	0.31	0.00	0.28	0.01	0.00	0.04	0.18	0.34	0.34	0.34	0.31	0.31
Delay/Veh:	35.3	0.0	35.0	45.5	0.0	45.7	49.6	25.0	25.0	38.8	16.2	16.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	35.3	0.0	35.0	45.5	0.0	45.7	49.6	25.0	25.0	38.8	16.2	16.2
LOS by Move:	D	A	C	D	A	D	D	C	C	D	B	B
HCM2kAvgQ:	4	0	4	0	0	0	1	6	6	4	6	6

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Ex + P AM

Intersection #26: Stevens Creek Blvd and Finch Avenue (CUP)



Street Name:	Finch Avenue				Stevens Creek Blvd										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	10	10	10	10	10	10	7	10	10	10	7	10	10		
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		

Volume Module:

Base Vol:	127	0	116	4	0	6	20	587	126	117	864	10			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	127	0	116	4	0	6	20	587	126	117	864	10			
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Hyatt:	0	0	0	0	0	0	0	0	0	0	0	1			
Initial Fut:	127	0	116	4	0	6	20	587	126	117	865	10			
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Volume:	127	0	116	4	0	6	20	587	126	117	865	10			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	127	0	116	4	0	6	20	587	126	117	865	10			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
FinalVolume:	127	0	116	4	0	6	20	587	126	117	865	10			

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900			
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92			
Lanes:	1.00	0.00	1.00	2.00	0.00	1.00	1.00	2.43	0.57	1.00	2.96	0.04			
Final Sat.:	1750	0	1750	3150	0	1750	1750	4623	992	1750	5629	65			

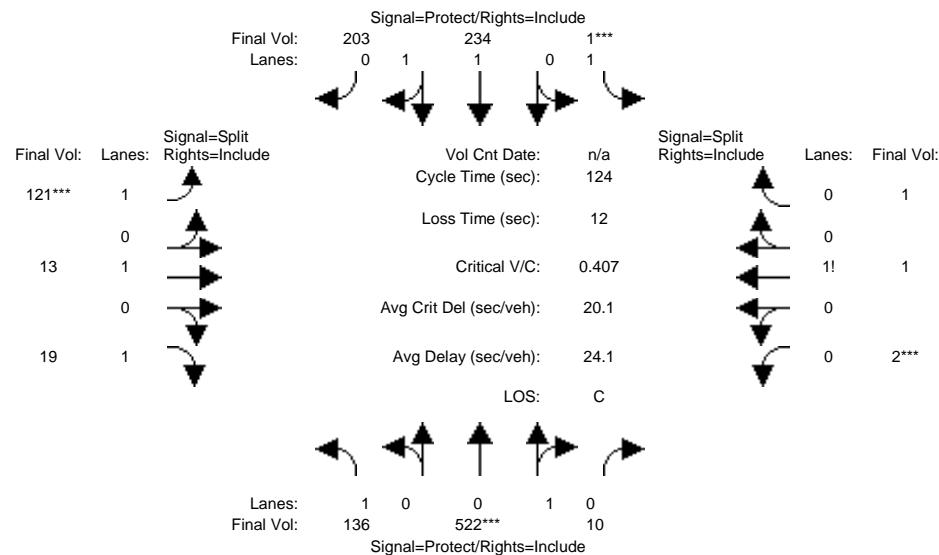
Capacity Analysis Module:

Vol/Sat:	0.07	0.00	0.07	0.00	0.00	0.00	0.01	0.13	0.13	0.07	0.15	0.15			
Crit Moves:	****		****		****		****		****		****				
Green Time:	26.0	0.0	26.0	10.0	0.0	10.0	7.0	40.6	40.6	21.4	55.0	55.0			
Volume/Cap:	0.31	0.00	0.28	0.01	0.00	0.04	0.18	0.34	0.34	0.34	0.31	0.31			
Delay/Veh:	35.0	0.0	34.7	45.5	0.0	45.7	49.6	25.2	25.2	38.9	16.3	16.3			
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
AdjDel/Veh:	35.0	0.0	34.7	45.5	0.0	45.7	49.6	25.2	25.2	38.9	16.3	16.3			
LOS by Move:	D	A	C	D	A	D	D	C	C	D	B	B			
HCM2kAvgQ:	4	0	4	0	0	0	1	6	6	4	6	6			

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Existing AM

Intersection #31: Tantau Avenue and Vallco Parkway(CUP)



Street Name:	Tantau Avenue				Vallco Parkway										
	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	10	10	10	10	10	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	136	522	10	1	234	203	121	13	19	2	1	1
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	136	522	10	1	234	203	121	13	19	2	1	1
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	136	522	10	1	234	203	121	13	19	2	1	1
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	136	522	10	1	234	203	121	13	19	2	1	1
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	136	522	10	1	234	203	121	13	19	2	1	1
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	136	522	10	1	234	203	121	13	19	2	1	1

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.95	0.95	0.92	1.00	0.95	0.92	1.00	0.92	0.92	0.92	0.92
Lanes:	1.00	0.98	0.02	1.00	1.05	0.95	1.00	1.00	1.00	0.50	0.25	0.25
Final Sat.:	1750	1766	34	1750	1980	1718	1750	1900	1750	875	438	438

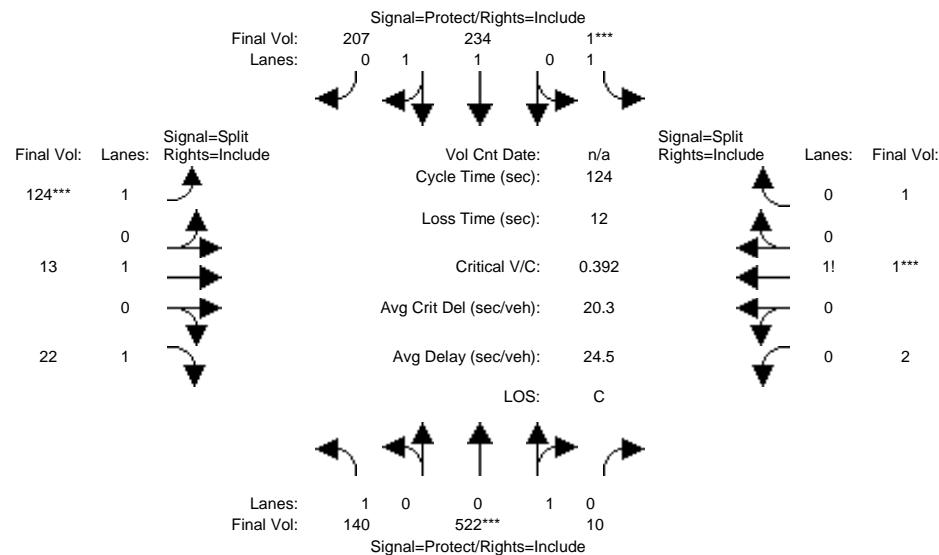
Capacity Analysis Module:

Vol/Sat:	0.08	0.30	0.30	0.00	0.12	0.12	0.07	0.01	0.01	0.00	0.00	0.00
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	33.3	77.0	77.0	7.0	50.7	50.7	18.0	18.0	18.0	10.0	10.0	10.0
Volume/Cap:	0.29	0.48	0.48	0.01	0.29	0.29	0.48	0.05	0.07	0.03	0.03	0.03
Delay/Veh:	36.3	13.0	13.0	55.3	24.7	24.7	50.1	45.7	45.9	52.6	52.6	52.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	36.3	13.0	13.0	55.3	24.7	24.7	50.1	45.7	45.9	52.6	52.6	52.6
LOS by Move:	D	B	B	E	C	C	D	D	D	D	D	D
HCM2kAvgQ:	4	11	11	0	5	5	5	0	1	0	0	0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Ex + P AM

Intersection #31: Tantau Avenue and Vallco Parkway(CUP)

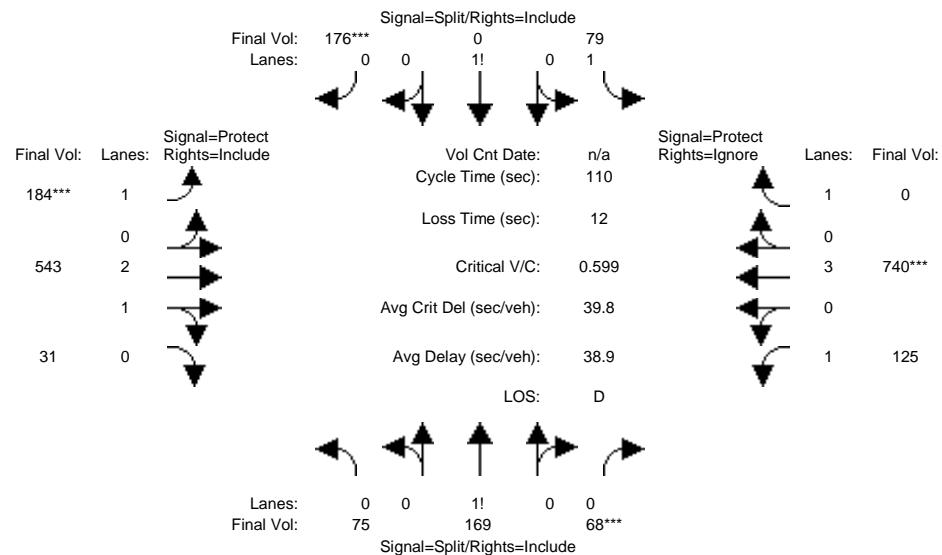


Street Name: Tantau Avenue Vallco Parkway															
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7 10		10 7		10 10		10 10		10 10		10 10		10 10		
Y+R:	4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		
Volume Module:	<hr/>														
Base Vol:	136	522	10	1	234	203	121	13	19	2	1	1			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	136	522	10	1	234	203	121	13	19	2	1	1			
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Hyatt:	4	0	0	0	0	4	3	0	3	0	0	0			
Initial Fut:	140	522	10	1	234	207	124	13	22	2	1	1			
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Volume:	140	522	10	1	234	207	124	13	22	2	1	1			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	140	522	10	1	234	207	124	13	22	2	1	1			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
FinalVolume:	140	522	10	1	234	207	124	13	22	2	1	1			
Saturation Flow Module:	<hr/>														
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900			
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92			
Lanes:	1.00	0.98	0.02	1.00	1.02	0.98	1.00	1.00	1.00	0.51	0.23	0.26			
Final Sat.:	1750	1861	36	1750	1938	1715	1750	1900	1750	893	446	446			
Capacity Analysis Module:	<hr/>														
Vol/Sat:	0.08	0.28	0.28	0.00	0.12	0.12	0.07	0.01	0.01	0.00	0.00	0.00			
Crit Moves:	****			****			****			****					
Green Time:	33.0	75.8	75.8	7.0	49.8	49.8	19.2	19.2	19.2	10.0	10.0	10.0			
Volume/Cap:	0.30	0.46	0.46	0.01	0.30	0.30	0.46	0.04	0.08	0.03	0.03	0.03			
Delay/Veh:	36.6	13.3	13.3	55.3	25.3	25.3	48.9	44.7	45.0	52.6	52.6	52.6			
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
AdjDel/Veh:	36.6	13.3	13.3	55.3	25.3	25.3	48.9	44.7	45.0	52.6	52.6	52.6			
LOS by Move:	D	B	B	E	C	C	D	D	D	D	D	D			
HCM2kAvgQ:	4	11	11	0	6	6	5	0	1	0	0	0			

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Existing AM

Intersection #32: Stevens Creek Blvd and Tantau Avenue (CUP)

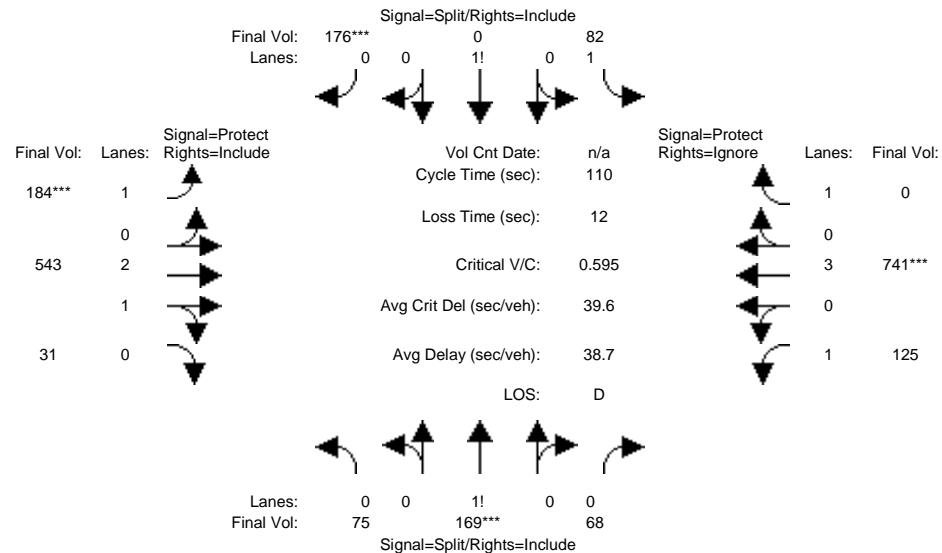


Street Name: Tantau Avenue Stevens Creek Blvd															
Approach: North Bound			South Bound			East Bound			West Bound						
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7 10		10 7		10 10		7 10		10 10		7 10		10 10		
Y+R:	4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		
Volume Module:	<hr/>														
Base Vol:	75	169	68	79	0	176	184	543	31	125	740	315			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	75	169	68	79	0	176	184	543	31	125	740	315			
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0			
Initial Fut:	75	169	68	79	0	176	184	543	31	125	740	315			
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00			
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00			
PHF Volume:	75	169	68	79	0	176	184	543	31	125	740	0			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	75	169	68	79	0	176	184	543	31	125	740	0			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00			
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00			
FinalVolume:	75	169	68	79	0	176	184	543	31	125	740	0			
Saturation Flow Module:	<hr/>														
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900			
Adjustment:	0.92	0.92	0.92	0.92	1.00	0.95	0.92	0.98	0.95	0.92	1.00	0.92			
Lanes:	0.24	0.54	0.22	1.19	0.00	0.81	1.00	2.83	0.17	1.00	3.00	1.00			
Final Sat.:	421	948	381	2078	0	1462	1750	5297	302	1750	5700	1750			
Capacity Analysis Module:	<hr/>														
Vol/Sat:	0.18	0.18	0.18	0.04	0.00	0.12	0.11	0.10	0.10	0.07	0.13	0.00			
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****			
Green Time:	32.7	32.7	32.7	22.1	0.0	22.1	19.3	25.4	25.4	17.7	23.8	0.0			
Volume/Cap:	0.60	0.60	0.60	0.19	0.00	0.60	0.60	0.44	0.44	0.44	0.60	0.00			
Delay/Veh:	34.9	34.9	34.9	36.6	0.0	42.3	45.0	36.5	36.5	42.8	39.6	0.0			
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
AdjDel/Veh:	34.9	34.9	34.9	36.6	0.0	42.3	45.0	36.5	36.5	42.8	39.6	0.0			
LOS by Move:	C	C	C	D	A	D	D	D	D	D	D	A			
HCM2kAvgQ:	10	10	10	2	0	7	6	5	5	4	7	0			

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Ex + P AM

Intersection #32: Stevens Creek Blvd and Tantau Avenue (CUP)



Street Name:	Tantau Avenue					Stevens Creek Blvd									
	North Bound			South Bound		East Bound			West Bound						
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	7	10	7	10	10	
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	

Volume Module:

Base Vol:	75	169	68	79	0	176	184	543	31	125	740	315
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	75	169	68	79	0	176	184	543	31	125	740	315
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Hyatt:	0	0	0	3	0	0	0	0	0	0	1	4
Initial Fut:	75	169	68	82	0	176	184	543	31	125	741	319
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Volume:	75	169	68	82	0	176	184	543	31	125	741	0
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	75	169	68	82	0	176	184	543	31	125	741	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
FinalVolume:	75	169	68	82	0	176	184	543	31	125	741	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	0.25	0.52	0.23	1.19	0.00	0.81	1.00	2.82	0.18	1.00	3.00	1.00
Final Sat.:	439	990	398	2081	0	1419	1750	5367	306	1750	5700	1750

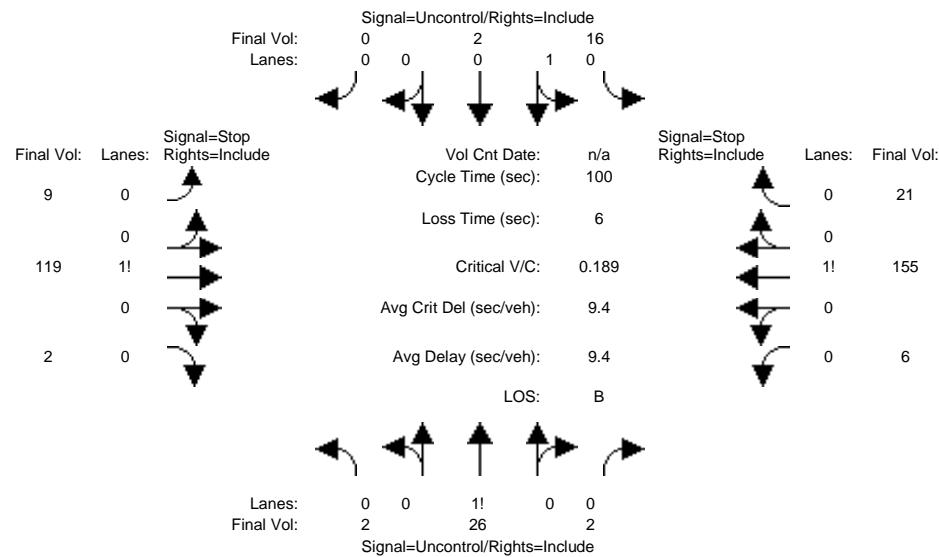
Capacity Analysis Module:

Vol/Sat:	0.17	0.17	0.17	0.04	0.00	0.12	0.11	0.10	0.10	0.07	0.13	0.00
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	31.6	31.6	31.6	22.9	0.0	22.9	19.4	25.5	25.5	18.0	24.0	0.0
Volume/Cap:	0.59	0.59	0.59	0.19	0.00	0.59	0.59	0.44	0.44	0.44	0.59	0.00
Delay/Veh:	35.6	35.6	35.6	35.9	0.0	41.6	44.8	36.3	36.3	42.5	39.4	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	35.6	35.6	35.6	35.9	0.0	41.6	44.8	36.3	36.3	42.5	39.4	0.0
LOS by Move:	D	D	D	D	A	D	D	D	D	D	D	A
HCM2kAvgQ:	10	10	10	2	0	7	6	5	5	4	7	0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Existing AM

Intersection #501: Finch Road and Vallco Pkwy (CUP)



Street Name:	Finch Road				Vallco Pkwy										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
----- ----- ----- ----- ----- ----- ----- ----- ----- ----- ----- ----- ----- ----- ----- -----															

Volume Module:

Base Vol:	2	26	2	16	2	0	9	119	2	6	155	21
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	2	26	2	16	2	0	9	119	2	6	155	21
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	2	26	2	16	2	0	9	119	2	6	155	21
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	2	26	2	16	2	0	9	119	2	6	155	21
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	2	26	2	16	2	0	9	119	2	6	155	21

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxx	4.1	xxxx	xxxxx	7.1	6.5	6.2	7.1	6.5	6.2
FollowUpTim:	2.2	xxxx	xxxxx	2.2	xxxx	xxxxx	3.5	4.0	3.3	3.5	4.0	3.3

Capacity Module:

Cnflict Vol:	2	xxxx	xxxxx	28	xxxx	xxxxx	153	66	2	126	65	27
Potent Cap.:	1634	xxxx	xxxxx	1599	xxxx	xxxxx	819	829	1088	853	830	1054
Move Cap.:	1634	xxxx	xxxxx	1599	xxxx	xxxxx	680	819	1088	750	820	1054
Volume/Cap:	0.00	xxxx	xxxx	0.01	xxxx	xxxx	0.01	0.15	0.00	0.01	0.19	0.02

Level Of Service Module:

2Way95thQ:	0.0	xxxx	xxxxx	0.0	xxxx	xxxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Control Del:	7.2	xxxx	xxxxx	7.3	xxxx	xxxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
LOS by Move:	A	*	*	A	*	*	*	*	*	*	*	*
Movement:	LT - LTR - RT											
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	811	xxxxx	xxxx	839	xxxxx	xxxx
SharedQueue:	xxxxx	xxxx	xxxxx	0.0	xxxx	xxxxx	0.6	xxxxx	xxxxx	0.8	xxxxx	xxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	7.3	xxxx	xxxxx	10.3	xxxxx	xxxxx	10.5	xxxxx	xxxx
Shared LOS:	*	*	*	A	*	*	*	B	*	*	B	*
ApproachDel:	xxxxx	xxxxxx	xxxxxx				10.3			10.5		
ApproachLOS:	*		*					B			B	

Note: Queue reported is the number of cars per lane.

Peak Hour Delay Signal Warrant Report

Intersection #501 Finch Road and Vallco Pkwy (CUP)

Future Volume Alternative: Peak Hour Warrant NOT Met

-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Lanes:	0 0 1! 0 0	0 1 0 0 0	0 0 1! 0 0	0 0 1! 0 0
Initial Vol:	2 26 2	16 2 0	9 119 2	6 155 21
ApproachDel:	xxxxxx	xxxxxx	10.3	10.5

Approach[eastbound][lanes=1][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=0.4]
 FAIL - Vehicle-hours less than 4 for one lane approach.
 Signal Warrant Rule #2: [approach volume=130]
 SUCCEED - Approach volume greater than or equal to 100 for one lane approach.
 Signal Warrant Rule #3: [approach count=4][total volume=360]
 FAIL - Total volume less than 650 for intersection
 with less than four approaches.

Approach[westbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.5] FAIL - Vehicle-hours less than 4 for one lane approach. Signal Warrant Rule #2: [approach volume=182] SUCCEED - Approach volume greater than or equal to 100 for one lane approach. Signal Warrant Rule #3: [approach count=4][total volume=360] FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #501 Finch Road and Vallco Pkwy (CUP)

Future Volume Alternative: Peak Hour Warrant NOT Met
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Lanes: 0 0 1! 0 0 0 1 0 0 0 0 0 1! 0 0 0 0 1! 0 0
Initial Vol: 2 26 2 16 2 0 9 119 2 6 155 21

Major Street Volume: 48
 Minor Approach Volume: 182
 Minor Approach Volume Threshold: 1029

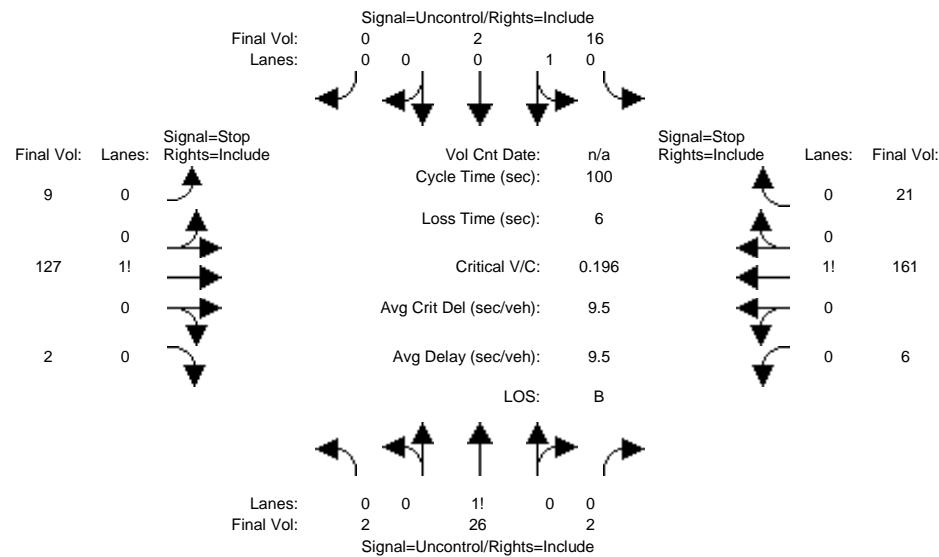
SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Ex + P AM

Intersection #501: Finch Road and Vallco Pkwy (CUP)



Street Name:	Finch Road				Vallco Pkwy										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
----- ----- ----- ----- ----- ----- ----- ----- ----- ----- ----- ----- ----- ----- ----- -----															

Volume Module:

Base Vol:	2	26	2	16	2	0	9	119	2	6	155	21
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	2	26	2	16	2	0	9	119	2	6	155	21
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Hyatt:	0	0	0	0	0	0	0	8	0	0	6	0
Initial Fut:	2	26	2	16	2	0	9	127	2	6	161	21
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	2	26	2	16	2	0	9	127	2	6	161	21
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	2	26	2	16	2	0	9	127	2	6	161	21

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxx	4.1	xxxx	xxxxx	7.1	6.5	6.2	7.1	6.5	6.2
FollowUpTim:	2.2	xxxx	xxxxx	2.2	xxxx	xxxxx	3.5	4.0	3.3	3.5	4.0	3.3

Capacity Module:

Cnflict Vol:	2	xxxx	xxxxx	28	xxxx	xxxxx	156	66	2	130	65	27
Potent Cap.:	1634	xxxx	xxxxx	1599	xxxx	xxxxx	815	829	1088	848	830	1054
Move Cap.:	1634	xxxx	xxxxx	1599	xxxx	xxxxx	672	819	1088	739	820	1054
Volume/Cap:	0.00	xxxx	xxxx	0.01	xxxx	xxxx	0.01	0.16	0.00	0.01	0.20	0.02

Level Of Service Module:

2Way95thQ:	0.0	xxxx	xxxxx	0.0	xxxx	xxxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx			
Control Del:	7.2	xxxx	xxxxx	7.3	xxxx	xxxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx			
LOS by Move:	A	*	*	A	*	*	*	*	*	*	*	*			
Movement:	LT	-	LTR	-	RT	LT	-	LTR	-	RT	LT	-	LTR	-	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	811	xxxxx	xxxx	838	xxxxx	xxxx			
SharedQueue:	xxxxx	xxxx	xxxxx	0.0	xxxx	xxxxx	0.6	xxxxx	xxxxx	0.9	xxxxx	xxxx			
Shrd ConDel:	xxxxx	xxxx	xxxxx	7.3	xxxx	xxxxx	xxxxx	10.4	xxxxx	xxxxx	10.5	xxxxx			
Shared LOS:	*	*	*	A	*	*	*	B	*	*	B	*			
ApproachDel:	xxxxx			xxxxxx				10.4			10.5				
ApproachLOS:	*			*				B			B				

Note: Queue reported is the number of cars per lane.

Peak Hour Delay Signal Warrant Report

Intersection #501 Finch Road and Vallco Pkwy (CUP)

Future Volume Alternative: Peak Hour Warrant NOT Met

-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Lanes:	0 0 1! 0 0	0 1 0 0 0	0 0 1! 0 0	0 0 1! 0 0
Initial Vol:	2 26 2	16 2 0	9 127 2	6 161 21
ApproachDel:	xxxxxx	xxxxxx	10.4	10.5

Approach[eastbound][lanes=1][control=Stop Sign]
 Signal Warrant Rule #1: [vehicle-hours=0.4]
 FAIL - Vehicle-hours less than 4 for one lane approach.
 Signal Warrant Rule #2: [approach volume=138]
 SUCCEED - Approach volume greater than or equal to 100 for one lane approach.
 Signal Warrant Rule #3: [approach count=4][total volume=374]
 FAIL - Total volume less than 650 for intersection
 with less than four approaches.

Approach[westbound][lanes=1][control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.6] FAIL - Vehicle-hours less than 4 for one lane approach. Signal Warrant Rule #2: [approach volume=188] SUCCEED - Approach volume greater than or equal to 100 for one lane approach. Signal Warrant Rule #3: [approach count=4][total volume=374] FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #501 Finch Road and Vallco Pkwy (CUP)

Future Volume Alternative: Peak Hour Warrant NOT Met
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Lanes: 0 0 1! 0 0 0 1 0 0 0 0 0 1! 0 0 0 0 1! 0 0
Initial Vol: 2 26 2 16 2 0 9 127 2 6 161 21

Major Street Volume: 48
 Minor Approach Volume: 188
 Minor Approach Volume Threshold: 1029

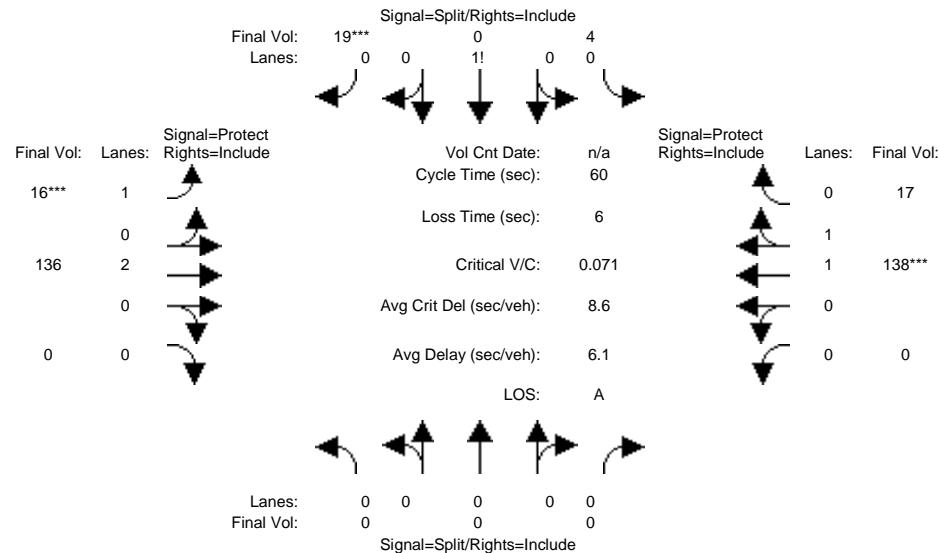
SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Existing AM

Intersection #504: Perimeter Rd & Vallco Pkwy



Street Name:	Perimeter Rd						Vallco Pkwy								
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10		
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		

Volume Module:

Base Vol:	0	0	0	4	0	19	16	136	0	0	138	17
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	4	0	19	16	136	0	0	138	17
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	4	0	19	16	136	0	0	138	17
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	4	0	19	16	136	0	0	138	17
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	4	0	19	16	136	0	0	138	17
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	4	0	19	16	136	0	0	138	17

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	0.92	0.92	0.92	1.00	0.92	0.92	0.98	0.95
Lanes:	0.00	0.00	0.00	0.17	0.00	0.83	1.00	2.00	0.00	0.00	1.77	0.23
Final Sat.:	0	0	0	304	0	1446	1750	3800	0	0	3294	406

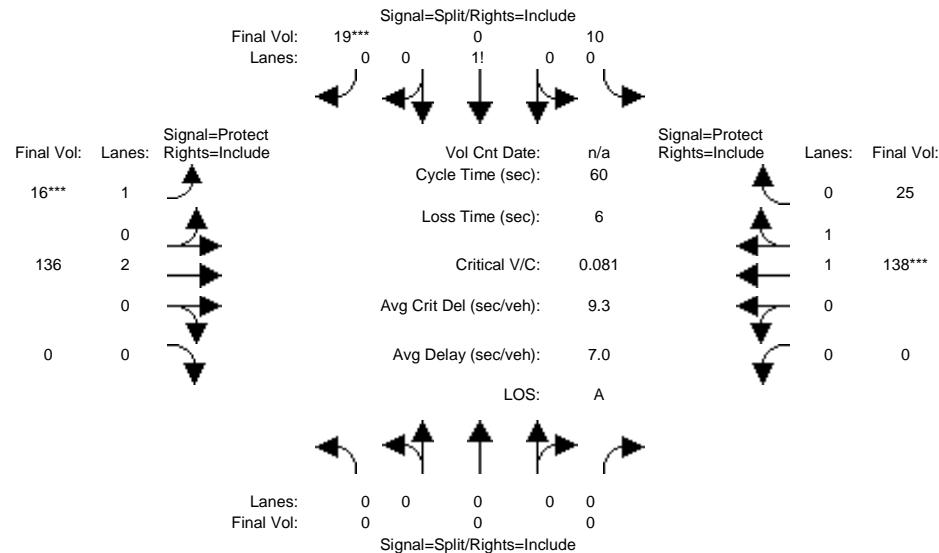
Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.01	0.00	0.01	0.01	0.04	0.00	0.00	0.04	0.04
Crit Moves:						****	****				****	
Green Time:	0.0	0.0	0.0	11.1	0.0	11.1	7.7	42.9	0.0	0.0	35.2	35.2
Volume/Cap:	0.00	0.00	0.00	0.07	0.00	0.07	0.07	0.05	0.00	0.00	0.07	0.07
Delay/Veh:	0.0	0.0	0.0	20.3	0.0	20.3	23.1	2.5	0.0	0.0	5.3	5.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	20.3	0.0	20.3	23.1	2.5	0.0	0.0	5.3	5.3
LOS by Move:	A	A	A	C	A	C	C	A	A	A	A	A
HCM2kAvgQ:	0	0	0	0	0	0	0	0	0	0	1	1

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Ex + P AM

Intersection #504: Perimeter Rd & Vallco Pkwy



Street Name:	Perimeter Rd				Vallco Pkwy										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	0	0	4	0	19	16	136	0	0	138	17			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	0	0	0	4	0	19	16	136	0	0	138	17			
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Hyatt:	0	0	0	6	0	0	0	0	0	0	0	0			8
Initial Fut:	0	0	0	10	0	19	16	136	0	0	138	25			
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Volume:	0	0	0	10	0	19	16	136	0	0	138	25			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			0
Reduced Vol:	0	0	0	10	0	19	16	136	0	0	138	25			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
FinalVolume:	0	0	0	10	0	19	16	136	0	0	138	25			

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.83	0.88	1.00	0.92	0.92	0.98	0.90
Lanes:	0.00	0.00	0.00	0.34	0.00	0.66	1.00	2.00	0.00	0.00	1.67	0.33
Final Sat.:	0	0	0	541	0	1028	1663	3800	0	0	3102	562

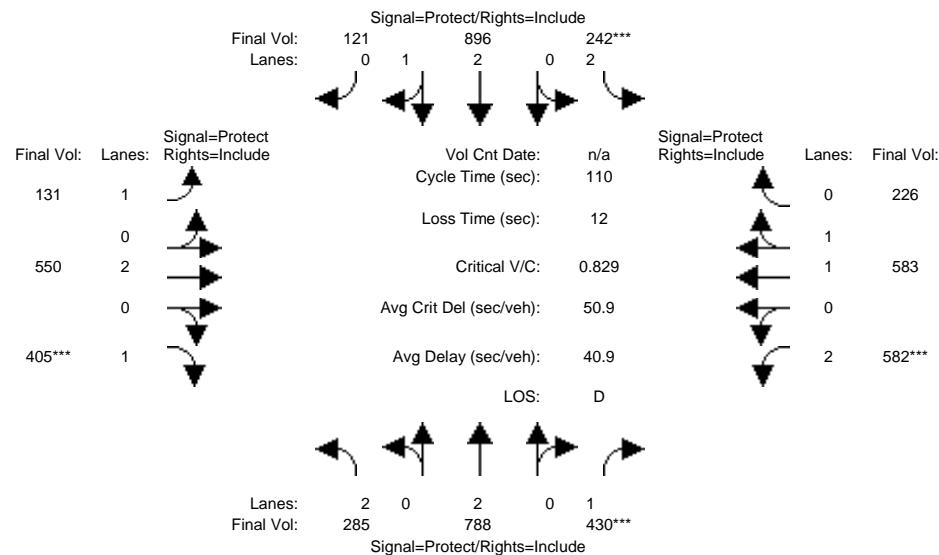
Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.02	0.00	0.02	0.01	0.04	0.00	0.00	0.04	0.04
Crit Moves:						****	****				****	
Green Time:	0.0	0.0	0.0	13.8	0.0	13.8	7.2	40.2	0.0	0.0	33.1	33.1
Volume/Cap:	0.00	0.00	0.00	0.08	0.00	0.08	0.08	0.05	0.00	0.00	0.08	0.08
Delay/Veh:	0.0	0.0	0.0	18.3	0.0	18.3	23.7	3.4	0.0	0.0	6.3	6.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	18.3	0.0	18.3	23.7	3.4	0.0	0.0	6.3	6.3
LOS by Move:	A	A	A	B	A	B	C	A	A	A	A	A
HCM2kAvgQ:	0	0	0	0	0	0	0	0	0	0	1	1

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd AM

Intersection #18: Wolfe Road and Homestead Road (CUP)



Street Name:

Wolfe Road

Homestead Road

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R

Min. Green:	7	10	10	7	10	10	7	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	264	826	193	141	774	117	126	463	181	257	545	109
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	264	826	193	141	774	117	126	463	181	257	545	109
Added Vol:	17	9	38	44	81	0	0	103	148	161	7	5
App+Div:	4	-47	199	57	41	4	5	-16	76	164	31	112
Initial Fut:	285	788	430	242	896	121	131	550	405	582	583	226
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	285	788	430	242	896	121	131	550	405	582	583	226
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	285	788	430	242	896	121	131	550	405	582	583	226
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	285	788	430	242	896	121	131	550	405	582	583	226

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.83	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92
Lanes:	2.00	2.00	1.00	2.00	2.62	0.38	1.00	2.00	1.00	2.00	1.41	0.59
Final Sat.:	3150	3800	1750	3150	4971	671	1750	3800	1750	3150	2674	1037

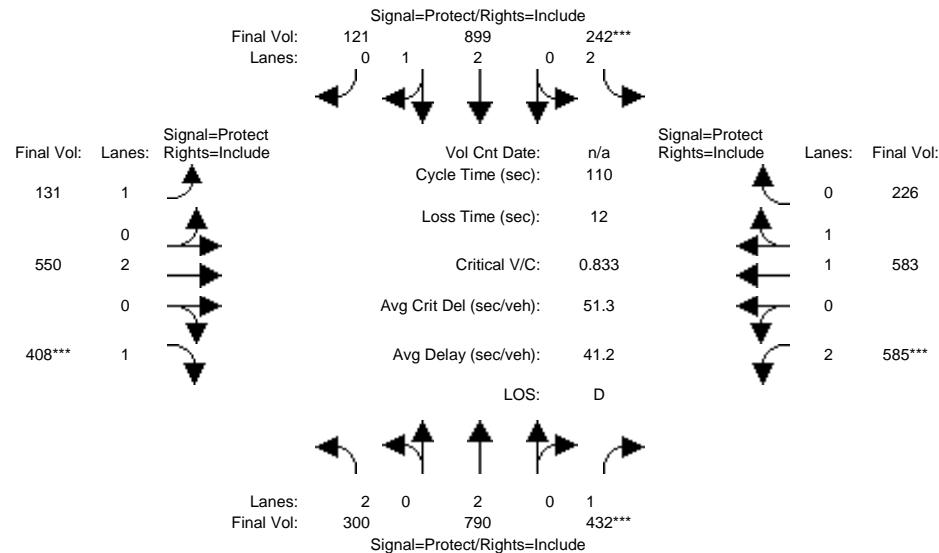
Capacity Analysis Module:

Vol/Sat:	0.09	0.21	0.25	0.08	0.18	0.18	0.07	0.14	0.23	0.18	0.22	0.22
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	14.3	32.6	32.6	10.2	28.5	28.5	14.1	30.7	30.7	24.5	41.1	41.1
Volume/Cap:	0.70	0.70	0.83	0.83	0.70	0.70	0.58	0.52	0.83	0.83	0.58	0.58
Uniform Del:	45.8	34.4	36.1	49.0	36.8	36.8	45.2	33.4	37.2	40.7	27.6	27.6
IncremntDel:	5.2	2.0	10.7	17.7	1.5	1.5	3.9	0.5	11.3	8.2	0.6	0.6
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	50.9	36.3	46.8	66.7	38.3	38.3	49.1	33.9	48.5	48.9	28.2	28.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	50.9	36.3	46.8	66.7	38.3	38.3	49.1	33.9	48.5	48.9	28.2	28.2
LOS by Move:	D	D	D	E	D	D	D	C	D	D	C	C
HCM2kAvgQ:	5	11	14	5	10	10	4	7	14	12	11	11

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + Proj AM

Intersection #18: Wolfe Road and Homestead Road (CUP)



Street Name:	Wolfe Road				Homestead Road										
	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	7	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	281	779	392	198	815	121	131	447	257	421	576	221			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	281	779	392	198	815	121	131	447	257	421	576	221			
Added Vol:	17	9	38	44	81	0	0	103	148	161	7	5			
Hyatt:	2	2	2	0	3	0	0	0	3	3	0	0			
Initial Fut:	300	790	432	242	899	121	131	550	408	585	583	226			
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Volume:	300	790	432	242	899	121	131	550	408	585	583	226			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	300	790	432	242	899	121	131	550	408	585	583	226			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
FinalVolume:	300	790	432	242	899	121	131	550	408	585	583	226			

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.83	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92
Lanes:	2.00	2.00	1.00	2.00	2.62	0.38	1.00	2.00	1.00	2.00	1.41	0.59
Final Sat.:	3150	3800	1750	3150	4973	669	1750	3800	1750	3150	2674	1037

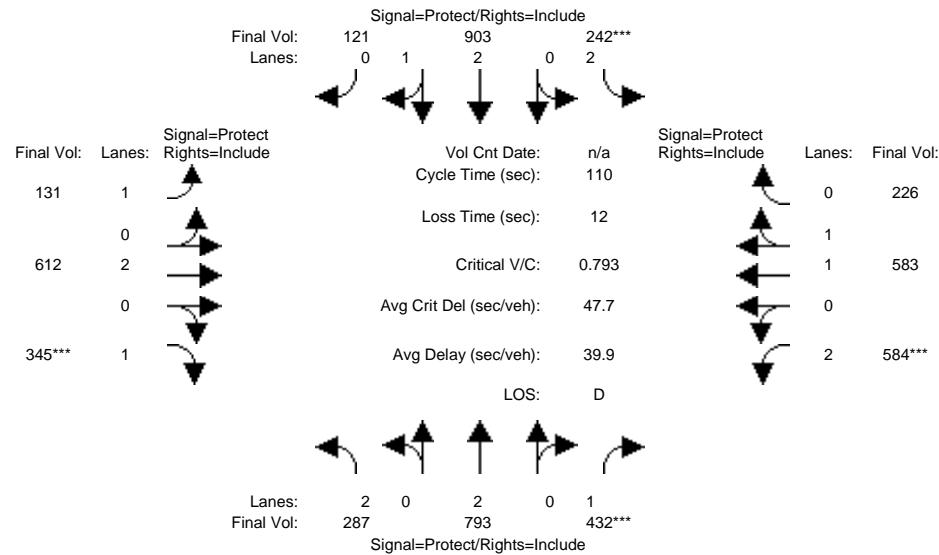
Capacity Analysis Module:

Vol/Sat:	0.10	0.21	0.25	0.08	0.18	0.18	0.07	0.14	0.23	0.19	0.22	0.22
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	14.7	32.6	32.6	10.1	28.0	28.0	14.1	30.8	30.8	24.5	41.1	41.1
Volume/Cap:	0.71	0.70	0.83	0.83	0.71	0.71	0.58	0.52	0.83	0.83	0.58	0.58
Uniform Del:	45.6	34.4	36.2	49.1	37.3	37.3	45.2	33.4	37.2	40.8	27.6	27.6
IncremntDel:	5.6	2.0	11.1	18.3	1.7	1.7	3.9	0.4	11.7	8.5	0.6	0.6
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	51.2	36.4	47.3	67.4	39.0	39.0	49.0	33.8	48.9	49.3	28.2	28.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	51.2	36.4	47.3	67.4	39.0	39.0	49.0	33.8	48.9	49.3	28.2	28.2
LOS by Move:	D	D	D	E	D	D	D	C	D	D	C	C
HCM2kAvgQ:	6	11	15	5	10	10	4	7	14	12	11	11

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative AM

Intersection #18: Wolfe Road and Homestead Road (CUP)



Street Name:

Wolfe Road

Homestead Road

Approach:	North Bound			South Bound			East Bound			West Bound				
	L	-	T	-	R	L	-	T	-	R	L	-	T	-

Min. Green:	7	10	10	7	10	10	7	10	10	7	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	264	826	193	141	774	117	126	463	181	257	545	109
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	264	826	193	141	774	117	126	463	181	257	545	109
Added Vol:	17	9	38	44	81	0	0	103	148	161	7	5
A+P+D:	6	-42	201	57	48	4	5	46	16	166	31	112
Initial Fut:	287	793	432	242	903	121	131	612	345	584	583	226
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	287	793	432	242	903	121	131	612	345	584	583	226
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	287	793	432	242	903	121	131	612	345	584	583	226
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	287	793	432	242	903	121	131	612	345	584	583	226

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.83	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92
Lanes:	2.00	2.00	1.00	2.00	2.62	0.38	1.00	2.00	1.00	2.00	1.41	0.59
Final Sat.:	3150	3800	1750	3150	4976	667	1750	3800	1750	3150	2674	1037

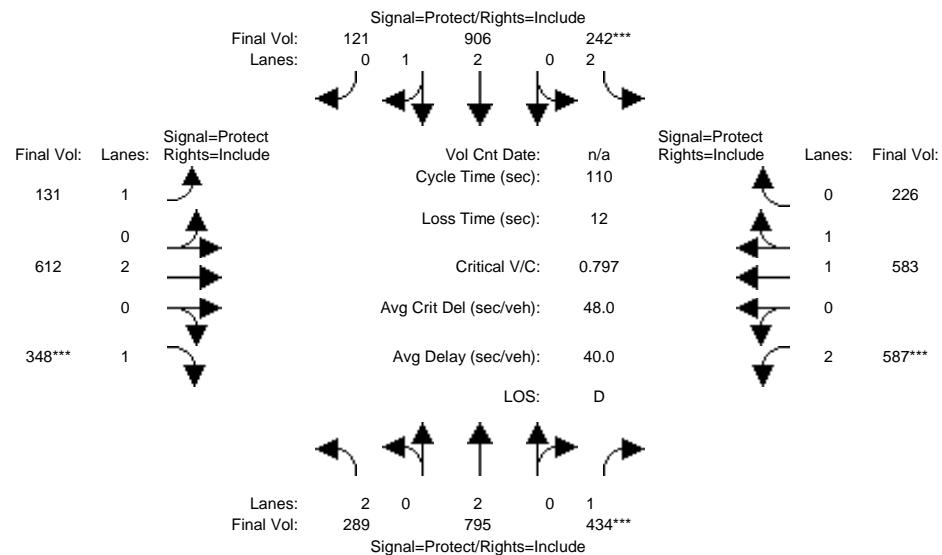
Capacity Analysis Module:

Vol/Sat:	0.09	0.21	0.25	0.08	0.18	0.18	0.07	0.16	0.20	0.19	0.22	0.22
Crit Moves:	****	****					****	****				
Green Time:	15.0	34.3	34.3	10.7	29.9	29.9	13.6	27.4	27.4	25.7	39.5	39.5
Volume/Cap:	0.67	0.67	0.79	0.79	0.67	0.67	0.61	0.65	0.79	0.79	0.61	0.61
Delay/Veh:	49.1	34.5	42.4	61.8	36.8	36.8	50.6	38.6	48.3	45.5	29.7	29.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	49.1	34.5	42.4	61.8	36.8	36.8	50.6	38.6	48.3	45.5	29.7	29.7
LOS by Move:	D	C	D	E	D	D	D	D	D	D	C	C
HCM2kAvgQ:	5	11	14	5	10	10	4	9	12	11	11	11

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + Proj AM

Intersection #18: Wolfe Road and Homestead Road (CUP)



Street Name:	Wolfe Road						Homestead Road								
	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	10	
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	

Volume Module:

Base Vol:	270	784	394	198	822	121	131	509	197	423	576	221
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	270	784	394	198	822	121	131	509	197	423	576	221
Added Vol:	17	9	38	44	81	0	0	103	148	161	7	5
Hyatt:	2	2	2	0	3	0	0	0	3	3	0	0
Initial Fut:	289	795	434	242	906	121	131	612	348	587	583	226
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	289	795	434	242	906	121	131	612	348	587	583	226
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	289	795	434	242	906	121	131	612	348	587	583	226
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	289	795	434	242	906	121	131	612	348	587	583	226

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.83	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92
Lanes:	2.00	2.00	1.00	2.00	2.62	0.38	1.00	2.00	1.00	2.00	1.41	0.59
Final Sat.:	3150	3800	1750	3150	4978	665	1750	3800	1750	3150	2674	1037

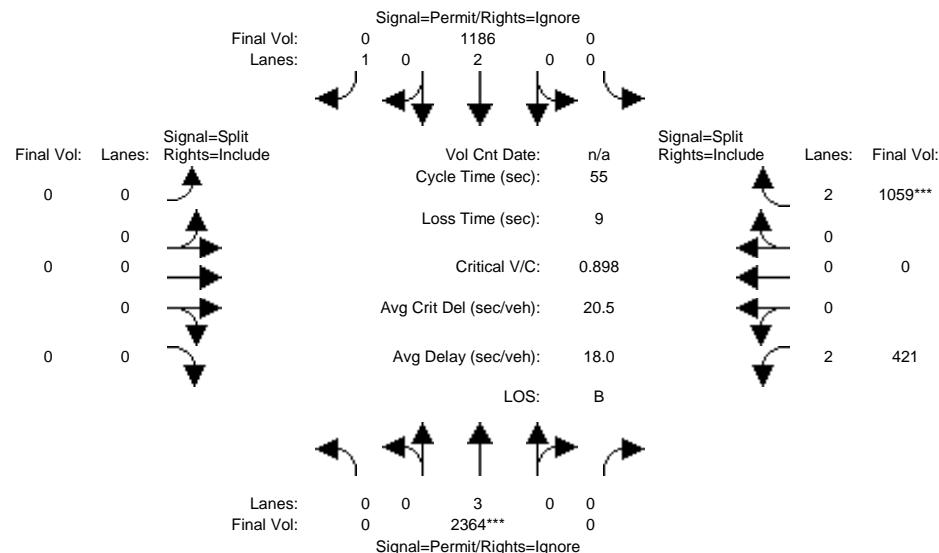
Capacity Analysis Module:

Vol/Sat:	0.09	0.21	0.25	0.08	0.18	0.18	0.07	0.16	0.20	0.19	0.22	0.22
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	15.0	34.2	34.2	10.6	29.8	29.8	13.6	27.4	27.4	25.7	39.6	39.6
Volume/Cap:	0.67	0.67	0.80	0.80	0.67	0.67	0.61	0.65	0.80	0.80	0.61	0.61
Delay/Veh:	49.3	34.5	42.8	62.3	36.9	36.9	50.5	38.5	48.5	45.8	29.6	29.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	49.3	34.5	42.8	62.3	36.9	36.9	50.5	38.5	48.5	45.8	29.6	29.6
LOS by Move:	D	C	D	E	D	D	D	D	D	D	C	C
HCM2kAvgQ:	5	11	14	5	10	10	4	9	12	11	11	11

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd AM

Intersection #21: Wolfe Road and I-280 NB Ramps (CUP/CMP)



Street Name:		Wolfe Road						I-280 NB Ramps					
Approach:	North Bound	South Bound			East Bound			West Bound					
Movement:	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R
Min. Green:	7	10	10	7	10	10	0	0	0	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	1554	364	0	1102	482	0	0	0	415	0	683
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1554	364	0	1102	482	0	0	0	415	0	683
Added Vol:	0	810	48	0	84	86	0	0	0	6	0	376
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	2364	412	0	1186	568	0	0	0	421	0	1059
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	2364	0	0	1186	0	0	0	0	421	0	1059
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	2364	0	0	1186	0	0	0	0	421	0	1059
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	2364	0	0	1186	0	0	0	0	421	0	1059

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.83
Lanes:	0.00	3.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	2.00	0.00	2.00
Final Sat.:	0	5700	0	0	3800	1750	0	0	0	3150	0	3150

Capacity Analysis Module:

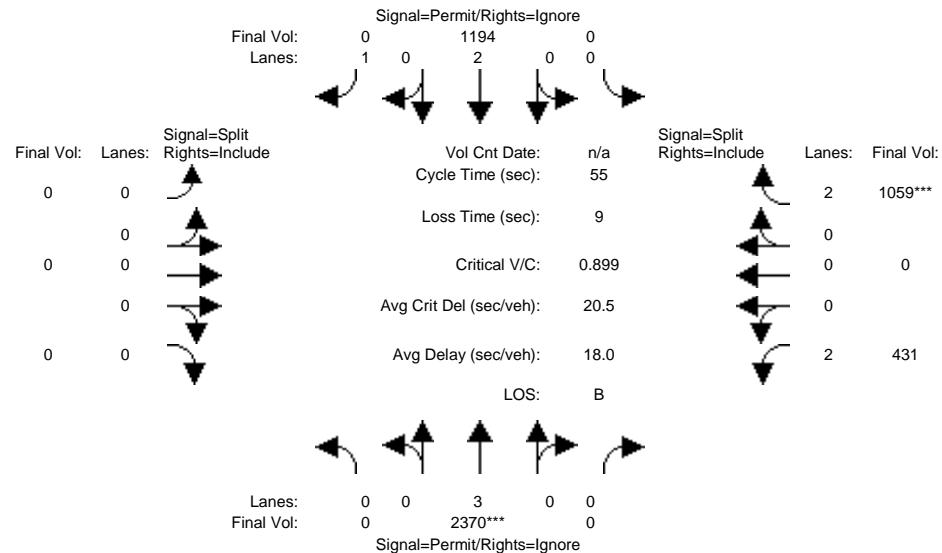
Vol/Sat:	0.00	0.41	0.00	0.00	0.31	0.00	0.00	0.00	0.00	0.13	0.00	0.34
Crit Moves:	****											****
Green Time:	0.0	25.4	0.0	0.0	25.4	0.0	0.0	0.0	0.0	20.6	0.0	20.6
Volume/Cap:	0.00	0.90	0.00	0.00	0.68	0.00	0.00	0.00	0.00	0.36	0.00	0.90
Uniform Del:	0.0	13.6	0.0	0.0	11.6	0.0	0.0	0.0	0.0	12.4	0.0	16.2
IncremntDel:	0.0	4.6	0.0	0.0	1.1	0.0	0.0	0.0	0.0	0.2	0.0	9.3
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	0.00	1.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	0.00	1.00
Delay/Veh:	0.0	18.2	0.0	0.0	12.6	0.0	0.0	0.0	0.0	12.6	0.0	25.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	18.2	0.0	0.0	12.6	0.0	0.0	0.0	0.0	12.6	0.0	25.5
LOS by Move:	A	B	A	A	B	A	A	A	A	B	A	C
HCM2kAvgQ:	0	12	0	0	8	0	0	0	0	3	0	15

Note: Queue reported is the number of cars per lane.

Hyatt House Hotel TIA
Hexagon Transportation Consultants, Inc.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + Proj AM

Intersection #21: Wolfe Road and I-280 NB Ramps (CUP/CMP)



Volume Module:

Base Vol:	0	1554	364	0	1102	482	0	0	0	415	0	683
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1554	364	0	1102	482	0	0	0	415	0	683
Added Vol:	0	810	48	0	84	86	0	0	0	6	0	376
Hyatt:	0	6	0	0	8	0	0	0	0	10	0	0
Initial Fut:	0	2370	412	0	1194	568	0	0	0	431	0	1059
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	2370	0	0	1194	0	0	0	0	431	0	1059
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	2370	0	0	1194	0	0	0	0	431	0	1059
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	2370	0	0	1194	0	0	0	0	431	0	1059

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Saturation Flow Module:
Sat/Lane:    1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900
Adjustment:   0.92 1.00 0.92 0.92 1.00 0.92 0.92 1.00 0.92 0.92 0.83 1.00 0.83
Lanes:        0.00 3.00 0.00 0.00 2.00 1.00 0.00 0.00 0.00 2.00 0.00 2.00
Final Sat.:   0 5700 0 3800 1750 0 0 0 3150 0 3150

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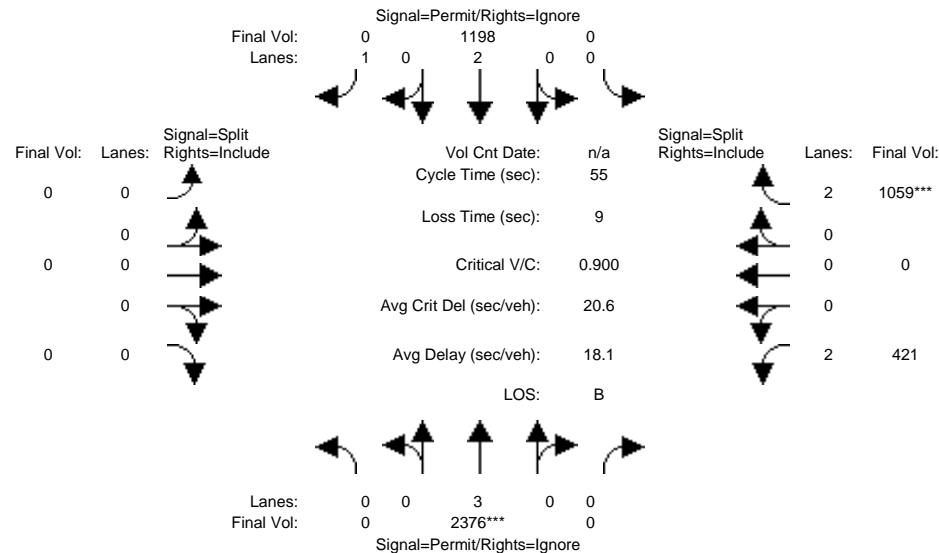
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Capacity Analysis Module:												
Vol/Sat:	0.00	0.42	0.00	0.00	0.31	0.00	0.00	0.00	0.00	0.14	0.00	0.34
Crit Moves:	****									****		
Green Time:	0.0	25.4	0.0	0.0	25.4	0.0	0.0	0.0	0.0	20.6	0.0	20.6
Volume/Cap:	0.00	0.90	0.00	0.00	0.68	0.00	0.00	0.00	0.00	0.37	0.00	0.90
Uniform Del:	0.0	13.6	0.0	0.0	11.6	0.0	0.0	0.0	0.0	12.5	0.0	16.2
IncremntDel:	0.0	4.6	0.0	0.0	1.1	0.0	0.0	0.0	0.0	0.2	0.0	9.5
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	0.00	1.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	0.00	1.00
Delay/Veh:	0.0	18.2	0.0	0.0	12.7	0.0	0.0	0.0	0.0	12.7	0.0	25.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	18.2	0.0	0.0	12.7	0.0	0.0	0.0	0.0	12.7	0.0	25.7
LOS by Move:	A	B	A	A	B	A	A	A	A	B	A	C
HCM2kAvgQ:	0	12	0	0	8	0	0	0	0	3	0	15

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative AM

Intersection #21: Wolfe Road and I-280 NB Ramps (CUP/CMP)



Street Name:		Wolfe Road												I-280 NB Ramps												
Approach:		North Bound				South Bound				East Bound				West Bound												
Movement:		L	-	T	-	R	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:		7	10	10	7	10	10	0	0	0	0	0	10	10	10	10										
Y+R:		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0										

Volume Module:

Base Vol:	0	1566	426	0	1114	482	0	0	0	0	415	0	683
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1566	426	0	1114	482	0	0	0	0	415	0	683
Added Vol:	0	810	48	0	84	86	0	0	0	0	6	0	376
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	2376	474	0	1198	568	0	0	0	0	421	0	1059
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	2376	0	0	1198	0	0	0	0	0	421	0	1059
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	2376	0	0	1198	0	0	0	0	0	421	0	1059
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	2376	0	0	1198	0	0	0	0	0	421	0	1059

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.83	
Lanes:	0.00	3.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	2.00	0.00	2.00	
Final Sat.:	0	5700	0	0	3800	1750	0	0	0	3150	0	3150	

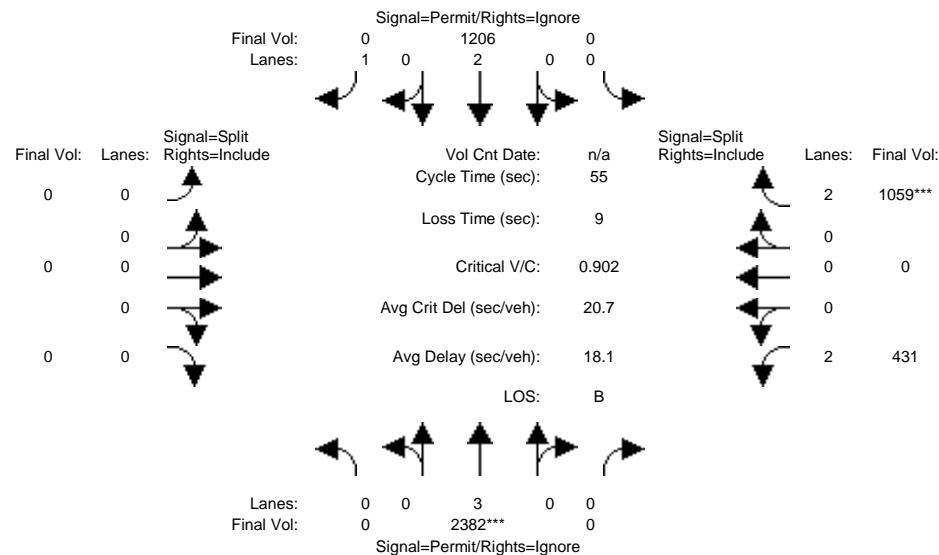
Capacity Analysis Module:

Vol/Sat:	0.00	0.42	0.00	0.00	0.32	0.00	0.00	0.00	0.00	0.13	0.00	0.34
Crit Moves:	****											****
Green Time:	0.0	25.5	0.0	0.0	25.5	0.0	0.0	0.0	0.0	20.5	0.0	20.5
Volume/Cap:	0.00	0.90	0.00	0.00	0.68	0.00	0.00	0.00	0.00	0.36	0.00	0.90
Delay/Veh:	0.0	18.3	0.0	0.0	12.7	0.0	0.0	0.0	0.0	12.7	0.0	25.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	18.3	0.0	0.0	12.7	0.0	0.0	0.0	0.0	12.7	0.0	25.8
LOS by Move:	A	B	A	A	B	A	A	A	A	B	A	C
HCM2kAvgQ:	0	12	0	0	8	0	0	0	0	3	0	15

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + Proj AM

Intersection #21: Wolfe Road and I-280 NB Ramps (CUP/CMP)



Street Name:		Wolfe Road						I-280 NB Ramps					
Approach:	North Bound	South Bound			East Bound			West Bound					
Movement:	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R
Min. Green:	7	10	10	7	10	10	0	0	0	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	1566	426	0	1114	482	0	0	0	415	0	683
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1566	426	0	1114	482	0	0	0	415	0	683
Added Vol:	0	810	48	0	84	86	0	0	0	6	0	376
Hyatt:	0	6	0	0	8	0	0	0	0	10	0	0
Initial Fut:	0	2382	474	0	1206	568	0	0	0	431	0	1059
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	2382	0	0	1206	0	0	0	0	431	0	1059
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	2382	0	0	1206	0	0	0	0	431	0	1059
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	2382	0	0	1206	0	0	0	0	431	0	1059

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.83
Lanes:	0.00	3.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	2.00	0.00	2.00
Final Sat.:	0	5700	0	0	3800	1750	0	0	0	3150	0	3150

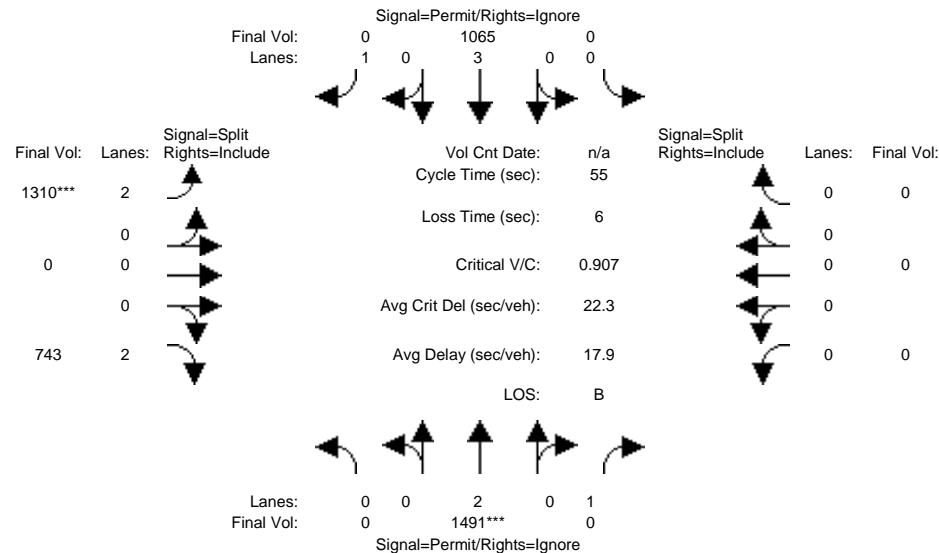
Capacity Analysis Module:

Vol/Sat:	0.00	0.42	0.00	0.00	0.32	0.00	0.00	0.00	0.00	0.14	0.00	0.34
Crit Moves:	****									****		
Green Time:	0.0	25.5	0.0	0.0	25.5	0.0	0.0	0.0	0.0	20.5	0.0	20.5
Volume/Cap:	0.00	0.90	0.00	0.00	0.68	0.00	0.00	0.00	0.00	0.37	0.00	0.90
Delay/Veh:	0.0	18.3	0.0	0.0	12.7	0.0	0.0	0.0	0.0	12.7	0.0	26.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	18.3	0.0	0.0	12.7	0.0	0.0	0.0	0.0	12.7	0.0	26.0
LOS by Move:	A	B	A	A	B	A	A	A	A	B	A	C
HCM2kAvgQ:	0	12	0	0	8	0	0	0	0	3	0	15

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd AM

Intersection #22: Wolfe Road and I-280 SB Ramps (CUP/CMP)



Street Name:		Wolfe Road												I-280 SB Ramps												
Approach:		North Bound				South Bound				East Bound				West Bound												
Movement:		L	-	T	-	R	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:		7	10	10	7	10	10	10	10	10	10	10	10	10	10	10	0	0	0	0	0					
Y+R:		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0					

Volume Module:

Base Vol:	0	1217	467	0	1036	485	726	0	477	0	0	0	0														
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00														
Initial Bse:	0	1217	467	0	1036	485	726	0	477	0	0	0	0														
Added Vol:	0	274	6	0	29	61	584	0	266	0	0	0	0														
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0	0														
Initial Fut:	0	1491	473	0	1065	546	1310	0	743	0	0	0	0														
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00														
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00														
PHF Volume:	0	1491	0	0	1065	0	1310	0	743	0	0	0	0														
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0														
Reduced Vol:	0	1491	0	0	1065	0	1310	0	743	0	0	0	0														
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00														
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00														
FinalVolume:	0	1491	0	0	1065	0	1310	0	743	0	0	0	0														

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900													
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.83	0.92	1.00	0.92	0.92													
Lanes:	0.00	2.00	1.00	0.00	3.00	1.00	2.00	0.00	2.00	0.00	2.00	0.00	2.00													
Final Sat.:	0	3800	1750	0	5700	1750	3150	0	3150	0	3150	0	3150	0												

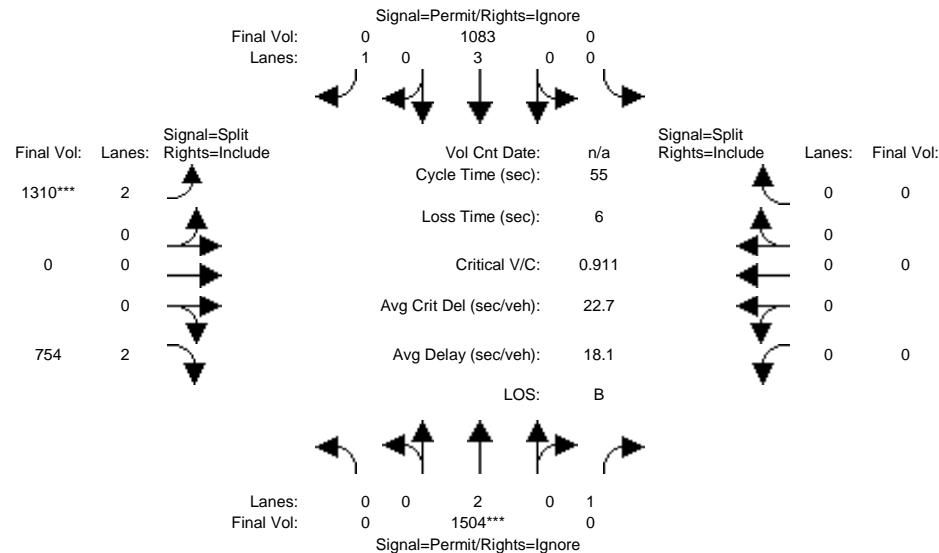
Capacity Analysis Module:

Vol/Sat:	0.00	0.39	0.00	0.00	0.19	0.00	0.42	0.00	0.24	0.00	0.00	0.00	0.00													
Crit Moves:	****						****																			
Green Time:	0.0	23.8	0.0	0.0	23.8	0.0	25.2	0.0	25.2	0.0	0.0	0.0	0.0													
Volume/Cap:	0.00	0.91	0.00	0.00	0.43	0.00	0.91	0.00	0.51	0.00	0.00	0.00	0.00													
Uniform Del:	0.0	14.6	0.0	0.0	10.9	0.0	13.8	0.0	10.6	0.0	0.0	0.0	0.0													
IncremntDel:	0.0	7.7	0.0	0.0	0.1	0.0	8.6	0.0	0.3	0.0	0.0	0.0	0.0													
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0													
Delay Adj:	0.00	1.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00	0.00	0.00													
Delay/Veh:	0.0	22.3	0.0	0.0	11.0	0.0	22.4	0.0	10.9	0.0	0.0	0.0	0.0													
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00													
AdjDel/Veh:	0.0	22.3	0.0	0.0	11.0	0.0	22.4	0.0	10.9	0.0	0.0	0.0	0.0													
LOS by Move:	A	C	A	A	B	A	C	A	B	A	A	A	A													
HCM2kAvgQ:	0	13	0	0	4	0	17	0	6	0	0	0	0													

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + Proj AM

Intersection #22: Wolfe Road and I-280 SB Ramps (CUP/CMP)



Street Name:		Wolfe Road					I-280 SB Ramps				
		North Bound			South Bound		East Bound			West Bound	
Movement:	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R
Min. Green:	7	10	10	7	10	10	10	10	10	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	1217	467	0	1036	485	726	0	477	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1217	467	0	1036	485	726	0	477	0	0	0
Added Vol:	0	274	6	0	29	61	584	0	266	0	0	0
Hyatt:	0	13	7	0	18	0	0	0	11	0	0	0
Initial Fut:	0	1504	480	0	1083	546	1310	0	754	0	0	0
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1504	0	0	1083	0	1310	0	754	0	0	0
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1504	0	0	1083	0	1310	0	754	0	0	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1504	0	0	1083	0	1310	0	754	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.83	0.92	1.00
Lanes:	0.00	2.00	1.00	0.00	3.00	1.00	2.00	0.00	2.00	0.00	0.00
Final Sat.:	0	3800	1750	0	5700	1750	3150	0	3150	0	0

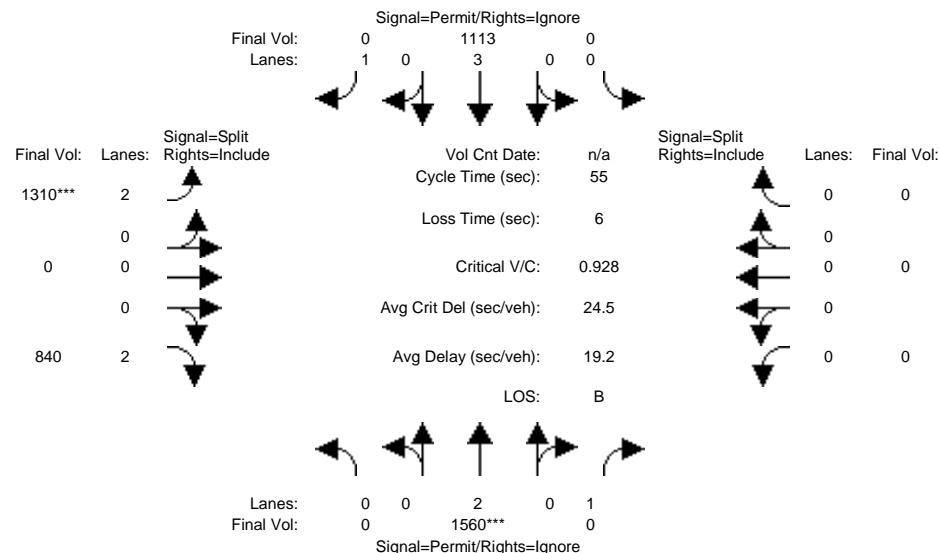
Capacity Analysis Module:

Vol/Sat:	0.00	0.40	0.00	0.00	0.19	0.00	0.42	0.00	0.24	0.00	0.00	0.00
Crit Moves:	****						****					
Green Time:	0.0	23.9	0.0	0.0	23.9	0.0	25.1	0.0	25.1	0.0	0.0	0.0
Volume/Cap:	0.00	0.91	0.00	0.00	0.44	0.00	0.91	0.00	0.52	0.00	0.00	0.00
Uniform Del:	0.0	14.6	0.0	0.0	10.9	0.0	13.9	0.0	10.7	0.0	0.0	0.0
IncremntDel:	0.0	8.0	0.0	0.0	0.1	0.0	9.0	0.0	0.4	0.0	0.0	0.0
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	0.00	1.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00	0.00
Delay/Veh:	0.0	22.5	0.0	0.0	11.0	0.0	22.9	0.0	11.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	22.5	0.0	0.0	11.0	0.0	22.9	0.0	11.0	0.0	0.0	0.0
LOS by Move:	A	C	A	A	B	A	C	A	B	A	A	A
HCM2kAvgQ:	0	13	0	0	4	0	17	0	6	0	0	0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative AM

Intersection #22: Wolfe Road and I-280 SB Ramps (CUP/CMP)



Street Name:		Wolfe Road										I-280 SB Ramps					
Approach:	North Bound	South Bound			East Bound			West Bound									
Movement:	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	
Min. Green:	7	10	10	7	10	10	10	10	10	10	0	0	0	0	0	0	
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	

Volume Module:

Base Vol:	0	1286	490	0	1084	485	726	0	574	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1286	490	0	1084	485	726	0	574	0	0	0	0	0	0	0
Added Vol:	0	274	6	0	29	61	584	0	266	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1560	496	0	1113	546	1310	0	840	0	0	0	0	0	0	0
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1560	0	0	1113	0	1310	0	840	0	0	0	0	0	0	0
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1560	0	0	1113	0	1310	0	840	0	0	0	0	0	0	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1560	0	0	1113	0	1310	0	840	0	0	0	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.83	0.92	1.00	0.92	1.00	0.92	1.00
Lanes:	0.00	2.00	1.00	0.00	3.00	1.00	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00
Final Sat.:	0	3800	1750	0	5700	1750	3150	0	3150	0	0	0	0	0	0

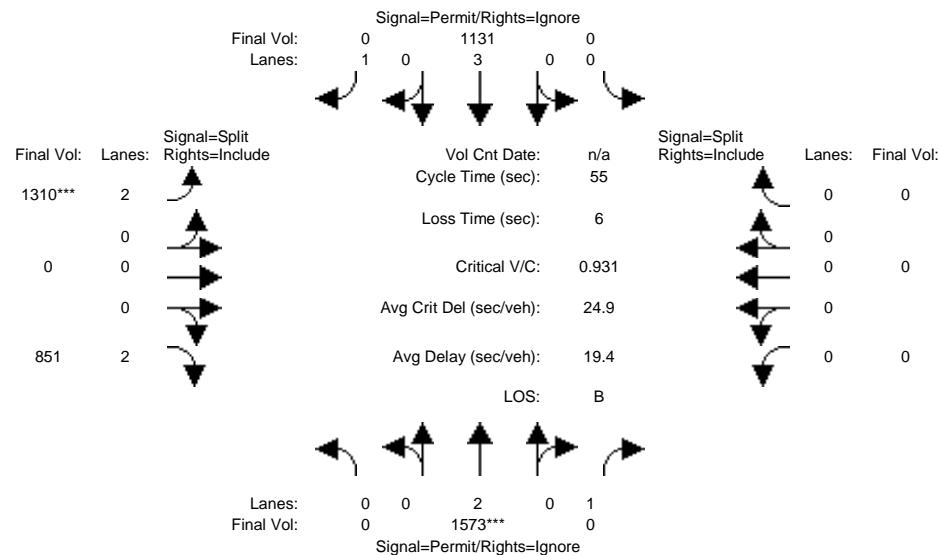
Capacity Analysis Module:

Vol/Sat:	0.00	0.41	0.00	0.00	0.20	0.00	0.42	0.00	0.27	0.00	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****						****								
Green Time:	0.0	24.3	0.0	0.0	24.3	0.0	24.7	0.0	24.7	0.0	0.0	0.0	0.0	0.0	0.0
Volume/Cap:	0.00	0.93	0.00	0.00	0.44	0.00	0.93	0.00	0.59	0.00	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	0.0	23.9	0.0	0.0	10.7	0.0	25.2	0.0	12.1	0.0	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	23.9	0.0	0.0	10.7	0.0	25.2	0.0	12.1	0.0	0.0	0.0	0.0	0.0	0.0
LOS by Move:	A	C	A	A	B	A	C	A	B	A	A	A	A	A	A
HCM2kAvgQ:	0	14	0	0	4	0	18	0	7	0	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + Proj AM

Intersection #22: Wolfe Road and I-280 SB Ramps (CUP/CMP)



Street Name:		Wolfe Road												I-280 SB Ramps												
Approach:		North Bound				South Bound				East Bound				West Bound												
Movement:		L	-	T	-	R	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:		7	10	10	7	10	10	10	10	10	10	10	10	10	10	10	0	0	0	0	0	0	0	0	0	0
Y+R:		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	1286	490	0	1084	485	726	0	574	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Initial Bse:	0	1286	490	0	1084	485	726	0	574	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Added Vol:	0	274	6	0	29	61	584	0	266	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hyatt:	0	13	7	0	18	0	0	0	11	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1573	503	0	1131	546	1310	0	851	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
PHF Volume:	0	1573	0	0	1131	0	1310	0	851	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1573	0	0	1131	0	1310	0	851	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Final Volume:	0	1573	0	0	1131	0	1310	0	851	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.83	1.00	0.83	1.00	0.83	1.00	0.83	1.00	0.83	1.00	0.83	1.00	0.83	1.00	0.83	1.00	
Lanes:	0.00	2.00	1.00	0.00	3.00	1.00	2.00	0.00	2.00	0.00	2.00	0.00	2.00	0.00	2.00	0.00	2.00	0.00	2.00	0.00	2.00	0.00	2.00	0.00	
Final Sat.:	0	3800	1750	0	5700	1750	3150	0	3150	0	3150	0	3150	0	3150	0	3150	0	3150	0	3150	0	3150	0	3150

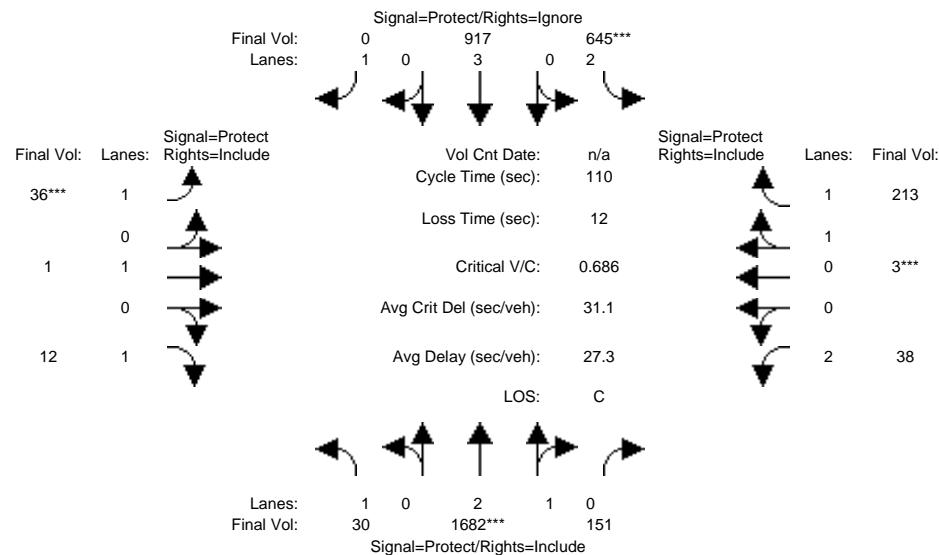
Capacity Analysis Module:

Vol/Sat:	0.00	0.41	0.00	0.00	0.20	0.00	0.42	0.00	0.27	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Crit Moves:	****						****																					
Green Time:	0.0	24.4	0.0	0.0	24.4	0.0	24.6	0.0	24.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
Volume/Cap:	0.00	0.93	0.00	0.00	0.45	0.00	0.93	0.00	0.61	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Delay/Veh:	0.0	24.3	0.0	0.0	10.7	0.0	25.7	0.0	12.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
AdjDel/Veh:	0.0	24.3	0.0	0.0	10.7	0.0	25.7	0.0	12.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
LOS by Move:	A	C	A	A	B	A	C	A	B	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A		
HCM2kAvgQ:	0	14	0	0	4	0	18	0	7	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd AM

Intersection #23: Wolfe Road and Vallco Parkway (CUP)



Street Name:		Wolfe Road				Vallco Parkway			
Approach:	North Bound	South Bound		East Bound		West Bound			
Movement:	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R
Min. Green:	7	10	10	7	10	10	7	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	30	1456	151	373	894	74	36	1	12	38	3	159
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	30	1456	151	373	894	74	36	1	12	38	3	159
Added Vol:	0	226	0	272	23	0	0	0	0	0	0	54
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	30	1682	151	645	917	74	36	1	12	38	3	213
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	30	1682	151	645	917	0	36	1	12	38	3	213
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	30	1682	151	645	917	0	36	1	12	38	3	213
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	30	1682	151	645	917	0	36	1	12	38	3	213

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92
Lanes:	1.00	2.73	0.27	2.00	3.00	1.00	1.00	1.00	1.00	2.00	0.03	1.97
Final Sat.:	1750	5194	466	3150	5700	1750	1750	1900	1750	3150	49	3455

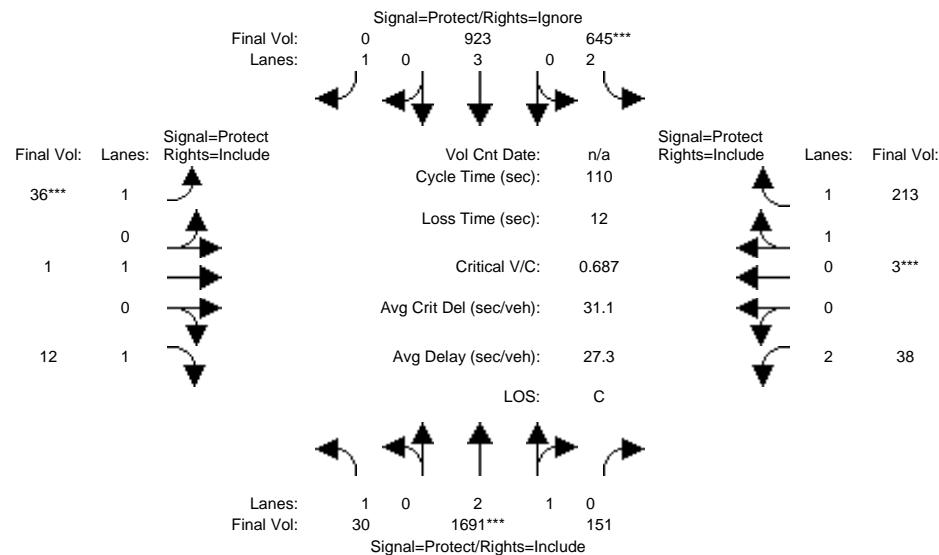
Capacity Analysis Module:

Vol/Sat:	0.02	0.32	0.32	0.20	0.16	0.00	0.02	0.00	0.01	0.01	0.06	0.06
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	23.0	49.6	49.6	31.4	58.0	0.0	7.0	10.0	10.0	7.0	10.0	10.0
Volume/Cap:	0.08	0.72	0.72	0.72	0.30	0.00	0.32	0.01	0.08	0.19	0.68	0.68
Uniform Del:	35.0	24.5	24.5	35.3	14.6	0.0	49.2	45.5	45.8	48.8	48.4	48.4
IncremntDel:	0.1	1.0	1.0	2.8	0.1	0.0	1.7	0.0	0.2	0.5	5.8	5.8
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	35.1	25.5	25.5	38.2	14.7	0.0	50.9	45.5	46.0	49.3	54.2	54.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	35.1	25.5	25.5	38.2	14.7	0.0	50.9	45.5	46.0	49.3	54.2	54.2
LOS by Move:	D	C	C	D	B	A	D	D	D	D	D	D
HCM2kAvgQ:	1	16	16	12	6	0	2	0	0	1	4	4

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + Proj AM

Intersection #23: Wolfe Road and Vallco Parkway (CUP)



Street Name:	Wolfe Road						Vallco Parkway								
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10		
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		

Volume Module:

Base Vol:	30	1456	151	373	894	74	36	1	12	38	3	159
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	30	1456	151	373	894	74	36	1	12	38	3	159
Added Vol:	0	226	0	272	23	0	0	0	0	0	0	54
Hyatt:	0	9	0	0	6	0	0	0	0	0	0	0
Initial Fut:	30	1691	151	645	923	74	36	1	12	38	3	213
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	30	1691	151	645	923	0	36	1	12	38	3	213
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	30	1691	151	645	923	0	36	1	12	38	3	213
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	30	1691	151	645	923	0	36	1	12	38	3	213

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92
Lanes:	1.00	2.73	0.27	2.00	3.00	1.00	1.00	1.00	1.00	2.00	0.03	1.97
Final Sat.:	1750	5196	464	3150	5700	1750	1750	1900	1750	3150	49	3455

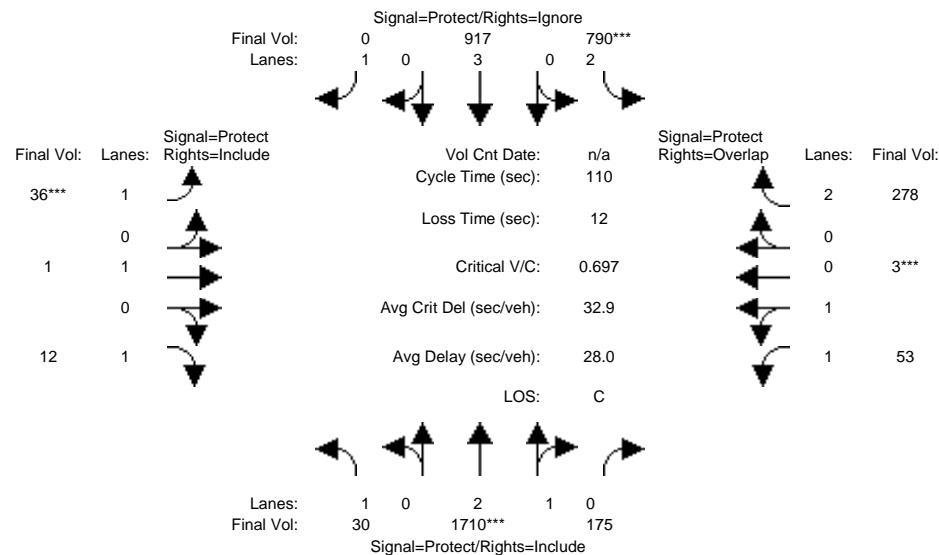
Capacity Analysis Module:

Vol/Sat:	0.02	0.33	0.33	0.20	0.16	0.00	0.02	0.00	0.01	0.01	0.06	0.06
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	22.9	49.7	49.7	31.3	58.1	0.0	7.0	10.0	10.0	7.0	10.0	10.0
Volume/Cap:	0.08	0.72	0.72	0.72	0.31	0.00	0.32	0.01	0.08	0.19	0.68	0.68
Uniform Del:	35.1	24.5	24.5	35.4	14.6	0.0	49.2	45.5	45.8	48.8	48.4	48.4
IncremntDel:	0.1	1.0	1.0	2.9	0.1	0.0	1.7	0.0	0.2	0.5	5.8	5.8
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	35.2	25.5	25.5	38.3	14.6	0.0	50.9	45.5	46.0	49.3	54.2	54.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	35.2	25.5	25.5	38.3	14.6	0.0	50.9	45.5	46.0	49.3	54.2	54.2
LOS by Move:	D	C	C	D	B	A	D	D	D	D	D	D
HCM2kAvgQ:	1	16	16	12	6	0	2	0	0	1	4	4

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative AM

Intersection #23: Wolfe Road and Vallco Parkway (CUP)



		Wolfe Road				Vallco Parkway			
Approach:	North Bound	South Bound		East Bound		West Bound			
Movement:	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R		
Min. Green:	7	10	10	7	10	10	7	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	30	1484	175	518	894	74	36	1	12	53	3	224
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	30	1484	175	518	894	74	36	1	12	53	3	224
Added Vol:	0	226	0	272	23	0	0	0	0	0	0	54
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	30	1710	175	790	917	74	36	1	12	53	3	278
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	30	1710	175	790	917	0	36	1	12	53	3	278
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	30	1710	175	790	917	0	36	1	12	53	3	278
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	30	1710	175	790	917	0	36	1	12	53	3	278

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.83
Lanes:	1.00	2.70	0.30	2.00	3.00	1.00	1.00	1.00	1.00	1.90	0.10	2.00
Final Sat.:	1750	5130	525	3150	5700	1750	1750	1900	1750	3327	188	3150

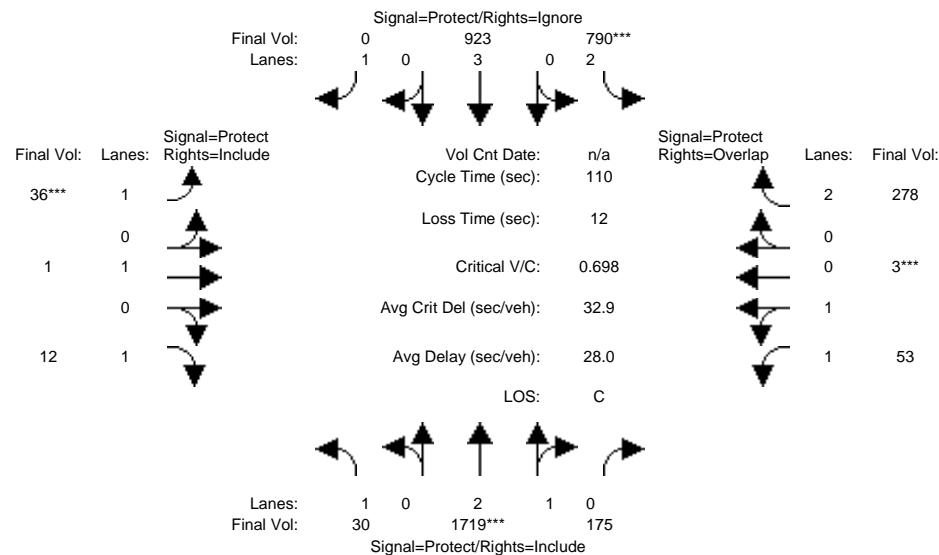
Capacity Analysis Module:

Vol/Sat:	0.02	0.33	0.33	0.25	0.16	0.00	0.02	0.00	0.01	0.02	0.02	0.09
Crit Moves:	****		****	****		****	****		****	****		****
Green Time:	23.0	46.2	46.2	34.8	58.0	0.0	7.0	10.0	10.0	7.0	10.0	44.8
Volume/Cap:	0.08	0.79	0.79	0.79	0.30	0.00	0.32	0.01	0.08	0.25	0.18	0.22
Delay/Veh:	35.1	29.6	29.6	38.8	14.7	0.0	50.9	45.5	46.0	49.6	46.5	21.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	35.1	29.6	29.6	38.8	14.7	0.0	50.9	45.5	46.0	49.6	46.5	21.3
LOS by Move:	D	C	C	D	B	A	D	D	D	D	D	C
HCM2kAvgQ:	1	18	18	15	6	0	2	0	0	1	1	4

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + Proj AM

Intersection #23: Wolfe Road and Vallco Parkway (CUP)



Street Name:

Wolfe Road

Vallco Parkway

Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R

Min. Green:	7	10	10	7	10	10	7	10	10	7	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	30	1484	175	518	894	74	36	1	12	53	3	224
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	30	1484	175	518	894	74	36	1	12	53	3	224
Added Vol:	0	226	0	272	23	0	0	0	0	0	0	54
Hyatt:	0	9	0	0	6	0	0	0	0	0	0	0
Initial Fut:	30	1719	175	790	923	74	36	1	12	53	3	278
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	30	1719	175	790	923	0	36	1	12	53	3	278
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	30	1719	175	790	923	0	36	1	12	53	3	278
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	30	1719	175	790	923	0	36	1	12	53	3	278

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00
Lanes:	1.00	2.70	0.30	2.00	3.00	1.00	1.00	1.00	1.00	1.90	0.10
Final Sat.:	1750	5133	523	3150	5700	1750	1750	1900	1750	3327	188

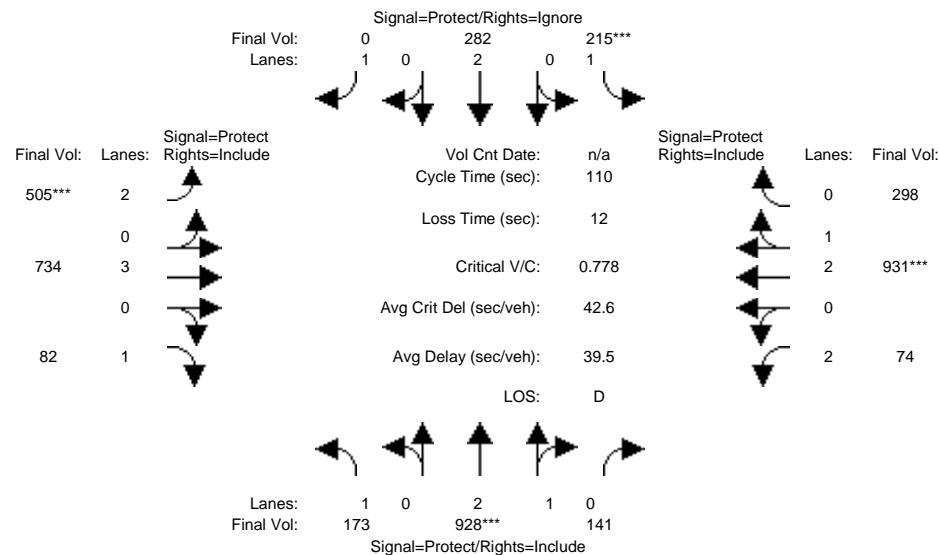
Capacity Analysis Module:

Vol/Sat:	0.02	0.33	0.33	0.25	0.16	0.00	0.02	0.00	0.01	0.02	0.02	0.09
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	22.9	46.3	46.3	34.7	58.1	0.0	7.0	10.0	10.0	7.0	10.0	44.7
Volume/Cap:	0.08	0.80	0.80	0.80	0.31	0.00	0.32	0.01	0.08	0.25	0.18	0.22
Delay/Veh:	35.2	29.7	29.7	38.9	14.6	0.0	50.9	45.5	46.0	49.6	46.5	21.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	35.2	29.7	29.7	38.9	14.6	0.0	50.9	45.5	46.0	49.6	46.5	21.4
LOS by Move:	D	C	C	D	B	A	D	D	D	D	D	C
HCM2kAvgQ:	1	18	18	15	6	0	2	0	0	1	1	4

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkdg AM

Intersection #24: Stevens Creek Blvd and Wolfe Rd/Miller Ave (CUP/CMP)



Street Name:	Wolfe Rd/Miller Ave					Stevens Creek Blvd									
Approach:	North Bound			South Bound		East Bound			West Bound						
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	7	10	10	7	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	173	899	136	206	279	512	451	648	82	72	907	155
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	173	899	136	206	279	512	451	648	82	72	907	155
Added Vol:	0	29	5	9	3	11	54	86	0	2	24	143
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	173	928	141	215	282	523	505	734	82	74	931	298
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	173	928	141	215	282	0	505	734	82	74	931	298
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	173	928	141	215	282	0	505	734	82	74	931	298
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	173	928	141	215	282	0	505	734	82	74	931	298

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92	0.83	1.00	0.92
Lanes:	1.00	2.58	0.42	1.00	2.00	1.00	2.00	3.00	1.00	2.00	2.23	0.77
Final Sat.:	1750	4893	743	1750	3800	1750	3150	5700	1750	3150	4230	1354

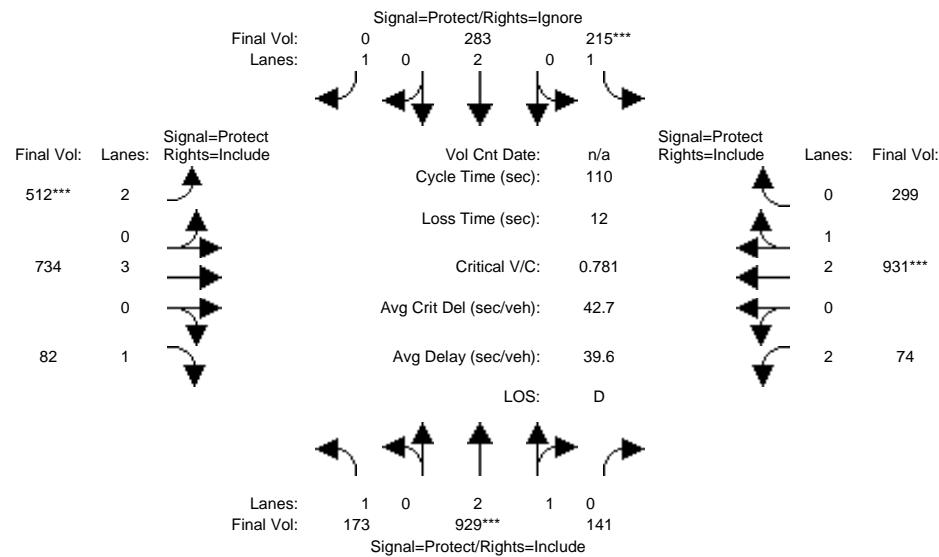
Capacity Analysis Module:

Vol/Sat:	0.10	0.19	0.19	0.12	0.07	0.00	0.16	0.13	0.05	0.02	0.22	0.22
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	23.0	26.8	26.8	17.4	21.2	0.0	22.7	36.0	36.0	17.8	31.1	31.1
Volume/Cap:	0.47	0.78	0.78	0.39	0.00	0.78	0.39	0.14	0.15	0.78	0.78	0.78
Uniform Del:	38.2	38.8	38.8	44.5	38.7	0.0	41.3	28.6	26.1	39.6	36.3	36.3
IncremntDel:	1.0	2.9	2.9	13.1	0.3	0.0	5.9	0.1	0.1	0.1	2.5	2.5
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	39.1	41.7	41.7	57.5	39.1	0.0	47.2	28.7	26.2	39.7	38.8	38.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	39.1	41.7	41.7	57.5	39.1	0.0	47.2	28.7	26.2	39.7	38.8	38.8
LOS by Move:	D	D	D	E	D	A	D	C	C	D	D	D
HCM2kAvgQ:	6	13	13	8	4	0	12	6	2	1	13	13

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + Proj AM

Intersection #24: Stevens Creek Blvd and Wolfe Rd/Miller Ave (CUP/CMP)



Street Name:	Wolfe Rd/Miller Ave					Stevens Creek Blvd									
Approach:	North Bound			South Bound		East Bound			West Bound						
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	10	
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	

Volume Module:

Base Vol:	173	899	136	206	279	512	451	648	82	72	907	155
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	173	899	136	206	279	512	451	648	82	72	907	155
Added Vol:	0	29	5	9	3	11	54	86	0	2	24	143
Hyatt:	0	1	0	0	1	5	7	0	0	0	0	1
Initial Fut:	173	929	141	215	283	528	512	734	82	74	931	299
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	173	929	141	215	283	0	512	734	82	74	931	299
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	173	929	141	215	283	0	512	734	82	74	931	299
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	173	929	141	215	283	0	512	734	82	74	931	299

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92	0.83	1.00	0.92
Lanes:	1.00	2.58	0.42	1.00	2.00	1.00	2.00	3.00	1.00	2.00	2.22	0.78
Final Sat.:	1750	4894	743	1750	3800	1750	3150	5700	1750	3150	4226	1357

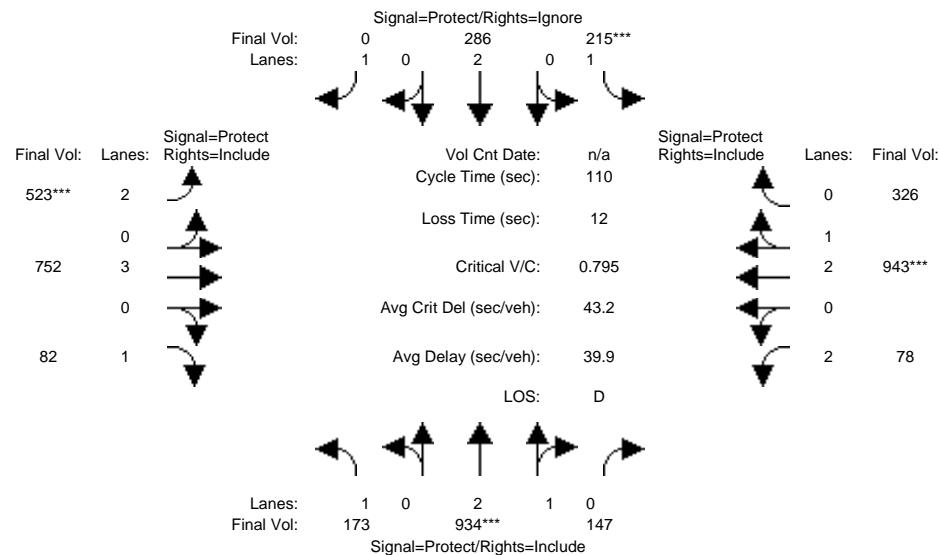
Capacity Analysis Module:

Vol/Sat:	0.10	0.19	0.19	0.12	0.07	0.00	0.16	0.13	0.05	0.02	0.22	0.22
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	23.0	26.7	26.7	17.3	21.1	0.0	22.9	36.1	36.1	17.8	31.0	31.0
Volume/Cap:	0.47	0.78	0.78	0.78	0.39	0.00	0.78	0.39	0.14	0.14	0.78	0.78
Uniform Del:	38.2	38.9	38.9	44.5	38.8	0.0	41.2	28.5	26.0	39.5	36.3	36.3
IncremntDel:	1.0	3.0	3.0	13.4	0.3	0.0	6.0	0.1	0.1	0.1	2.6	2.6
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	39.2	41.8	41.8	57.9	39.2	0.0	47.2	28.6	26.2	39.7	38.9	38.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	39.2	41.8	41.8	57.9	39.2	0.0	47.2	28.6	26.2	39.7	38.9	38.9
LOS by Move:	D	D	D	E	D	A	D	C	C	D	D	D
HCM2kAvgQ:	6	13	13	8	4	0	12	6	2	1	13	13

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative AM

Intersection #24: Stevens Creek Blvd and Wolfe Rd/Miller Ave (CUP/CMP)



Street Name:	Wolfe Rd/Miller Ave						Stevens Creek Blvd								
	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10		
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		

Volume Module:

Base Vol:	173	905	142	206	283	524	469	666	82	76	919	183
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	173	905	142	206	283	524	469	666	82	76	919	183
Added Vol:	0	29	5	9	3	11	54	86	0	2	24	143
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	173	934	147	215	286	535	523	752	82	78	943	326
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	173	934	147	215	286	0	523	752	82	78	943	326
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	173	934	147	215	286	0	523	752	82	78	943	326
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	173	934	147	215	286	0	523	752	82	78	943	326

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92	0.83	1.00	0.92
Lanes:	1.00	2.56	0.44	1.00	2.00	1.00	2.00	3.00	1.00	2.00	2.18	0.82
Final Sat.:	1750	4868	766	1750	3800	1750	3150	5700	1750	3150	4144	1433

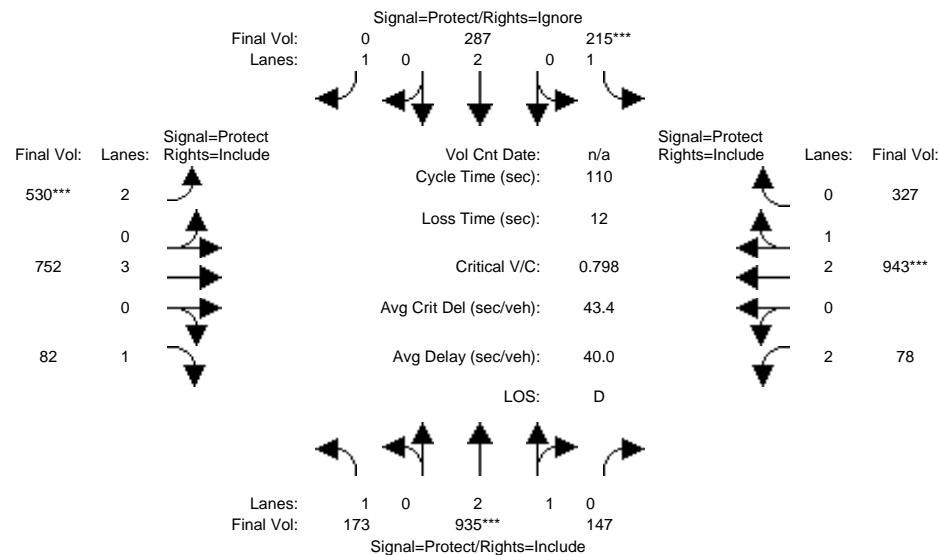
Capacity Analysis Module:

Vol/Sat:	0.10	0.19	0.19	0.12	0.08	0.00	0.17	0.13	0.05	0.02	0.23	0.23
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	22.7	26.5	26.5	17.0	20.9	0.0	23.0	36.7	36.7	17.7	31.5	31.5
Volume/Cap:	0.48	0.80	0.80	0.80	0.40	0.00	0.80	0.40	0.14	0.15	0.80	0.80
Delay/Veh:	39.5	42.5	42.5	59.8	39.4	0.0	47.9	28.2	25.7	39.8	39.1	39.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	39.5	42.5	42.5	59.8	39.4	0.0	47.9	28.2	25.7	39.8	39.1	39.1
LOS by Move:	D	D	D	E	D	A	D	C	C	D	D	D
HCM2kAvgQ:	6	14	14	8	4	0	12	7	2	1	14	14

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + Proj AM

Intersection #24: Stevens Creek Blvd and Wolfe Rd/Miller Ave (CUP/CMP)



Street Name:	Wolfe Rd/Miller Ave						Stevens Creek Blvd								
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10		
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		

Volume Module:

Base Vol:	173	905	142	206	283	524	469	666	82	76	919	183
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	173	905	142	206	283	524	469	666	82	76	919	183
Added Vol:	0	29	5	9	3	11	54	86	0	2	24	143
Hyatt:	0	1	0	0	1	5	7	0	0	0	0	1
Initial Fut:	173	935	147	215	287	540	530	752	82	78	943	327
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	173	935	147	215	287	0	530	752	82	78	943	327
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	173	935	147	215	287	0	530	752	82	78	943	327
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	173	935	147	215	287	0	530	752	82	78	943	327

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92	0.83	1.00	0.92
Lanes:	1.00	2.56	0.44	1.00	2.00	1.00	2.00	3.00	1.00	2.00	2.18	0.82
Final Sat.:	1750	4869	765	1750	3800	1750	3150	5700	1750	3150	4141	1436

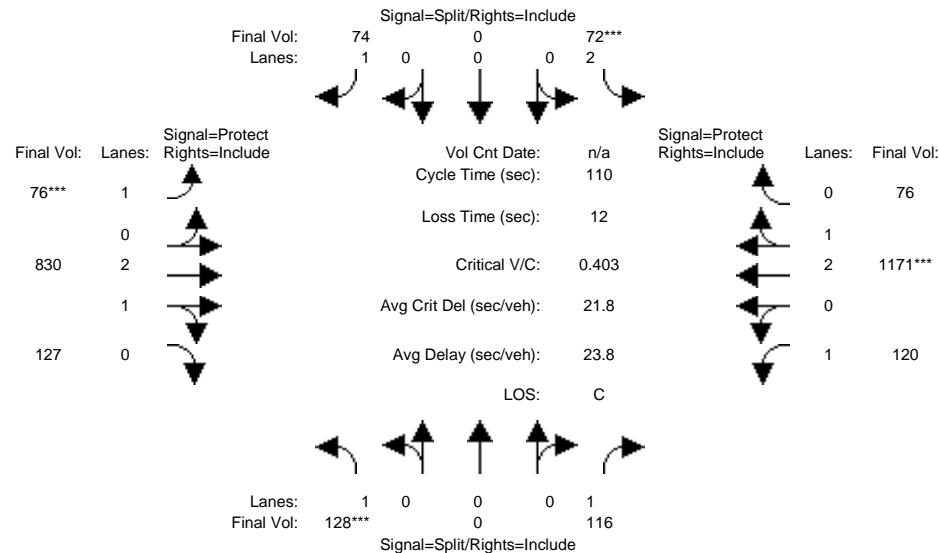
Capacity Analysis Module:

Vol/Sat:	0.10	0.19	0.19	0.12	0.08	0.00	0.17	0.13	0.05	0.02	0.23	0.23
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	22.6	26.5	26.5	16.9	20.8	0.0	23.2	36.8	36.8	17.8	31.4	31.4
Volume/Cap:	0.48	0.80	0.80	0.80	0.40	0.00	0.80	0.39	0.14	0.15	0.80	0.80
Delay/Veh:	39.5	42.7	42.7	60.2	39.5	0.0	47.9	28.2	25.6	39.8	39.3	39.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	39.5	42.7	42.7	60.2	39.5	0.0	47.9	28.2	25.6	39.8	39.3	39.3
LOS by Move:	D	D	D	E	D	A	D	C	C	D	D	D
HCM2kAvgQ:	6	14	14	8	4	0	12	7	2	1	14	14

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd AM

Intersection #26: Stevens Creek Blvd and Finch Avenue (CUP)



Street Name:	Finch Avenue				Stevens Creek Blvd										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	10	10	10	10	10	10	7	10	10	10	7	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	128	0	116	72	0	74	76	730	127	118	1002	76
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	128	0	116	72	0	74	76	730	127	118	1002	76
Added Vol:	0	0	0	0	0	0	0	100	0	2	169	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	128	0	116	72	0	74	76	830	127	120	1171	76
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	128	0	116	72	0	74	76	830	127	120	1171	76
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	128	0	116	72	0	74	76	830	127	120	1171	76
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	128	0	116	72	0	74	76	830	127	120	1171	76

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	1.00	0.00	1.00	2.00	0.00	1.00	1.00	2.57	0.43	1.00	2.80	0.20
Final Sat.:	1750	0	1750	3150	0	1750	1750	4888	748	1750	5325	346

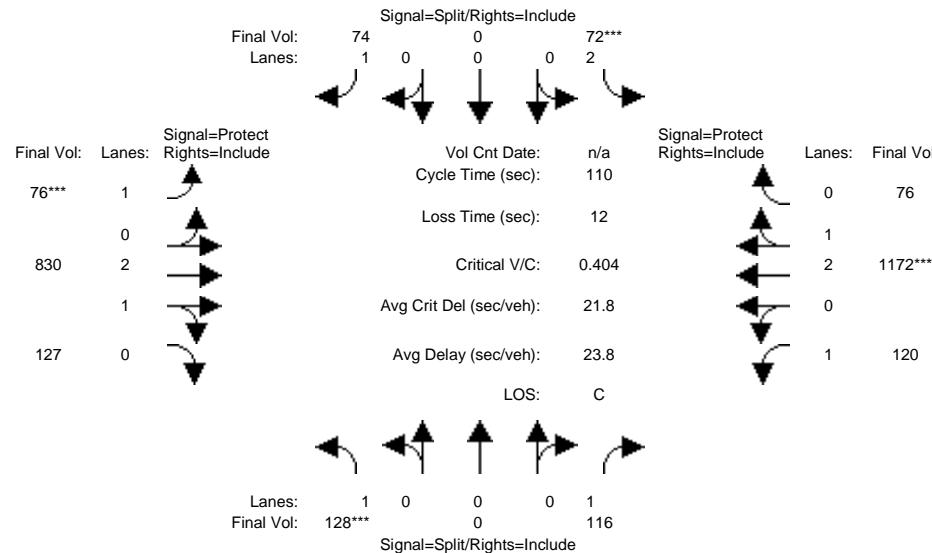
Capacity Analysis Module:

Vol/Sat:	0.07	0.00	0.07	0.02	0.00	0.04	0.04	0.17	0.17	0.07	0.22	0.22
Crit Moves:	****		****	****		****	****		****		****	
Green Time:	18.8	0.0	18.8	11.5	0.0	11.5	11.2	48.2	48.2	19.5	56.5	56.5
Volume/Cap:	0.43	0.00	0.39	0.22	0.00	0.40	0.43	0.39	0.39	0.39	0.43	0.43
Uniform Del:	40.8	0.0	40.5	45.1	0.0	46.0	46.4	20.9	20.9	40.0	16.7	16.7
IncremntDel:	1.0	0.0	0.8	0.3	0.0	1.5	1.7	0.1	0.1	0.8	0.1	0.1
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	41.8	0.0	41.3	45.4	0.0	47.5	48.1	21.0	21.0	40.8	16.8	16.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	41.8	0.0	41.3	45.4	0.0	47.5	48.1	21.0	21.0	40.8	16.8	16.8
LOS by Move:	D	A	D	D	A	D	D	C	C	D	B	B
HCM2kAvgQ:	5	0	4	1	0	3	3	7	7	4	8	8

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + Proj AM

Intersection #26: Stevens Creek Blvd and Finch Avenue (CUP)



Street Name:	Finch Avenue				Stevens Creek Blvd										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	10	10	10	10	10	10	7	10	10	10	7	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	128	0	116	72	0	74	76	730	127	118	1002	76
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	128	0	116	72	0	74	76	730	127	118	1002	76
Added Vol:	0	0	0	0	0	0	0	100	0	2	169	0
Hyatt:	0	0	0	0	0	0	0	0	0	0	1	0
Initial Fut:	128	0	116	72	0	74	76	830	127	120	1172	76
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	128	0	116	72	0	74	76	830	127	120	1172	76
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	128	0	116	72	0	74	76	830	127	120	1172	76
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	128	0	116	72	0	74	76	830	127	120	1172	76

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	1.00	0.00	1.00	2.00	0.00	1.00	1.00	2.57	0.43	1.00	2.80	0.20
Final Sat.:	1750	0	1750	3150	0	1750	1750	4888	748	1750	5325	345

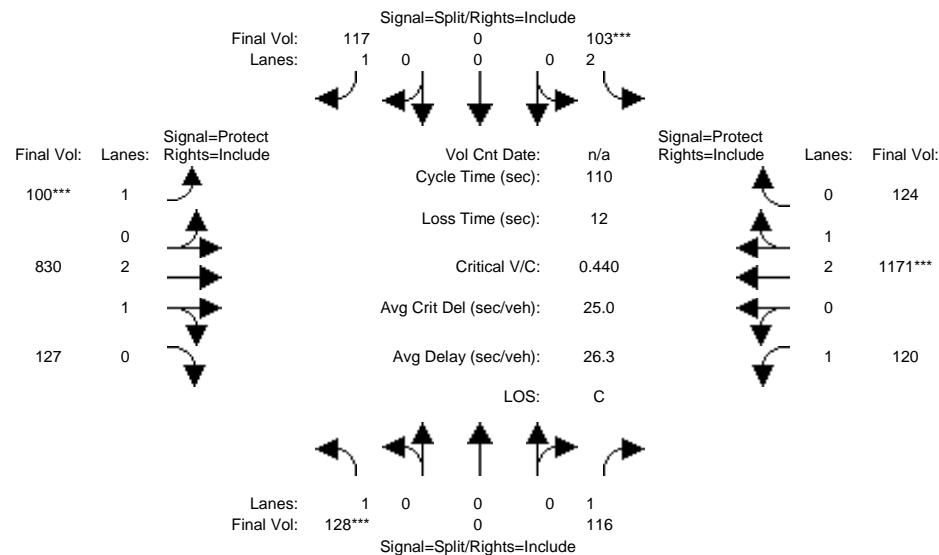
Capacity Analysis Module:

Vol/Sat:	0.07	0.00	0.07	0.02	0.00	0.04	0.04	0.17	0.17	0.07	0.22	0.22
Crit Moves:	****		****		****		****		****		****	
Green Time:	18.8	0.0	18.8	11.5	0.0	11.5	11.2	48.2	48.2	19.5	56.5	56.5
Volume/Cap:	0.43	0.00	0.39	0.22	0.00	0.40	0.43	0.39	0.39	0.39	0.43	0.43
Uniform Del:	40.8	0.0	40.5	45.1	0.0	46.0	46.4	20.9	20.9	40.0	16.7	16.7
IncremntDel:	1.0	0.0	0.8	0.3	0.0	1.5	1.7	0.1	0.1	0.8	0.1	0.1
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	41.8	0.0	41.3	45.4	0.0	47.5	48.1	21.0	21.0	40.8	16.8	16.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	41.8	0.0	41.3	45.4	0.0	47.5	48.1	21.0	21.0	40.8	16.8	16.8
LOS by Move:	D	A	D	D	A	D	D	C	C	D	B	B
HCM2kAvgQ:	5	0	4	1	0	3	3	7	7	4	8	8

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative AM

Intersection #26: Stevens Creek Blvd and Finch Avenue (CUP)

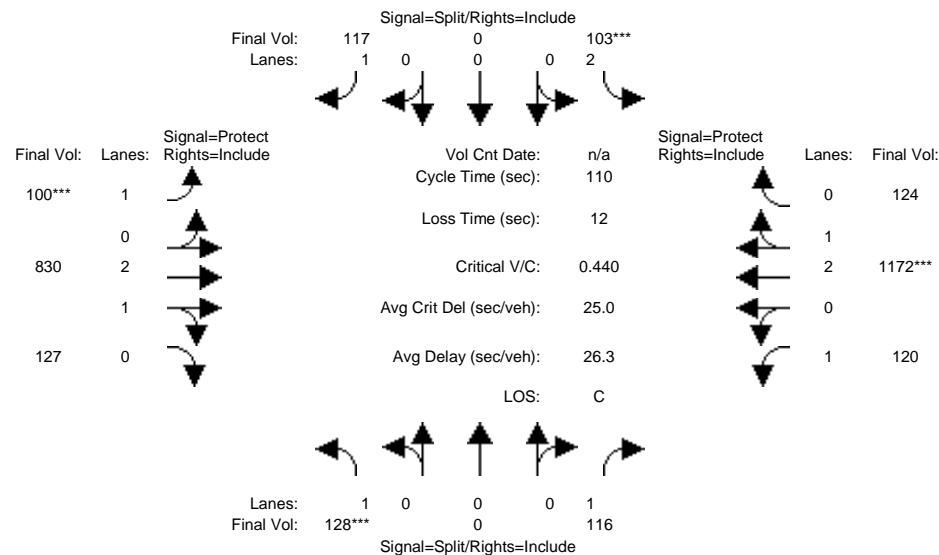


Street Name: Finch Avenue Stevens Creek Blvd														
Approach:	North Bound			South Bound			East Bound			West Bound				
	L	-	T	-	R	L	-	T	-	R	L	-	T	-
Min. Green:	10 10		10 10		10 10		7 10		10 10		7 10		10 10	
Y+R:	4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0	
Volume Module:	<hr/>													
Base Vol:	128	0	116	103	0	117	100	730	127	118	1002	124		
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Initial Bse:	128	0	116	103	0	117	100	730	127	118	1002	124		
Added Vol:	0	0	0	0	0	0	0	100	0	2	169	0		
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0		
Initial Fut:	128	0	116	103	0	117	100	830	127	120	1171	124		
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
PHF Volume:	128	0	116	103	0	117	100	830	127	120	1171	124		
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0		
Reduced Vol:	128	0	116	103	0	117	100	830	127	120	1171	124		
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
FinalVolume:	128	0	116	103	0	117	100	830	127	120	1171	124		
Saturation Flow Module:	<hr/>													
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92		
Lanes:	1.00	0.00	1.00	2.00	0.00	1.00	1.00	2.57	0.43	1.00	2.69	0.31		
Final Sat.:	1750	0	1750	3150	0	1750	1750	4888	748	1750	5112	541		
Capacity Analysis Module:	<hr/>													
Vol/Sat:	0.07	0.00	0.07	0.03	0.00	0.07	0.06	0.17	0.17	0.07	0.23	0.23		
Crit Moves:	****		****		****		****		****		****			
Green Time:	16.5	0.0	16.5	16.7	0.0	16.7	12.9	46.1	46.1	18.6	51.8	51.8		
Volume/Cap:	0.49	0.00	0.44	0.22	0.00	0.44	0.49	0.41	0.41	0.41	0.49	0.49		
Delay/Veh:	44.2	0.0	43.7	41.1	0.0	43.6	47.2	22.5	22.5	41.7	20.1	20.1		
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
AdjDel/Veh:	44.2	0.0	43.7	41.1	0.0	43.6	47.2	22.5	22.5	41.7	20.1	20.1		
LOS by Move:	D	A	D	D	A	D	D	C	C	D	C	C		
HCM2kAvgQ:	5	0	4	2	0	4	3	7	7	4	9	9		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + Proj AM

Intersection #26: Stevens Creek Blvd and Finch Avenue (CUP)



Street Name:	Finch Avenue				Stevens Creek Blvd										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	10	10	10	10	10	10	7	10	10	10	7	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	128	0	116	103	0	117	100	730	127	118	1002	124
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	128	0	116	103	0	117	100	730	127	118	1002	124
Added Vol:	0	0	0	0	0	0	0	100	0	2	169	0
Hyatt:	0	0	0	0	0	0	0	0	0	0	1	0
Initial Fut:	128	0	116	103	0	117	100	830	127	120	1172	124
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	128	0	116	103	0	117	100	830	127	120	1172	124
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	128	0	116	103	0	117	100	830	127	120	1172	124
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	128	0	116	103	0	117	100	830	127	120	1172	124

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	1.00	0.00	1.00	2.00	0.00	1.00	1.00	2.57	0.43	1.00	2.69	0.31
Final Sat.:	1750	0	1750	3150	0	1750	1750	4888	748	1750	5113	541

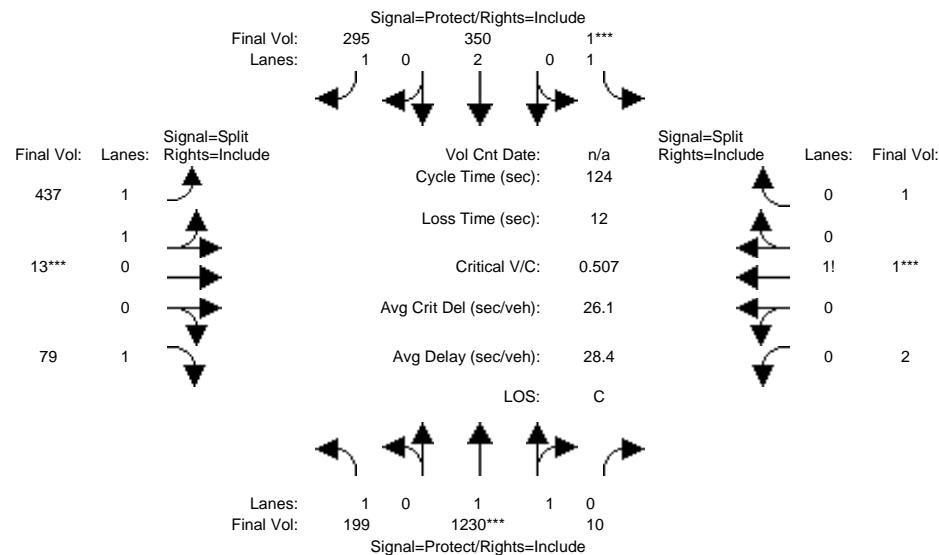
Capacity Analysis Module:

Vol/Sat:	0.07	0.00	0.07	0.03	0.00	0.07	0.06	0.17	0.17	0.07	0.23	0.23
Crit Moves:	****		****		****		****		****		****	
Green Time:	16.5	0.0	16.5	16.7	0.0	16.7	12.9	46.1	46.1	18.6	51.8	51.8
Volume/Cap:	0.49	0.00	0.44	0.22	0.00	0.44	0.49	0.40	0.40	0.40	0.49	0.49
Delay/Veh:	44.3	0.0	43.7	41.1	0.0	43.6	47.2	22.5	22.5	41.6	20.1	20.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	44.3	0.0	43.7	41.1	0.0	43.6	47.2	22.5	22.5	41.6	20.1	20.1
LOS by Move:	D	A	D	D	A	D	D	C	C	D	C	C
HCM2kAvgQ:	5	0	4	2	0	4	3	7	7	4	9	9

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd AM

Intersection #31: Tantau Avenue and Vallco Parkway (CUP)

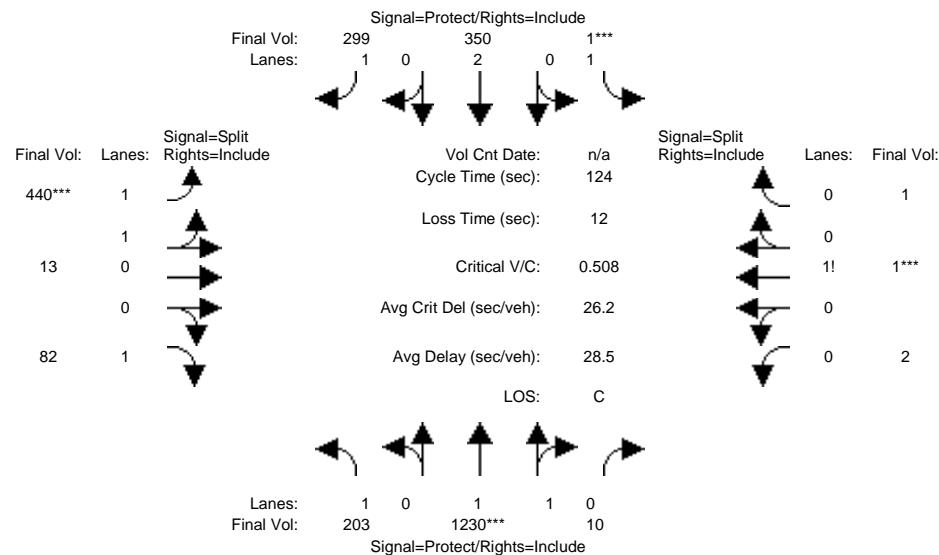


Street Name: Tantau Avenue Vallco Parkway															
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7		10	10		7	10		10	10		10	10		10
Y+R:	4.0		4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0		4.0
Volume Module:	<hr/>														
Base Vol:	199	530	10	1	286	241	165	13	79	2	1	1			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	199	530	10	1	286	241	165	13	79	2	1	1			
Added Vol:	0	700	0	0	64	54	272	0	0	0	0	0			
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0			
Initial Fut:	199	1230	10	1	350	295	437	13	79	2	1	1			
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Volume:	199	1230	10	1	350	295	437	13	79	2	1	1			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	199	1230	10	1	350	295	437	13	79	2	1	1			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
FinalVolume:	199	1230	10	1	350	295	437	13	79	2	1	1			
Saturation Flow Module:	<hr/>														
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900				
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92			
Lanes:	1.00	1.98	0.02	1.00	2.00	1.00	1.95	0.05	1.00	0.51	0.23	0.26			
Final Sat.:	1750	3767	31	1750	3800	1750	3407	101	1750	893	446	446			
Capacity Analysis Module:	<hr/>														
Vol/Sat:	0.11	0.33	0.33	0.00	0.09	0.17	0.13	0.13	0.05	0.00	0.00	0.00			
Crit Moves:	****			****			****			****					
Green Time:	30.3	68.2	68.2	7.0	44.9	44.9	26.8	26.8	26.8	10.0	10.0	10.0			
Volume/Cap:	0.47	0.59	0.59	0.01	0.25	0.47	0.59	0.59	0.21	0.03	0.03	0.03			
Uniform Del:	39.9	18.6	18.6	55.2	27.8	30.3	43.7	43.7	39.9	52.5	52.5	52.5			
IncremntDel:	0.8	0.5	0.5	0.0	0.1	0.5	1.3	1.3	0.3	0.1	0.1	0.1			
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Delay/Veh:	40.8	19.1	19.1	55.3	27.9	30.9	45.0	45.0	40.2	52.6	52.6	52.6			
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
AdjDel/Veh:	40.8	19.1	19.1	55.3	27.9	30.9	45.0	45.0	40.2	52.6	52.6	52.6			
LOS by Move:	D	B	B	E	C	C	D	D	D	D	D	D			
HCM2kAvgQ:	7	15	15	0	4	9	8	8	3	0	0	0			

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + Proj AM

Intersection #31: Tantau Avenue and Vallco Parkway (CUP)



Street Name:	Tantau Avenue				Vallco Parkway										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	10	10	10	10	10	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	199	530	10	1	286	241	165	13	79	2	1	1
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	199	530	10	1	286	241	165	13	79	2	1	1
Added Vol:	0	700	0	0	64	54	272	0	0	0	0	0
Hyatt:	4	0	0	0	0	4	3	0	3	0	0	0
Initial Fut:	203	1230	10	1	350	299	440	13	82	2	1	1
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	203	1230	10	1	350	299	440	13	82	2	1	1
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	203	1230	10	1	350	299	440	13	82	2	1	1
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	203	1230	10	1	350	299	440	13	82	2	1	1

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	1.00	1.98	0.02	1.00	2.00	1.00	1.95	0.05	1.00	0.51	0.23	0.26
Final Sat.:	1750	3767	31	1750	3800	1750	3407	101	1750	893	446	446

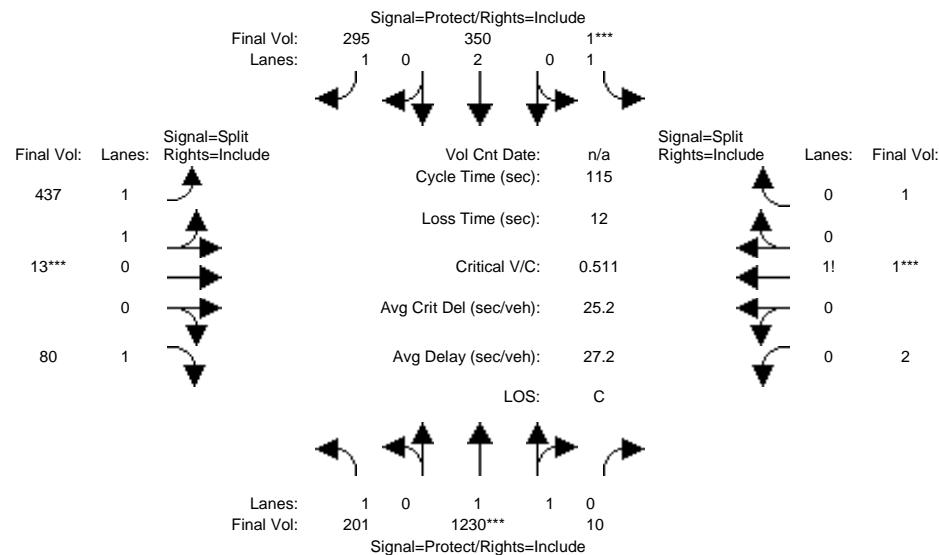
Capacity Analysis Module:

Vol/Sat:	0.12	0.33	0.33	0.00	0.09	0.17	0.13	0.13	0.05	0.00	0.00	0.00
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	30.4	68.1	68.1	7.0	44.7	44.7	26.9	26.9	26.9	10.0	10.0	10.0
Volume/Cap:	0.47	0.59	0.59	0.01	0.26	0.47	0.59	0.59	0.22	0.03	0.03	0.03
Uniform Del:	40.0	18.7	18.7	55.2	27.9	30.6	43.6	43.6	39.9	52.5	52.5	52.5
IncremntDel:	0.8	0.5	0.5	0.0	0.1	0.6	1.3	1.3	0.3	0.1	0.1	0.1
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	40.8	19.2	19.2	55.3	28.0	31.1	44.9	44.9	40.2	52.6	52.6	52.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	40.8	19.2	19.2	55.3	28.0	31.1	44.9	44.9	40.2	52.6	52.6	52.6
LOS by Move:	D	B	B	E	C	C	D	D	D	D	D	D
HCM2kAvgQ:	7	15	15	0	4	9	8	8	3	0	0	0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative AM

Intersection #31: Tantau Avenue and Vallco Parkway (CUP)



Street Name:	Tantau Avenue				Vallco Parkway							
	North Bound		South Bound		East Bound		West Bound					
Movement:	L -	T -	R	L -	T -	R	L -	T -	R	L -	T -	R
Min. Green:	7	10	10	7	10	10	10	10	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	201	530	10	1	286	241	165	13	80	2	1	1
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	201	530	10	1	286	241	165	13	80	2	1	1
Added Vol:	0	700	0	0	64	54	272	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	201	1230	10	1	350	295	437	13	80	2	1	1
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	201	1230	10	1	350	295	437	13	80	2	1	1
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	201	1230	10	1	350	295	437	13	80	2	1	1
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	201	1230	10	1	350	295	437	13	80	2	1	1

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	1.00	1.98	0.02	1.00	2.00	1.00	1.95	0.05	1.00	0.51	0.23	0.26
Final Sat.:	1750	3767	31	1750	3800	1750	3407	101	1750	893	446	446

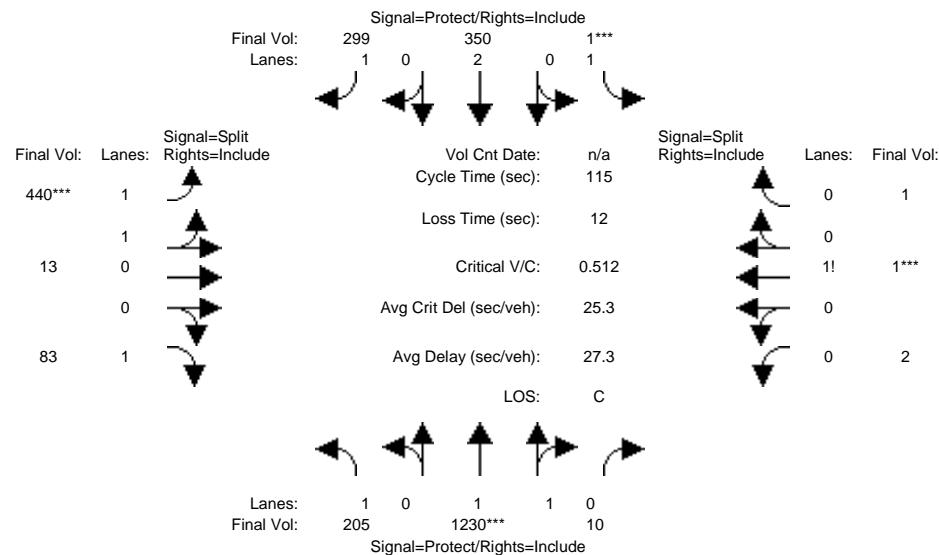
Capacity Analysis Module:

Vol/Sat:	0.11	0.33	0.33	0.00	0.09	0.17	0.13	0.13	0.05	0.00	0.00	0.00
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	27.9	61.7	61.7	7.0	40.9	40.9	24.3	24.3	24.3	10.0	10.0	10.0
Volume/Cap:	0.47	0.61	0.61	0.01	0.26	0.47	0.61	0.61	0.22	0.03	0.03	0.03
Delay/Veh:	38.1	18.8	18.8	50.8	26.4	29.3	42.5	42.5	37.8	48.1	48.1	48.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	38.1	18.8	18.8	50.8	26.4	29.3	42.5	42.5	37.8	48.1	48.1	48.1
LOS by Move:	D	B	B	D	C	C	D	D	D	D	D	D
HCM2kAvgQ:	6	14	14	0	4	8	8	8	2	0	0	0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + Proj AM

Intersection #31: Tantau Avenue and Vallco Parkway (CUP)



Street Name:	Tantau Avenue						Vallco Parkway								
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	10	10	10	10	10	10	10	10	
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	

Volume Module:

Base Vol:	201	530	10	1	286	241	165	13	80	2	1	1
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	201	530	10	1	286	241	165	13	80	2	1	1
Added Vol:	0	700	0	0	64	54	272	0	0	0	0	0
Hyatt:	4	0	0	0	0	4	3	0	3	0	0	0
Initial Fut:	205	1230	10	1	350	299	440	13	83	2	1	1
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	205	1230	10	1	350	299	440	13	83	2	1	1
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	205	1230	10	1	350	299	440	13	83	2	1	1
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	205	1230	10	1	350	299	440	13	83	2	1	1

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	1.00	1.98	0.02	1.00	2.00	1.00	1.95	0.05	1.00	0.51	0.23	0.26
Final Sat.:	1750	3767	31	1750	3800	1750	3407	101	1750	893	446	446

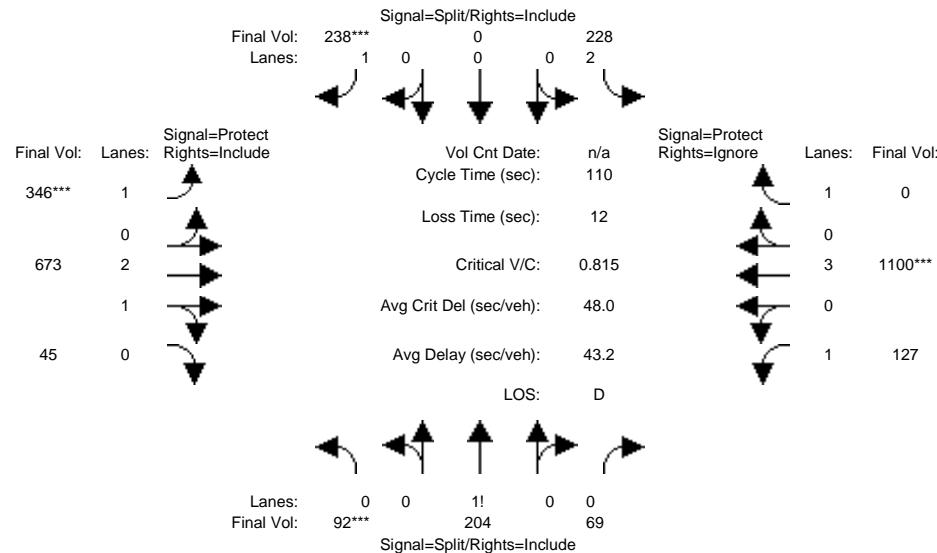
Capacity Analysis Module:

Vol/Sat:	0.12	0.33	0.33	0.00	0.09	0.17	0.13	0.13	0.05	0.00	0.00	0.00
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	27.9	61.6	61.6	7.0	40.7	40.7	24.4	24.4	24.4	10.0	10.0	10.0
Volume/Cap:	0.48	0.61	0.61	0.01	0.26	0.48	0.61	0.61	0.22	0.03	0.03	0.03
Delay/Veh:	38.2	18.9	18.9	50.8	26.5	29.5	42.5	42.5	37.8	48.1	48.1	48.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	38.2	18.9	18.9	50.8	26.5	29.5	42.5	42.5	37.8	48.1	48.1	48.1
LOS by Move:	D	B	B	D	C	C	D	D	D	D	D	D
HCM2kAvgQ:	6	14	14	0	4	9	8	8	2	0	0	0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd AM

Intersection #32: Stevens Creek Blvd and Tantau Avenue (CUP)



Street Name:	Tantau Avenue				Stevens Creek Blvd										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10		
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		

Volume Module:

Base Vol:	92	172	69	192	0	210	255	664	45	127	957	346			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	92	172	69	192	0	210	255	664	45	127	957	346			
Added Vol:	0	32	0	36	0	28	91	9	0	0	143	577			
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0			
Initial Fut:	92	204	69	228	0	238	346	673	45	127	1100	923			
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00		
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00		
PHF Volume:	92	204	69	228	0	238	346	673	45	127	1100	0			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	92	204	69	228	0	238	346	673	45	127	1100	0			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00		
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00		
FinalVolume:	92	204	69	228	0	238	346	673	45	127	1100	0			

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92		
Lanes:	0.26	0.54	0.20	2.00	0.00	1.00	1.00	2.80	0.20	1.00	3.00	1.00		
Final Sat.:	461	1023	346	3150	0	1750	1750	5314	355	1750	5700	1750		

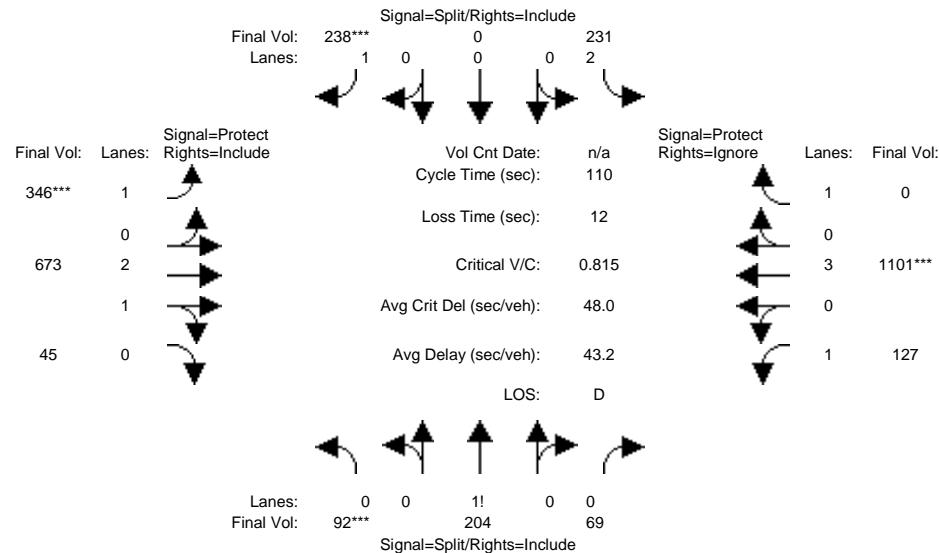
Capacity Analysis Module:

Vol/Sat:	0.20	0.20	0.20	0.07	0.00	0.14	0.20	0.13	0.13	0.07	0.19	0.00		
Crit Moves:	****			****	****	****	****	****	****	****	****	****		
Green Time:	26.9	26.9	26.9	18.4	0.0	18.4	26.7	33.5	33.5	19.2	26.0	0.0		
Volume/Cap:	0.81	0.81	0.81	0.43	0.00	0.81	0.81	0.42	0.42	0.42	0.81	0.00		
Uniform Del:	39.2	39.2	39.2	41.2	0.0	44.2	39.3	30.4	30.4	40.4	39.7	0.0		
IncremntDel:	11.0	11.0	11.0	0.6	0.0	16.0	11.5	0.2	0.2	0.9	3.9	0.0		
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Delay Adj:	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00		
Delay/Veh:	50.2	50.2	50.2	41.7	0.0	60.2	50.9	30.6	30.6	41.3	43.6	0.0		
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
AdjDel/Veh:	50.2	50.2	50.2	41.7	0.0	60.2	50.9	30.6	30.6	41.3	43.6	0.0		
LOS by Move:	D	D	D	D	A	E	D	C	C	D	D	A		
HCM2kAvgQ:	14	14	14	4	0	9	13	6	6	4	11	0		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + Proj AM

Intersection #32: Stevens Creek Blvd and Tantau Avenue (CUP)



Street Name:	Tantau Avenue				Stevens Creek Blvd										
	North Bound		South Bound		East Bound		West Bound								
Approach:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Movement:	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	7	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	92	172	69	192	0	210	255	664	45	127	957	346
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	92	172	69	192	0	210	255	664	45	127	957	346
Added Vol:	0	32	0	36	0	28	91	9	0	0	143	577
Hyatt:	0	0	0	3	0	0	0	0	0	0	1	4
Initial Fut:	92	204	69	231	0	238	346	673	45	127	1101	927
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Volume:	92	204	69	231	0	238	346	673	45	127	1101	0
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	92	204	69	231	0	238	346	673	45	127	1101	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
FinalVolume:	92	204	69	231	0	238	346	673	45	127	1101	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	0.26	0.54	0.20	2.00	0.00	1.00	1.00	2.80	0.20	1.00	3.00	1.00
Final Sat.:	461	1023	346	3150	0	1750	1750	5314	355	1750	5700	1750

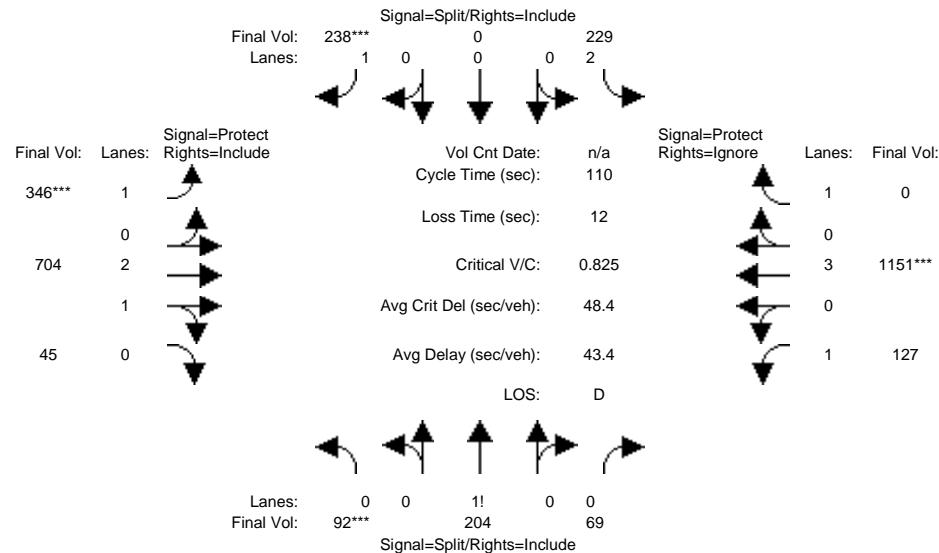
Capacity Analysis Module:

Vol/Sat:	0.20	0.20	0.20	0.07	0.00	0.14	0.20	0.13	0.13	0.07	0.19	0.00
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	26.9	26.9	26.9	18.4	0.0	18.4	26.7	33.5	33.5	19.2	26.1	0.0
Volume/Cap:	0.82	0.82	0.82	0.44	0.00	0.82	0.82	0.42	0.42	0.42	0.82	0.00
Uniform Del:	39.2	39.2	39.2	41.2	0.0	44.2	39.3	30.4	30.4	40.4	39.7	0.0
IncremntDel:	11.0	11.0	11.0	0.6	0.0	16.1	11.6	0.2	0.2	0.9	4.0	0.0
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Delay/Veh:	50.2	50.2	50.2	41.8	0.0	60.2	50.9	30.6	30.6	41.3	43.6	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	50.2	50.2	50.2	41.8	0.0	60.2	50.9	30.6	30.6	41.3	43.6	0.0
LOS by Move:	D	D	D	D	A	E	D	C	C	D	D	A
HCM2kAvgQ:	14	14	14	4	0	9	13	6	6	4	11	0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative AM

Intersection #32: Stevens Creek Blvd and Tantau Avenue (CUP)

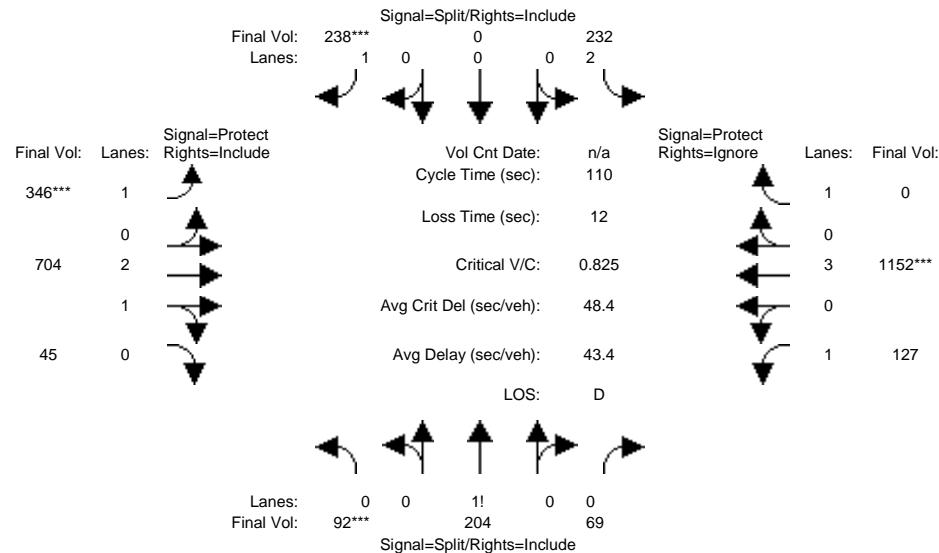


Street Name: Tantau Avenue Stevens Creek Blvd															
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	10		10		10		10		10		10		10		
Y+R:	4.0		4.0		4.0		4.0		4.0		4.0		4.0		
Volume Module:	<hr/>														
Base Vol:	92	172	69	193	0	210	255	695	45	127	1008	348			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	92	172	69	193	0	210	255	695	45	127	1008	348			
Added Vol:	0	32	0	36	0	28	91	9	0	0	143	577			
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0			
Initial Fut:	92	204	69	229	0	238	346	704	45	127	1151	925			
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00			
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00			
PHF Volume:	92	204	69	229	0	238	346	704	45	127	1151	0			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	92	204	69	229	0	238	346	704	45	127	1151	0			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00			
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00			
FinalVolume:	92	204	69	229	0	238	346	704	45	127	1151	0			
Saturation Flow Module:	<hr/>														
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900			
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92			
Lanes:	0.26	0.54	0.20	2.00	0.00	1.00	1.00	2.81	0.19	1.00	3.00	1.00			
Final Sat.:	461	1023	346	3150	0	1750	1750	5330	341	1750	5700	1750			
Capacity Analysis Module:	<hr/>														
Vol/Sat:	0.20	0.20	0.20	0.07	0.00	0.14	0.20	0.13	0.13	0.07	0.20	0.00			
Crit Moves:	****			****		****				****					
Green Time:	26.6	26.6	26.6	18.1	0.0	18.1	26.4	34.4	34.4	18.9	26.9	0.0			
Volume/Cap:	0.83	0.83	0.83	0.44	0.00	0.83	0.83	0.42	0.42	0.42	0.83	0.00			
Delay/Veh:	51.5	51.5	51.5	42.0	0.0	61.8	52.2	30.1	30.1	41.6	43.5	0.0			
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
AdjDel/Veh:	51.5	51.5	51.5	42.0	0.0	61.8	52.2	30.1	30.1	41.6	43.5	0.0			
LOS by Move:	D	D	D	D	A	E	D	C	C	D	D	A			
HCM2kAvgQ:	14	14	14	4	0	9	13	6	6	4	12	0			

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + Proj AM

Intersection #32: Stevens Creek Blvd and Tantau Avenue (CUP)



Street Name:	Tantau Avenue				Stevens Creek Blvd										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	10	10	10	10	10	10	7	10	10	10	7	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	92	172	69	193	0	210	255	695	45	127	1008	348
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	92	172	69	193	0	210	255	695	45	127	1008	348
Added Vol:	0	32	0	36	0	28	91	9	0	0	143	577
Hyatt:	0	0	0	3	0	0	0	0	0	0	1	4
Initial Fut:	92	204	69	232	0	238	346	704	45	127	1152	929
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Volume:	92	204	69	232	0	238	346	704	45	127	1152	0
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	92	204	69	232	0	238	346	704	45	127	1152	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
FinalVolume:	92	204	69	232	0	238	346	704	45	127	1152	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	0.26	0.54	0.20	2.00	0.00	1.00	1.00	2.81	0.19	1.00	3.00	1.00
Final Sat.:	461	1023	346	3150	0	1750	1750	5330	341	1750	5700	1750

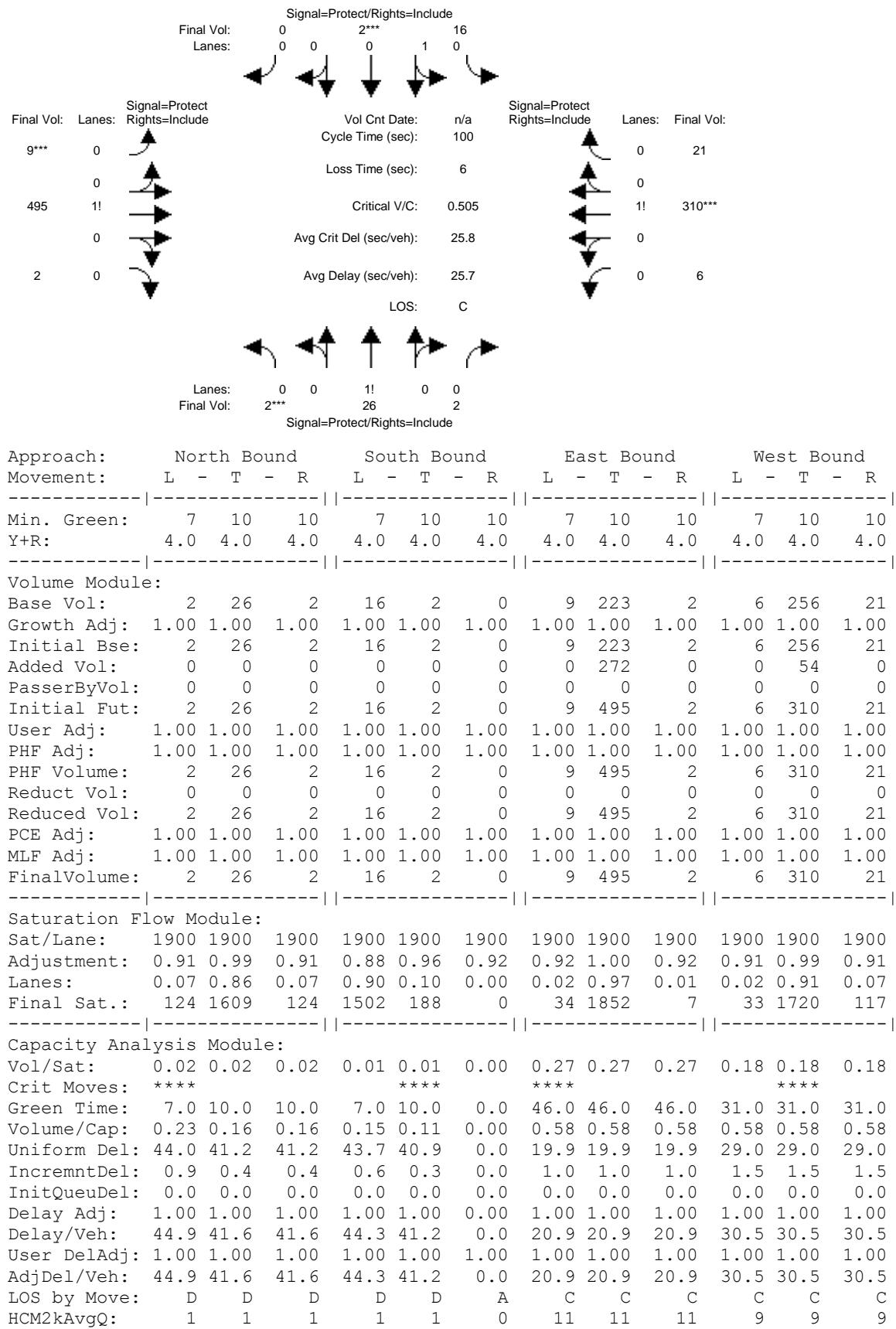
Capacity Analysis Module:

Vol/Sat:	0.20	0.20	0.20	0.07	0.00	0.14	0.20	0.13	0.13	0.07	0.20	0.00
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	26.6	26.6	26.6	18.1	0.0	18.1	26.4	34.4	34.4	18.9	26.9	0.0
Volume/Cap:	0.83	0.83	0.83	0.45	0.00	0.83	0.83	0.42	0.42	0.42	0.83	0.00
Delay/Veh:	51.5	51.5	51.5	42.0	0.0	61.8	52.2	30.1	30.1	41.6	43.5	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	51.5	51.5	51.5	42.0	0.0	61.8	52.2	30.1	30.1	41.6	43.5	0.0
LOS by Move:	D	D	D	D	A	E	D	C	C	D	D	A
HCM2kAvgQ:	14	14	14	4	0	9	13	6	6	4	12	0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd AM

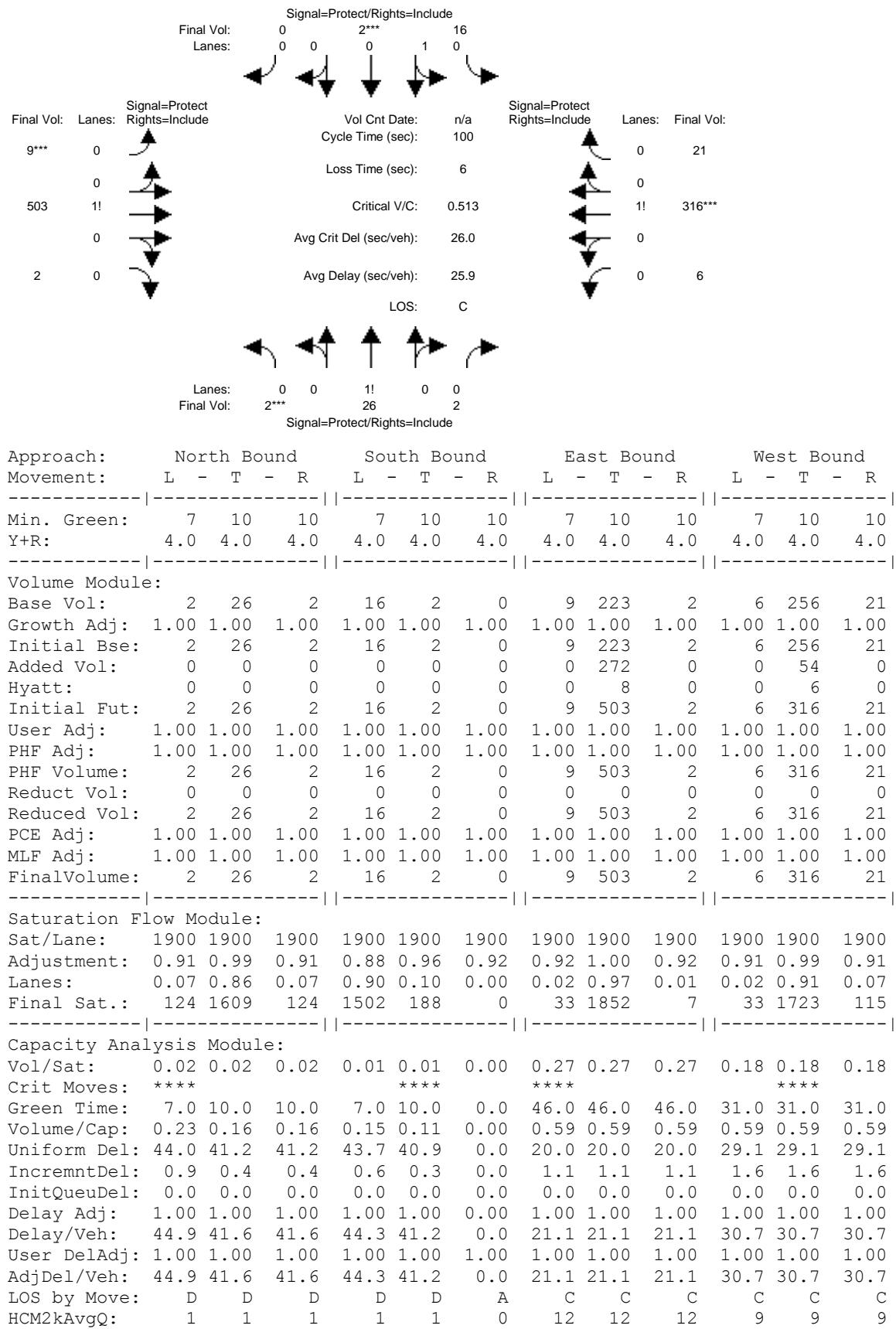
Intersection #501: Finch Road and Vallco Pkey (CUP)



Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + Proj AM

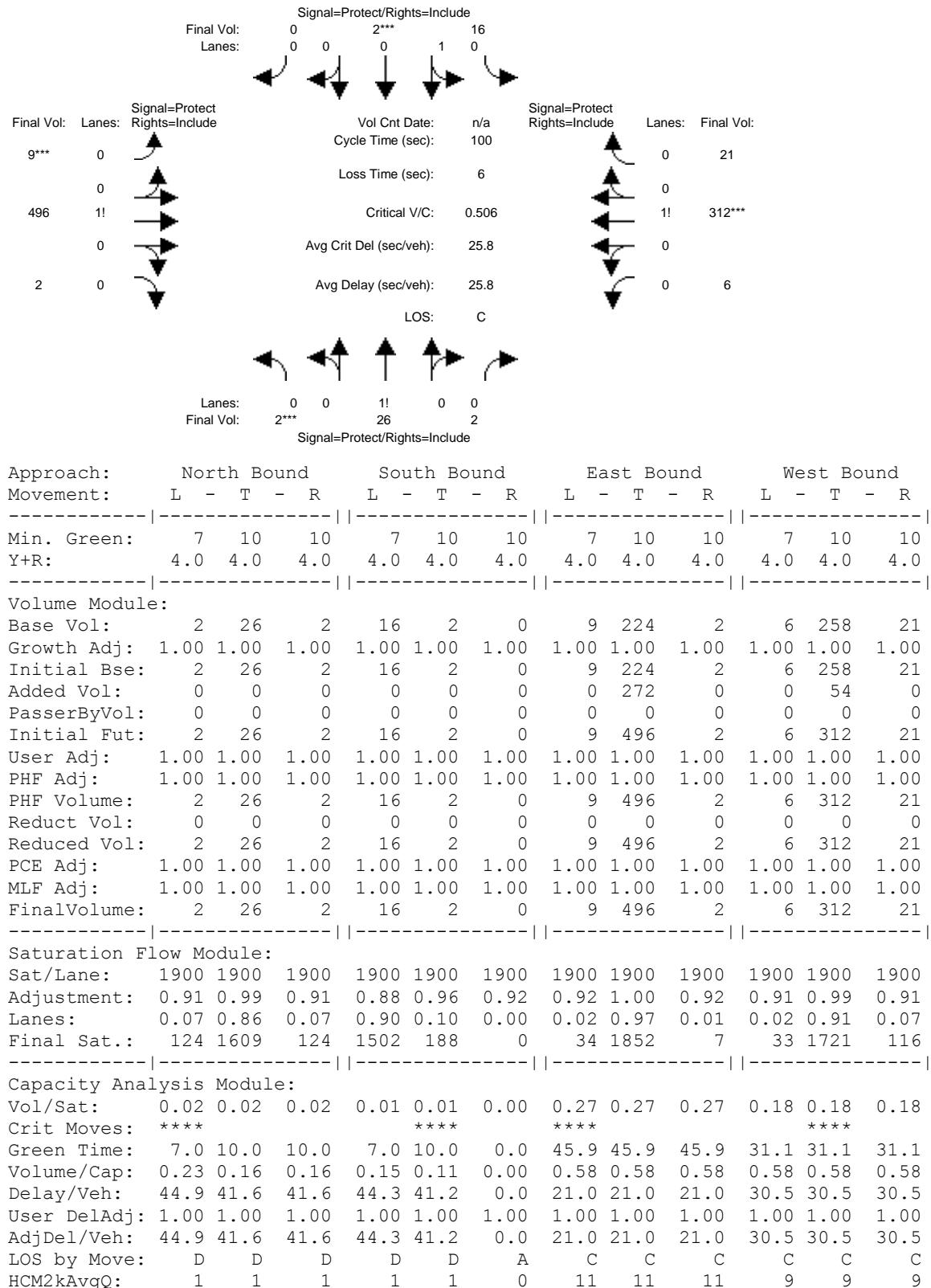
Intersection #501: Finch Road and Valco Pkey (CUP)



Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative AM

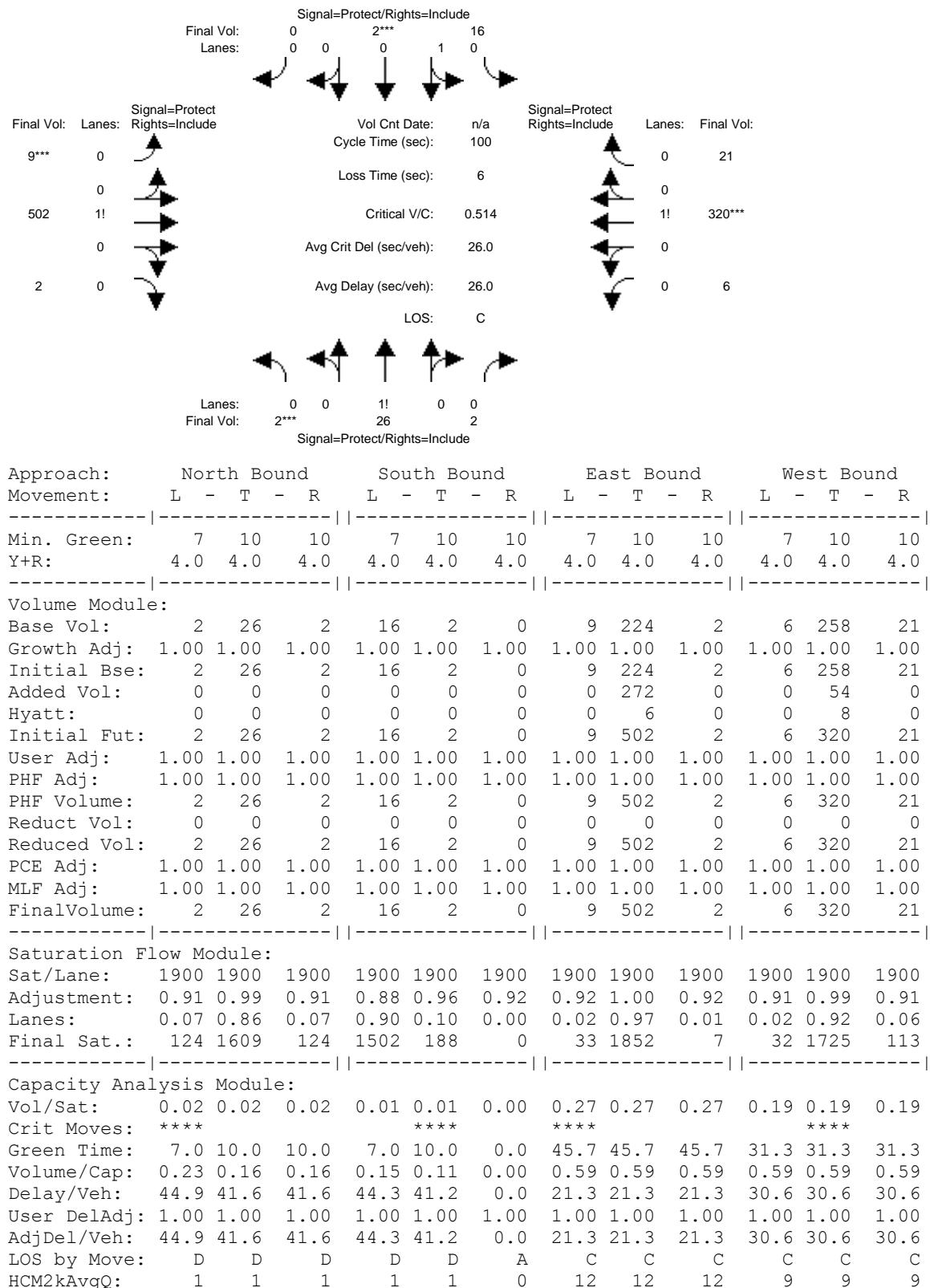
Intersection #501: Finch Road and Vallco Pkey (CUP)



Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + Proj AM

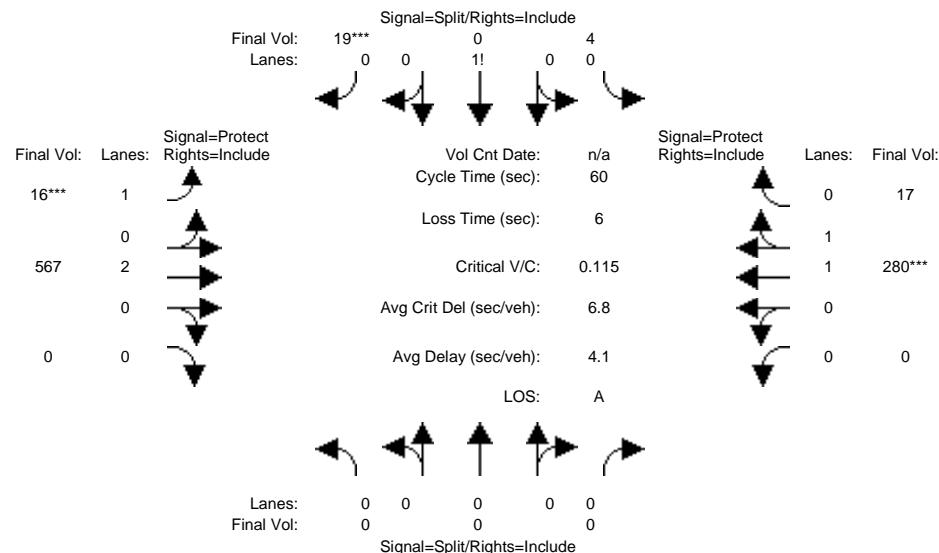
Intersection #501: Finch Road and Vallco Pkey (CUP)



Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd AM

Intersection #504: Perimeter Rd & Vallco Pkwy



Street Name:	Perimeter Rd				Vallco Pkwy										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	7	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	0	0	4	0	19	16	295	0	0	226	17				
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
Initial Bse:	0	0	0	4	0	19	16	295	0	0	226	17				
Added Vol:	0	0	0	0	0	0	0	272	0	0	54	0				
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0				
Initial Fut:	0	0	0	4	0	19	16	567	0	0	280	17				
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
PHF Volume:	0	0	0	4	0	19	16	567	0	0	280	17				
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0				
Reduced Vol:	0	0	0	4	0	19	16	567	0	0	280	17				
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
FinalVolume:	0	0	0	4	0	19	16	567	0	0	280	17				

Saturation Flow Module:

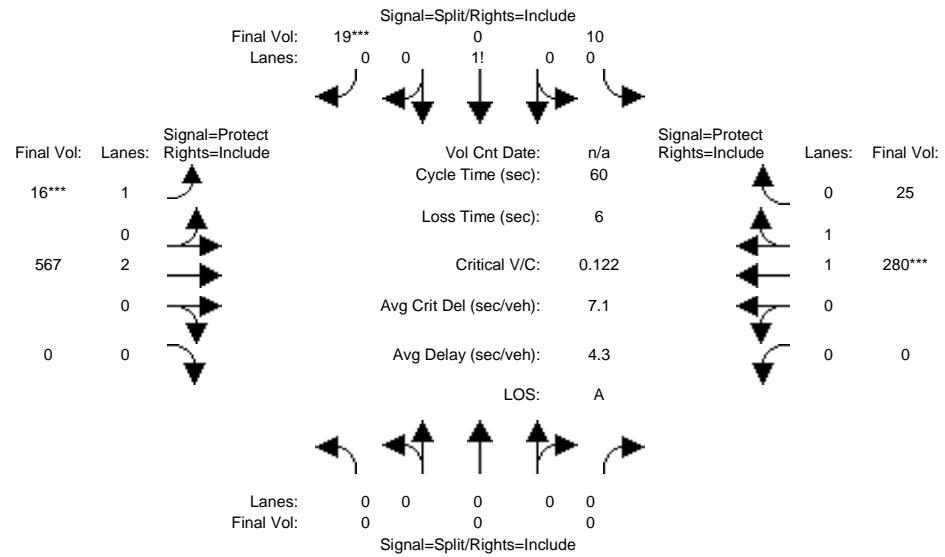
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.81	1.00	0.81	0.88	1.00	0.92	0.92	0.99	0.91
Lanes:	0.00	0.00	0.00	0.17	0.00	0.83	1.00	2.00	0.00	0.00	1.88	0.12
Final Sat.:	0	0	0	268	0	1272	1663	3800	0	0	3533	214

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.01	0.00	0.01	0.01	0.15	0.00	0.00	0.08	0.08
Crit Moves:				****	****						****	
Green Time:	0.0	0.0	0.0	10.0	0.0	10.0	7.0	44.0	0.0	0.0	37.0	37.0
Volume/Cap:	0.00	0.00	0.00	0.09	0.00	0.09	0.08	0.20	0.00	0.00	0.13	0.13
Uniform Del:	0.0	0.0	0.0	21.1	0.0	21.1	23.6	2.5	0.0	0.0	4.8	4.8
IncremntDel:	0.0	0.0	0.0	0.2	0.0	0.2	0.2	0.0	0.0	0.0	0.0	0.0
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00
Delay/Veh:	0.0	0.0	0.0	21.3	0.0	21.3	23.8	2.5	0.0	0.0	4.8	4.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	21.3	0.0	21.3	23.8	2.5	0.0	0.0	4.8	4.8
LOS by Move:	A	A	A	C	A	C	C	A	A	A	A	A
HCM2kAvgQ:	0	0	0	0	0	0	0	2	0	0	1	1

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + Proj AM

Intersection #504: Perimeter Rd & Vallco Pkwy

Street Name:			Perimeter Rd						Vallco Pkwy						
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10		
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		

Volume Module:

Base Vol:	0	0	0	4	0	19	16	295	0	0	226	17
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	4	0	19	16	295	0	0	226	17
Added Vol:	0	0	0	0	0	0	0	272	0	0	54	0
Hyatt:	0	0	0	6	0	0	0	0	0	0	0	8
Initial Fut:	0	0	0	10	0	19	16	567	0	0	280	25
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	10	0	19	16	567	0	0	280	25
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	10	0	19	16	567	0	0	280	25
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	10	0	19	16	567	0	0	280	25

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.83	0.88	1.00	0.92	0.92	0.99	0.91
Lanes:	0.00	0.00	0.00	0.34	0.00	0.66	1.00	2.00	0.00	0.00	1.82	0.18
Final Sat.:	0	0	0	541	0	1028	1663	3800	0	0	3423	306

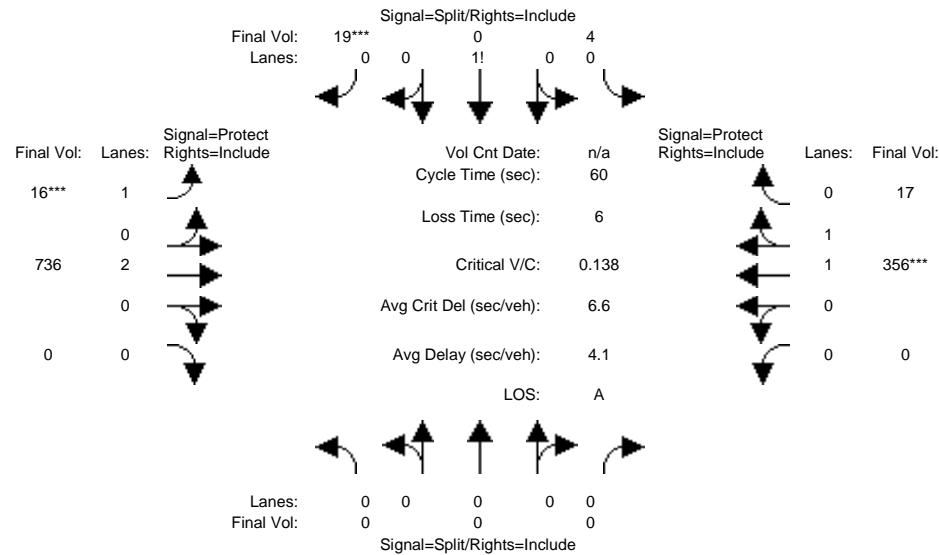
Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.02	0.00	0.02	0.01	0.15	0.00	0.00	0.08	0.08
Crit Moves:							****	****				****
Green Time:	0.0	0.0	0.0	10.0	0.0	10.0	7.0	44.0	0.0	0.0	37.0	37.0
Volume/Cap:	0.00	0.00	0.00	0.11	0.00	0.11	0.08	0.20	0.00	0.00	0.13	0.13
Uniform Del:	0.0	0.0	0.0	21.2	0.0	21.2	23.6	2.5	0.0	0.0	4.8	4.8
IncremntDel:	0.0	0.0	0.0	0.2	0.0	0.2	0.2	0.0	0.0	0.0	0.0	0.0
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00
Delay/Veh:	0.0	0.0	0.0	21.4	0.0	21.4	23.8	2.5	0.0	0.0	4.8	4.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	21.4	0.0	21.4	23.8	2.5	0.0	0.0	4.8	4.8
LOS by Move:	A	A	A	C	A	C	C	A	A	A	A	A
HCM2kAvgQ:	0	0	0	1	0	1	0	2	0	0	1	1

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative AM

Intersection #504: Perimeter Rd & Vallco Pkwy



Street Name:	Perimeter Rd				Vallco Pkwy										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	0	0	4	0	19	16	464	0	0	302	17
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	4	0	19	16	464	0	0	302	17
Added Vol:	0	0	0	0	0	0	0	272	0	0	54	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	4	0	19	16	736	0	0	356	17
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	4	0	19	16	736	0	0	356	17
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	4	0	19	16	736	0	0	356	17
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	4	0	19	16	736	0	0	356	17

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.81	1.00	0.81	0.88	1.00	0.92	0.92	0.99	0.91
Lanes:	0.00	0.00	0.00	0.17	0.00	0.83	1.00	2.00	0.00	0.00	1.90	0.10
Final Sat.:	0	0	0	268	0	1272	1663	3800	0	0	3587	171

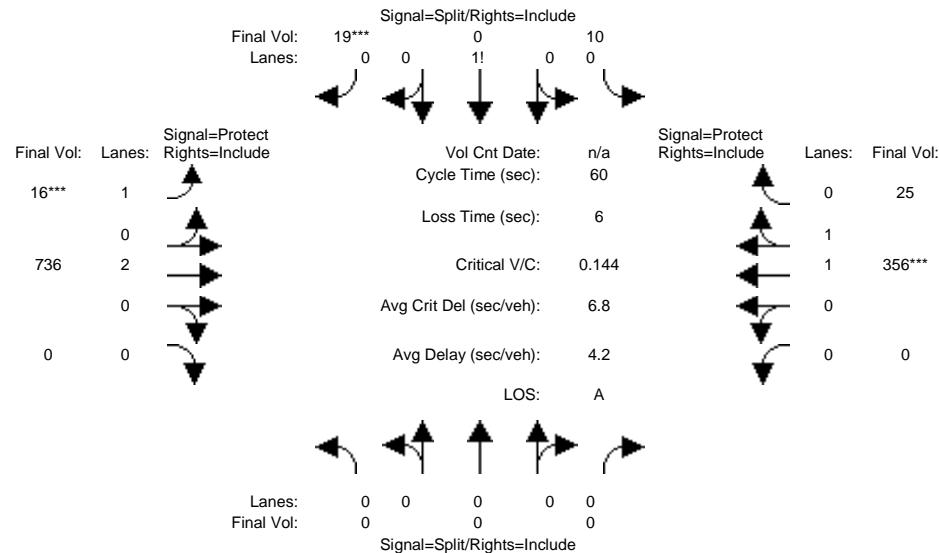
Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.01	0.00	0.01	0.01	0.19	0.00	0.00	0.10	0.10
Crit Moves:				****		****					****	
Green Time:	0.0	0.0	0.0	10.0	0.0	10.0	7.0	44.0	0.0	0.0	37.0	37.0
Volume/Cap:	0.00	0.00	0.00	0.09	0.00	0.09	0.08	0.26	0.00	0.00	0.16	0.16
Delay/Veh:	0.0	0.0	0.0	21.3	0.0	21.3	23.8	2.7	0.0	0.0	4.9	4.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	21.3	0.0	21.3	23.8	2.7	0.0	0.0	4.9	4.9
LOS by Move:	A	A	A	C	A	C	C	A	A	A	A	A
HCM2kAvgQ:	0	0	0	0	0	0	0	2	0	0	1	1

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + Proj AM

Intersection #504: Perimeter Rd & Vallco Pkwy



Street Name:	Perimeter Rd				Vallco Pkwy										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	7	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	0	0	4	0	19	16	464	0	0	302	17
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	4	0	19	16	464	0	0	302	17
Added Vol:	0	0	0	0	0	0	0	272	0	0	54	0
Hyatt:	0	0	0	6	0	0	0	0	0	0	0	8
Initial Fut:	0	0	0	10	0	19	16	736	0	0	356	25
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	10	0	19	16	736	0	0	356	25
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	10	0	19	16	736	0	0	356	25
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	10	0	19	16	736	0	0	356	25

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.83	0.88	1.00	0.92	0.92	0.99	0.91
Lanes:	0.00	0.00	0.00	0.34	0.00	0.66	1.00	2.00	0.00	0.00	1.86	0.14
Final Sat.:	0	0	0	541	0	1028	1663	3800	0	0	3495	245

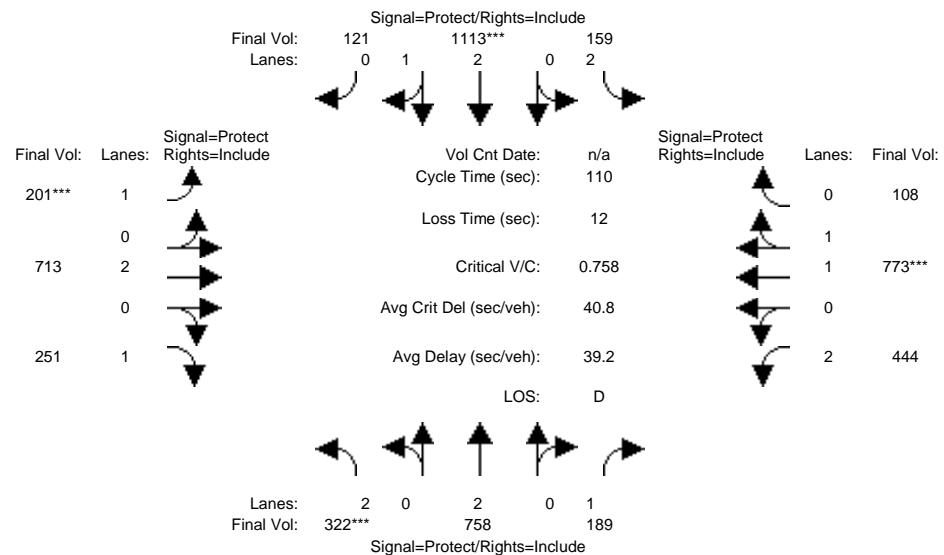
Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.02	0.00	0.02	0.01	0.19	0.00	0.00	0.10	0.10
Crit Moves:						****	****				****	
Green Time:	0.0	0.0	0.0	10.0	0.0	10.0	7.0	44.0	0.0	0.0	37.0	37.0
Volume/Cap:	0.00	0.00	0.00	0.11	0.00	0.11	0.08	0.26	0.00	0.00	0.17	0.17
Delay/Veh:	0.0	0.0	0.0	21.4	0.0	21.4	23.8	2.7	0.0	0.0	4.9	4.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	21.4	0.0	21.4	23.8	2.7	0.0	0.0	4.9	4.9
LOS by Move:	A	A	A	C	A	C	C	A	A	A	A	A
HCM2kAvgQ:	0	0	0	1	0	1	0	2	0	0	1	1

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Existing PM

Intersection #18: Wolfe Road and Homestead Road (CUP)



Street Name:	Wolfe Road						Homestead Road								
	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10		
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		

Volume Module:

Base Vol:	322	758	189	159	1113	121	201	713	251	444	773	108
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	322	758	189	159	1113	121	201	713	251	444	773	108
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	322	758	189	159	1113	121	201	713	251	444	773	108
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	322	758	189	159	1113	121	201	713	251	444	773	108
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	322	758	189	159	1113	121	201	713	251	444	773	108
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	322	758	189	159	1113	121	201	713	251	444	773	108

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.83	1.00	0.92	0.83	0.99	0.95	0.92	1.00	0.92	0.83	0.98	0.95
Lanes:	2.00	2.00	1.00	2.00	2.69	0.31	1.00	2.00	1.00	2.00	1.75	0.25
Final Sat.:	3150	3800	1750	3150	5050	549	1750	3800	1750	3150	3246	454

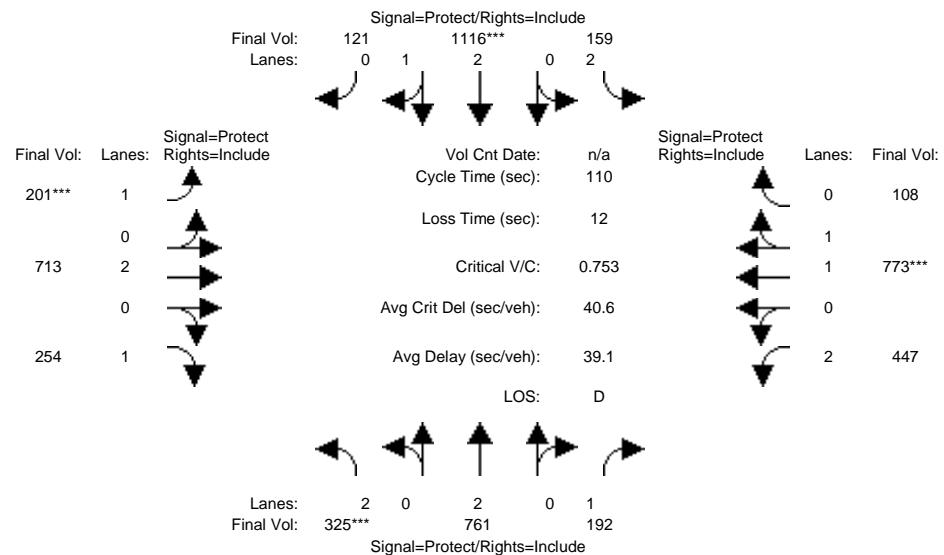
Capacity Analysis Module:

Vol/Sat:	0.10	0.20	0.11	0.05	0.22	0.22	0.11	0.19	0.14	0.14	0.24	0.24
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	14.8	35.5	35.5	11.3	32.0	32.0	16.7	29.2	29.2	22.0	34.5	34.5
Volume/Cap:	0.76	0.62	0.33	0.49	0.76	0.76	0.76	0.71	0.54	0.71	0.76	0.76
Uniform Del:	45.9	31.5	28.3	46.6	35.5	35.5	44.7	36.5	34.6	41.0	34.0	34.0
IncremntDel:	7.7	1.0	0.4	1.2	2.1	2.1	11.9	2.3	1.3	3.7	2.9	2.9
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	53.6	32.5	28.7	47.8	37.6	37.6	56.7	38.8	35.9	44.7	36.9	36.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	53.6	32.5	28.7	47.8	37.6	37.6	56.7	38.8	35.9	44.7	36.9	36.9
LOS by Move:	D	C	C	D	D	D	E	D	D	D	D	D
HCM2kAvgQ:	7	11	5	3	13	13	7	11	8	8	14	14

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Ex + P PM

Intersection #18: Wolfe Road and Homestead Road (CUP)

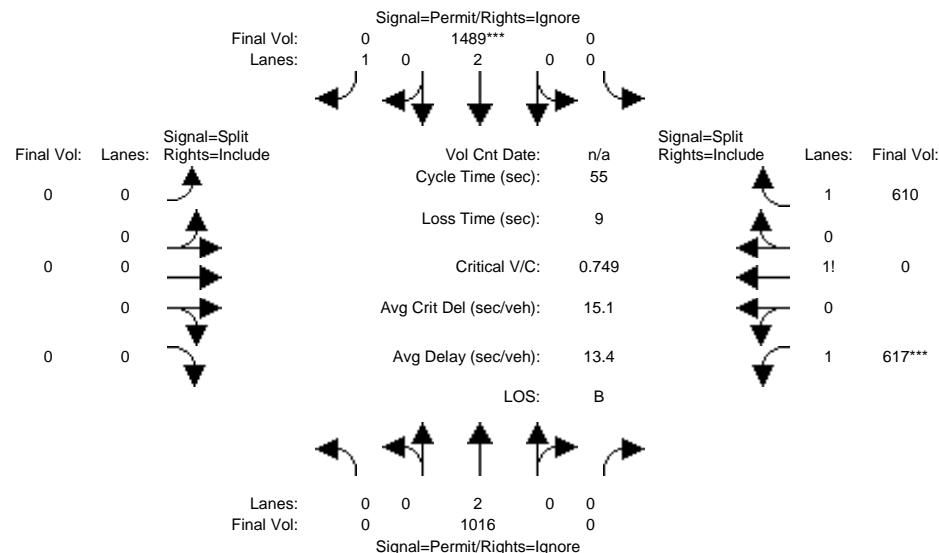


Street Name: Wolfe Road Homestead Road													
Approach:	North Bound			South Bound			East Bound			West Bound			
	Movement:	L -	T -	R	L -	T -	R	L -	T -	R	L -	T -	R
Min. Green:		7	10	10	7	10	10	7	10	10	7	10	10
Y+R:		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Volume Module:													
Base Vol:		322	758	189	159	1113	121	201	713	251	444	773	108
Growth Adj:		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:		322	758	189	159	1113	121	201	713	251	444	773	108
Added Vol:		0	0	0	0	0	0	0	0	0	0	0	0
Hyatt:		3	3	3	0	3	0	0	0	3	3	0	0
Initial Fut:		325	761	192	159	1116	121	201	713	254	447	773	108
User Adj:		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:		325	761	192	159	1116	121	201	713	254	447	773	108
Reduc Vol:		0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:		325	761	192	159	1116	121	201	713	254	447	773	108
PCE Adj:		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:		325	761	192	159	1116	121	201	713	254	447	773	108
Saturation Flow Module:													
Sat/Lane:		1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:		0.83	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92
Lanes:		2.00	2.00	1.00	2.00	2.68	0.32	1.00	2.00	1.00	2.00	1.74	0.26
Final Sat.:		3150	3800	1750	3150	5100	553	1750	3800	1750	3150	3299	461
Capacity Analysis Module:													
Vol/Sat:		0.10	0.20	0.11	0.05	0.22	0.22	0.11	0.19	0.15	0.14	0.23	0.23
Crit Moves:		****	****	****	****	****	****	****	****	****	****	****	****
Green Time:		15.1	35.7	35.7	11.3	32.0	32.0	16.8	29.0	29.0	22.0	34.2	34.2
Volume/Cap:		0.75	0.62	0.34	0.49	0.75	0.75	0.75	0.71	0.55	0.71	0.75	0.75
Delay/Veh:		53.0	32.3	28.6	47.8	37.5	37.5	56.1	39.1	36.3	44.9	36.9	36.9
User DelAdj:		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:		53.0	32.3	28.6	47.8	37.5	37.5	56.1	39.1	36.3	44.9	36.9	36.9
LOS by Move:		D	C	C	D	D	D	E	D	D	D	D	D
HCM2kAvgQ:		7	11	5	3	13	13	7	11	8	8	14	14

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Existing PM

Intersection #21: Wolfe Road and I-280 NB Ramps (CUP/CMP)



Street Name:		Wolfe Road						I-280 NB Ramps					
Approach:	North Bound	South Bound			East Bound			West Bound					
Movement:	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R
Min. Green:	7 10	10	7 10	10	0	0	0	0	10 10	10	10	10	10
Y+R:	4.0 4.0	4.0	4.0 4.0	4.0	4.0	4.0	4.0	4.0	4.0 4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	1016	440	0	1489	702	0	0	0	617	0	610
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1016	440	0	1489	702	0	0	0	617	0	610
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1016	440	0	1489	702	0	0	0	617	0	610
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1016	0	0	1489	0	0	0	0	617	0	610
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1016	0	0	1489	0	0	0	0	617	0	610
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1016	0	0	1489	0	0	0	0	617	0	610

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.97	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	0.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	1.50	0.00	1.50
Final Sat.:	0	3700	0	0	3800	1750	0	0	0	2630	0	2620

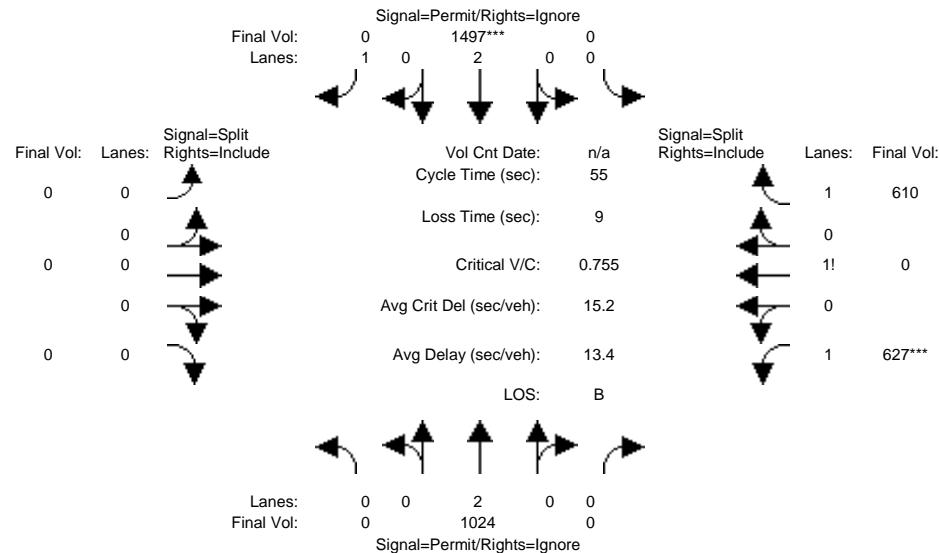
Capacity Analysis Module:

Vol/Sat:	0.00	0.27	0.00	0.00	0.39	0.00	0.00	0.00	0.00	0.23	0.00	0.23
Crit Moves:					****					****		
Green Time:	0.0	28.8	0.0	0.0	28.8	0.0	0.0	0.0	0.0	17.2	0.0	17.2
Volume/Cap:	0.00	0.52	0.00	0.00	0.75	0.00	0.00	0.00	0.00	0.75	0.00	0.74
Uniform Del:	0.0	8.6	0.0	0.0	10.3	0.0	0.0	0.0	0.0	16.9	0.0	16.9
IncremntDel:	0.0	0.3	0.0	0.0	1.6	0.0	0.0	0.0	0.0	2.0	0.0	1.9
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	0.00	1.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	0.00	1.00
Delay/Veh:	0.0	8.9	0.0	0.0	11.9	0.0	0.0	0.0	0.0	18.9	0.0	18.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	8.9	0.0	0.0	11.9	0.0	0.0	0.0	0.0	18.9	0.0	18.8
LOS by Move:	A	A	A	A	B	A	A	A	A	B	A	B
HCM2kAvgQ:	0	6	0	0	10	0	0	0	0	9	0	9

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Ex + P PM

Intersection #21: Wolfe Road and I-280 NB Ramps (CUP/CMP)



Street Name:		Wolfe Road						I-280 NB Ramps								
Approach:		North Bound			South Bound			East Bound			West Bound					
Movement:		L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:		7	10	10	7	10	10	0	0	0	0	10	10	10	10	
Y+R:		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	

Volume Module:

Base Vol:	0	1016	440	0	1489	702	0	0	0	617	0	610
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1016	440	0	1489	702	0	0	0	617	0	610
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Hyatt:	0	8	0	0	8	0	0	0	0	10	0	0
Initial Fut:	0	1024	440	0	1497	702	0	0	0	627	0	610
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1024	0	0	1497	0	0	0	0	627	0	610
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1024	0	0	1497	0	0	0	0	627	0	610
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1024	0	0	1497	0	0	0	0	627	0	610

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	0.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	1.51	0.00	1.49
Final Sat.:	0	3800	0	0	3800	1750	0	0	0	2637	0	2613

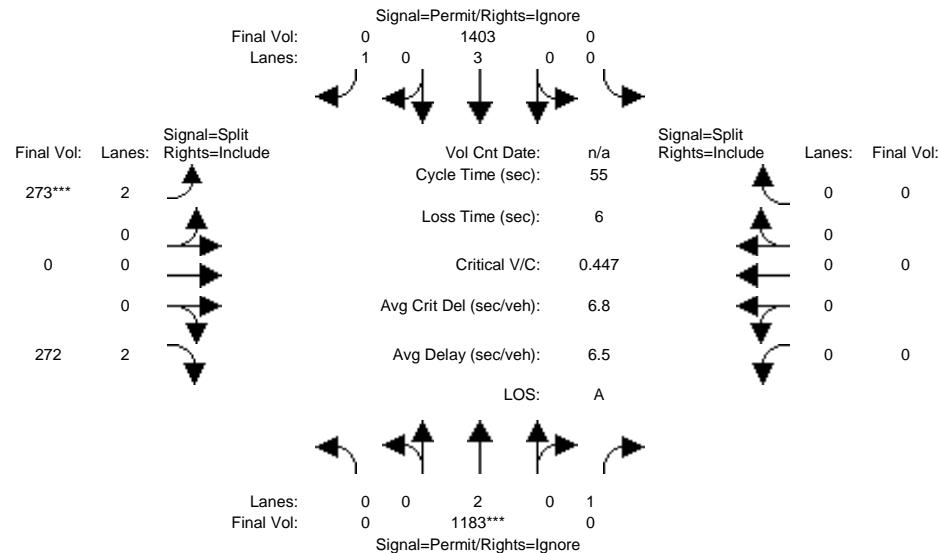
Capacity Analysis Module:

Vol/Sat:	0.00	0.27	0.00	0.00	0.39	0.00	0.00	0.00	0.00	0.24	0.00	0.23
Crit Moves:	*****											
Green Time:	0.0	28.7	0.0	0.0	28.7	0.0	0.0	0.0	0.0	17.3	0.0	17.3
Volume/Cap:	0.00	0.52	0.00	0.00	0.76	0.00	0.00	0.00	0.00	0.76	0.00	0.74
Delay/Veh:	0.0	8.9	0.0	0.0	12.1	0.0	0.0	0.0	0.0	19.0	0.0	18.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	8.9	0.0	0.0	12.1	0.0	0.0	0.0	0.0	19.0	0.0	18.7
LOS by Move:	A	A	A	A	B	A	A	A	A	B	A	B
HCM2kAvgQ:	0	6	0	0	10	0	0	0	0	9	0	9

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Existing PM

Intersection #22: Wolfe Road and I-280 SB Ramps (CUP/CMP)



Street Name:		Wolfe Road				I-280 SB Ramps			
Approach:	North Bound	South Bound		East Bound		West Bound			
Movement:	L - T - R	L - T - R		L - T - R		L - T - R		L - T - R	
Min. Green:	7	10	10	7	10	10	10	10	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	1183	560	0	1403	703	273	0	272	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1183	560	0	1403	703	273	0	272	0	0	0
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1183	560	0	1403	703	273	0	272	0	0	0
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1183	0	0	1403	0	273	0	272	0	0	0
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1183	0	0	1403	0	273	0	272	0	0	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1183	0	0	1403	0	273	0	272	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.83	0.92	1.00
Lanes:	0.00	2.00	1.00	0.00	3.00	1.00	2.00	0.00	2.00	0.00	0.00
Final Sat.:	0	3800	1750	0	5700	1750	3150	0	3150	0	0

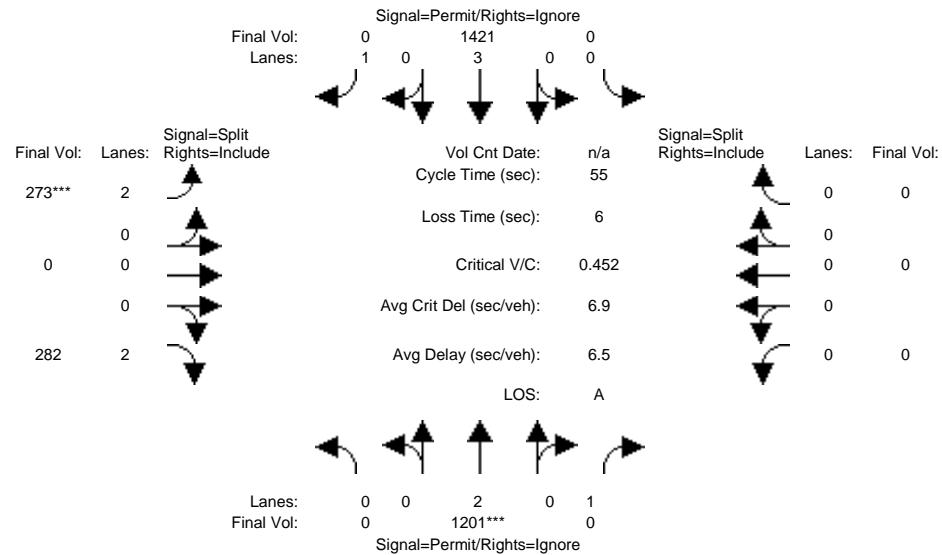
Capacity Analysis Module:

Vol/Sat:	0.00	0.31	0.00	0.00	0.25	0.00	0.09	0.00	0.09	0.00	0.00	0.00
Crit Moves:	****						****					
Green Time:	0.0	38.3	0.0	0.0	38.3	0.0	10.7	0.0	10.7	0.0	0.0	0.0
Volume/Cap:	0.00	0.45	0.00	0.00	0.35	0.00	0.45	0.00	0.45	0.00	0.00	0.00
Uniform Del:	0.0	3.7	0.0	0.0	3.4	0.0	19.6	0.0	19.6	0.0	0.0	0.0
IncremntDel:	0.0	0.1	0.0	0.0	0.1	0.0	0.5	0.0	0.5	0.0	0.0	0.0
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	0.00	1.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00	0.00
Delay/Veh:	0.0	3.8	0.0	0.0	3.4	0.0	20.1	0.0	20.1	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	3.8	0.0	0.0	3.4	0.0	20.1	0.0	20.1	0.0	0.0	0.0
LOS by Move:	A	A	A	A	A	A	C	A	C	A	A	A
HCM2kAvgQ:	0	4	0	0	3	0	3	0	3	0	0	0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
 2000 HCM Operations (Future Volume Alternative)
 Ex + P PM

Intersection #22: Wolfe Road and I-280 SB Ramps (CUP/CMP)

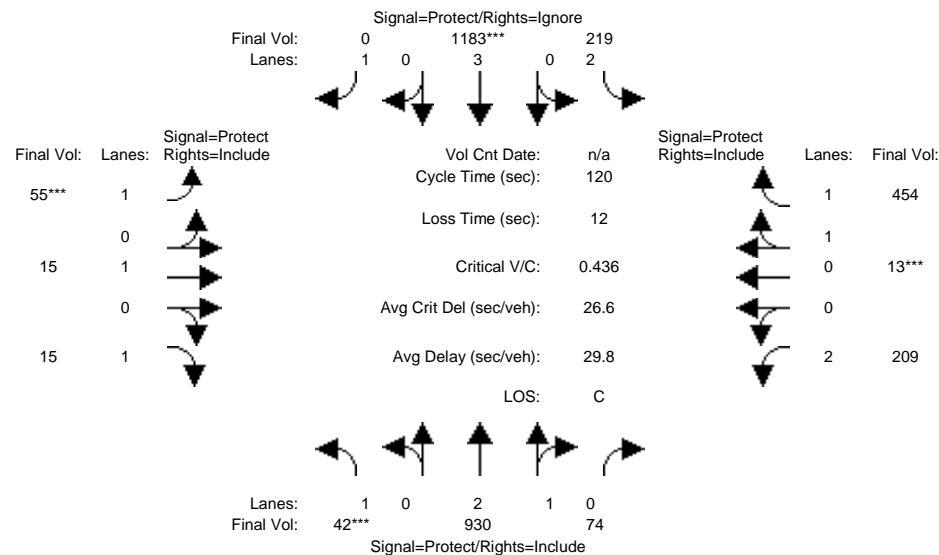


Street Name: Wolfe Road I-280 SB Ramps															
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	10	10	10	10	0	0	0	0	
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Volume Module:															
Base Vol:	0	1183	560	0	1403	703	273	0	272	0	0	0	0	0	
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Initial Bse:	0	1183	560	0	1403	703	273	0	272	0	0	0	0	0	
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Hyatt:	0	18	9	0	18	0	0	0	10	0	0	0	0	0	
Initial Fut:	0	1201	569	0	1421	703	273	0	282	0	0	0	0	0	
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
PHF Volume:	0	1201	0	0	1421	0	273	0	282	0	0	0	0	0	
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Reduced Vol:	0	1201	0	0	1421	0	273	0	282	0	0	0	0	0	
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
FinalVolume:	0	1201	0	0	1421	0	273	0	282	0	0	0	0	0	
Saturation Flow Module:															
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.83	0.92	1.00	0.92	0.92		
Lanes:	0.00	2.00	1.00	0.00	3.00	1.00	2.00	0.00	2.00	0.00	0.00	0.00	0.00		
Final Sat.:	0	3800	1750	0	5700	1750	3150	0	3150	0	0	0	0	0	
Capacity Analysis Module:															
Vol/Sat:	0.00	0.32	0.00	0.00	0.25	0.00	0.09	0.00	0.09	0.00	0.00	0.00	0.00		
Crit Moves:	****														
Green Time:	0.0	38.2	0.0	0.0	38.2	0.0	10.8	0.0	10.8	0.0	0.0	0.0	0.0		
Volume/Cap:	0.00	0.46	0.00	0.00	0.36	0.00	0.44	0.00	0.46	0.00	0.00	0.00	0.00		
Delay/Veh:	0.0	3.9	0.0	0.0	3.5	0.0	19.9	0.0	20.0	0.0	0.0	0.0	0.0		
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
AdjDel/Veh:	0.0	3.9	0.0	0.0	3.5	0.0	19.9	0.0	20.0	0.0	0.0	0.0	0.0		
LOS by Move:	A	A	A	A	A	A	B	A	C	A	A	A	A		
HCM2kAvgQ:	0	5	0	0	3	0	3	0	3	0	0	0	0		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Existing PM

Intersection #23: Wolfe Road and Vallco Parkway (CUP)



Street Name:	Wolfe Road						Vallco Parkway								
	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10		
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		

Volume Module:

Base Vol:	42	930	74	219	1183	52	55	15	15	209	13	454
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	42	930	74	219	1183	52	55	15	15	209	13	454
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	42	930	74	219	1183	52	55	15	15	209	13	454
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	42	930	74	219	1183	0	55	15	15	209	13	454
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	42	930	74	219	1183	0	55	15	15	209	13	454
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	42	930	74	219	1183	0	55	15	15	209	13	454

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.99	0.95	0.83	1.00	0.92	0.92	1.00	0.92	0.83	0.95	0.95
Lanes:	1.00	2.77	0.23	2.00	3.00	1.00	1.00	1.00	1.00	2.00	0.06	1.94
Final Sat.:	1750	5187	413	3150	5700	1750	1750	1900	1750	3150	100	3500

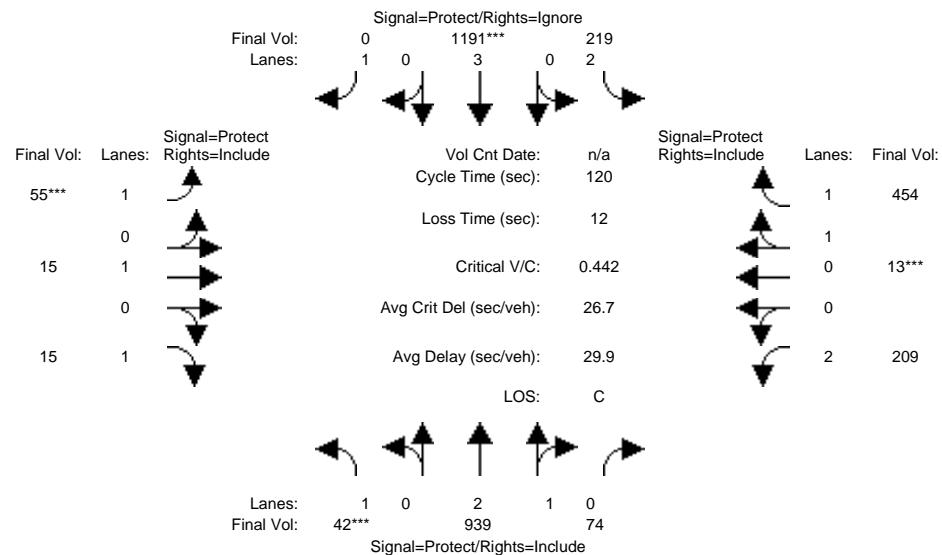
Capacity Analysis Module:

Vol/Sat:	0.02	0.18	0.18	0.07	0.21	0.00	0.03	0.01	0.01	0.07	0.13	0.13
Crit Moves:	****		****		****		****		****		****	
Green Time:	7.0	46.0	46.0	17.8	56.9	0.0	8.6	24.6	24.6	19.6	35.5	35.5
Volume/Cap:	0.41	0.47	0.47	0.47	0.44	0.00	0.44	0.04	0.04	0.41	0.44	0.44
Uniform Del:	54.5	27.8	27.8	46.7	21.0	0.0	53.4	38.2	38.3	45.0	34.2	34.2
IncremntDel:	2.7	0.2	0.2	0.7	0.1	0.0	2.4	0.0	0.0	0.5	0.3	0.3
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	57.2	28.0	28.0	47.5	21.1	0.0	55.8	38.3	38.3	45.5	34.4	34.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	57.2	28.0	28.0	47.5	21.1	0.0	55.8	38.3	38.3	45.5	34.4	34.4
LOS by Move:	E	C	C	D	C	A	E	D	D	D	C	C
HCM2kAvgQ:	2	9	9	4	9	0	3	0	0	4	7	7

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Ex + P PM

Intersection #23: Wolfe Road and Vallco Parkway (CUP)



Street Name:	Wolfe Road				Vallco Parkway										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	42	930	74	219	1183	52	55	15	15	209	13	454			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	42	930	74	219	1183	52	55	15	15	209	13	454			
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Hyatt:	0	9	0	0	8	0	0	0	0	0	0	0			
Initial Fut:	42	939	74	219	1191	52	55	15	15	209	13	454			
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Volume:	42	939	74	219	1191	0	55	15	15	209	13	454			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	42	939	74	219	1191	0	55	15	15	209	13	454			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
FinalVolume:	42	939	74	219	1191	0	55	15	15	209	13	454			

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92
Lanes:	1.00	2.76	0.24	2.00	3.00	1.00	1.00	1.00	1.00	2.00	0.05	1.95
Final Sat.:	1750	5251	414	3150	5700	1750	1750	1900	1750	3150	98	3410

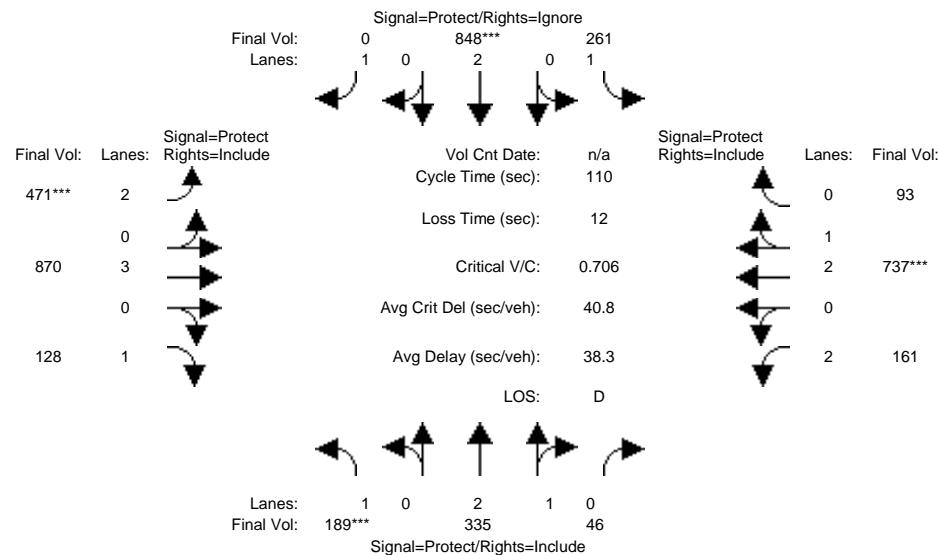
Capacity Analysis Module:

Vol/Sat:	0.02	0.18	0.18	0.07	0.21	0.00	0.03	0.01	0.01	0.07	0.13	0.13
Crit Moves:	****			****		****				****		
Green Time:	7.0	45.7	45.7	17.8	56.5	0.0	8.5	24.8	24.8	19.7	36.0	36.0
Volume/Cap:	0.41	0.47	0.47	0.47	0.44	0.00	0.44	0.04	0.04	0.40	0.44	0.44
Delay/Veh:	57.2	28.2	28.2	47.5	21.4	0.0	56.0	38.1	38.2	45.4	34.2	34.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	57.2	28.2	28.2	47.5	21.4	0.0	56.0	38.1	38.2	45.4	34.2	34.2
LOS by Move:	E	C	C	D	C	A	E	D	D	D	C	C
HCM2kAvgQ:	2	9	9	4	9	0	3	0	0	4	7	7

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Existing PM

Intersection #24: Stevens Creek Blvd and Wolfe Rd/Miller Ave (CUP/CMP)



Street Name:	Wolfe Rd/Miller Ave						Stevens Creek Blvd								
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10		
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		

Volume Module:

Base Vol:	189	335	46	261	848	444	471	870	128	161	737	93
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	189	335	46	261	848	444	471	870	128	161	737	93
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	189	335	46	261	848	444	471	870	128	161	737	93
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	189	335	46	261	848	0	471	870	128	161	737	93
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	189	335	46	261	848	0	471	870	128	161	737	93
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	189	335	46	261	848	0	471	870	128	161	737	93

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.99	0.95	0.92	1.00	0.92	0.83	1.00	0.92	0.83	0.99	0.95
Lanes:	1.00	2.62	0.38	1.00	2.00	1.00	2.00	3.00	1.00	2.00	2.65	0.35
Final Sat.:	1750	4923	676	1750	3800	1750	3150	5700	1750	3150	4972	627

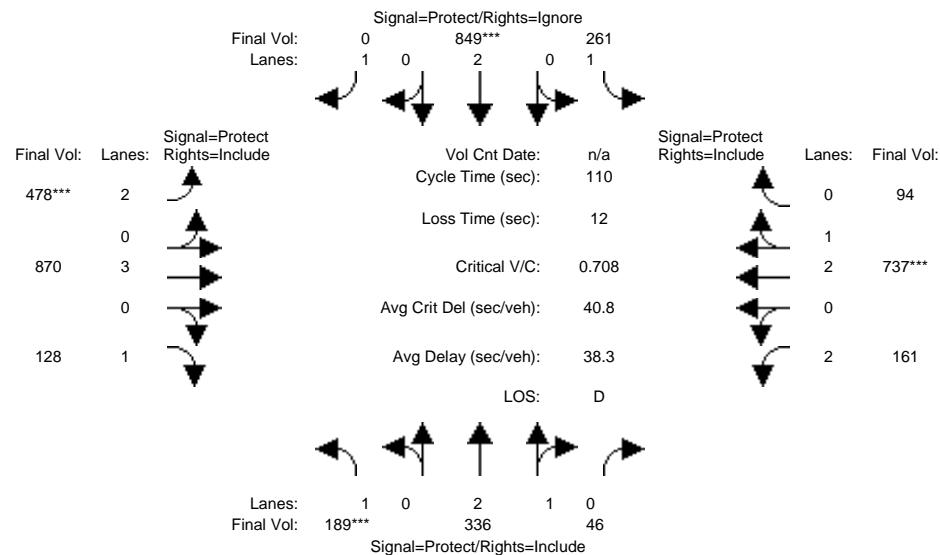
Capacity Analysis Module:

Vol/Sat:	0.11	0.07	0.07	0.15	0.22	0.00	0.15	0.15	0.07	0.05	0.15	0.15
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	16.8	19.5	19.5	32.1	34.8	0.0	23.3	32.7	32.7	13.7	23.1	23.1
Volume/Cap:	0.71	0.38	0.38	0.51	0.71	0.00	0.71	0.51	0.25	0.41	0.71	0.71
Uniform Del:	44.2	39.9	39.9	32.5	33.1	0.0	40.2	32.0	29.3	44.5	40.3	40.3
IncremntDel:	8.3	0.2	0.2	0.9	1.9	0.0	3.5	0.3	0.2	0.7	2.0	2.0
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	52.6	40.2	40.2	33.3	35.1	0.0	43.6	32.3	29.5	45.2	42.3	42.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	52.6	40.2	40.2	33.3	35.1	0.0	43.6	32.3	29.5	45.2	42.3	42.3
LOS by Move:	D	D	D	C	D	A	D	C	C	D	D	D
HCM2kAvgQ:	8	4	4	8	13	0	10	8	4	3	9	9

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Ex + P PM

Intersection #24: Stevens Creek Blvd and Wolfe Rd/Miller Ave (CUP/CMP)



Street Name:	Wolfe Rd/Miller Ave				Stevens Creek Blvd										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	189	335	46	261	848	444	471	870	128	161	737	93
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	189	335	46	261	848	444	471	870	128	161	737	93
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Hyatt:	0	1	0	0	1	6	7	0	0	0	0	1
Initial Fut:	189	336	46	261	849	450	478	870	128	161	737	94
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	189	336	46	261	849	0	478	870	128	161	737	94
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	189	336	46	261	849	0	478	870	128	161	737	94
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	189	336	46	261	849	0	478	870	128	161	737	94

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92	0.83	1.00	0.92
Lanes:	1.00	2.61	0.39	1.00	2.00	1.00	2.00	3.00	1.00	2.00	2.64	0.36
Final Sat.:	1750	4962	679	1750	3800	1750	3150	5700	1750	3150	5007	639

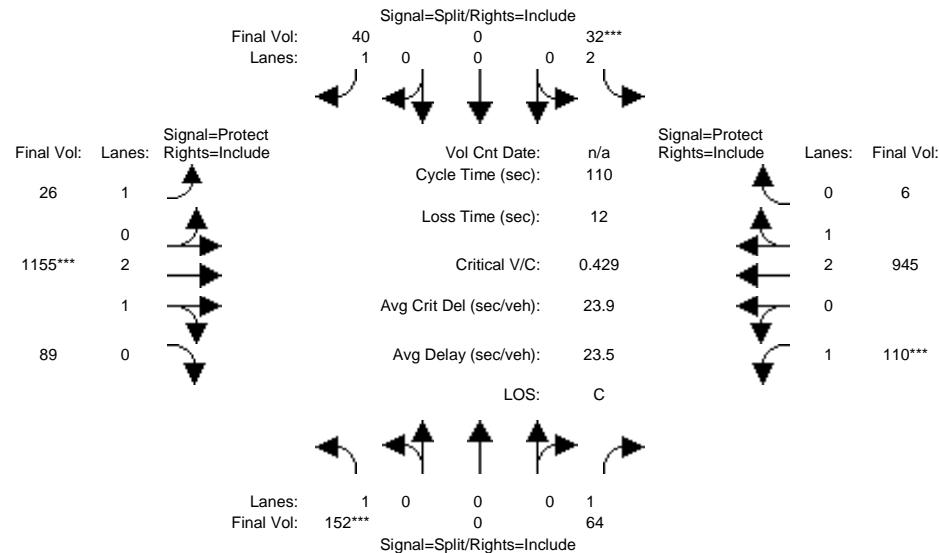
Capacity Analysis Module:

Vol/Sat:	0.11	0.07	0.07	0.15	0.22	0.00	0.15	0.15	0.07	0.05	0.15	0.15
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	16.8	19.5	19.5	32.0	34.7	0.0	23.6	32.8	32.8	13.7	22.9	22.9
Volume/Cap:	0.71	0.38	0.38	0.51	0.71	0.00	0.71	0.51	0.25	0.41	0.71	0.71
Delay/Veh:	52.7	40.2	40.2	33.4	35.1	0.0	43.5	32.2	29.5	45.2	42.5	42.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	52.7	40.2	40.2	33.4	35.1	0.0	43.5	32.2	29.5	45.2	42.5	42.5
LOS by Move:	D	D	D	C	D	A	D	C	C	D	D	D
HCM2kAvgQ:	8	4	4	8	13	0	10	8	4	3	9	9

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Existing PM

Intersection #26: Stevens Creek Blvd and Finch Avenue (CUP)



Street Name:	Finch Avenue				Stevens Creek Blvd										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	10	10	10	10	10	10	7	10	10	10	7	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	152	0	64	32	0	40	26	1155	89	110	945	6
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	152	0	64	32	0	40	26	1155	89	110	945	6
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	152	0	64	32	0	40	26	1155	89	110	945	6
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	152	0	64	32	0	40	26	1155	89	110	945	6
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	152	0	64	32	0	40	26	1155	89	110	945	6
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	152	0	64	32	0	40	26	1155	89	110	945	6

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	0.99	0.95	0.92	0.98	0.95
Lanes:	1.00	0.00	1.00	2.00	0.00	1.00	1.00	2.78	0.22	1.00	2.98	0.02
Final Sat.:	1750	0	1750	3150	0	1750	1750	5199	401	1750	5565	35

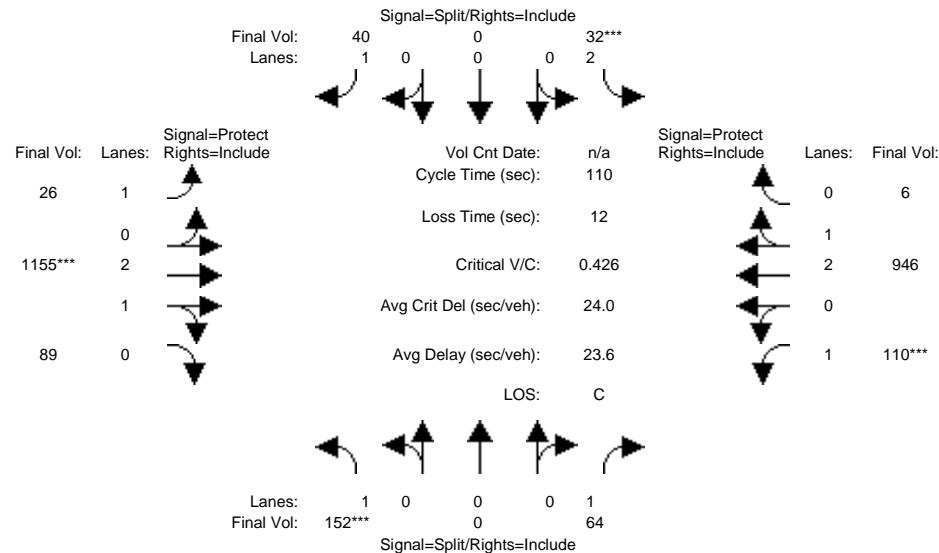
Capacity Analysis Module:

Vol/Sat:	0.09	0.00	0.04	0.01	0.00	0.02	0.01	0.22	0.22	0.06	0.17	0.17
Crit Moves:	****		****		****		****		****		****	
Green Time:	20.6	0.0	20.6	10.0	0.0	10.0	18.4	52.6	52.6	14.9	49.1	49.1
Volume/Cap:	0.46	0.00	0.20	0.11	0.00	0.25	0.09	0.46	0.46	0.46	0.38	0.38
Uniform Del:	39.8	0.0	37.7	45.9	0.0	46.5	38.7	19.3	19.3	43.9	20.3	20.3
IncremntDel:	1.0	0.0	0.3	0.2	0.0	0.8	0.1	0.1	0.1	1.4	0.1	0.1
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	40.9	0.0	38.0	46.1	0.0	47.4	38.9	19.4	19.4	45.3	20.4	20.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	40.9	0.0	38.0	46.1	0.0	47.4	38.9	19.4	19.4	45.3	20.4	20.4
LOS by Move:	D	A	D	D	A	D	D	B	B	D	C	C
HCM2kAvgQ:	5	0	2	1	0	2	1	9	9	4	7	7

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Ex + P PM

Intersection #26: Stevens Creek Blvd and Finch Avenue (CUP)



Street Name: Finch Avenue Stevens Creek Blvd																		
Approach:	North Bound			South Bound			East Bound			West Bound								
	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R			
Min. Green:	10		10		10		10		10		10		7		10		10	
Y+R:	4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0	

Volume Module:

Base Vol:	152	0	64	32	0	40	26	1155	89	110	945	6
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	152	0	64	32	0	40	26	1155	89	110	945	6
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Hyatt:	0	0	0	0	0	0	0	0	0	0	1	0
Initial Fut:	152	0	64	32	0	40	26	1155	89	110	946	6
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	152	0	64	32	0	40	26	1155	89	110	946	6
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	152	0	64	32	0	40	26	1155	89	110	946	6
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	152	0	64	32	0	40	26	1155	89	110	946	6

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00
Lanes:	1.00	0.00	1.00	2.00	0.00	1.00	1.00	2.77	0.23	1.00	2.98
Final Sat.:	1750	0	1750	3150	0	1750	1750	5260	405	1750	5661

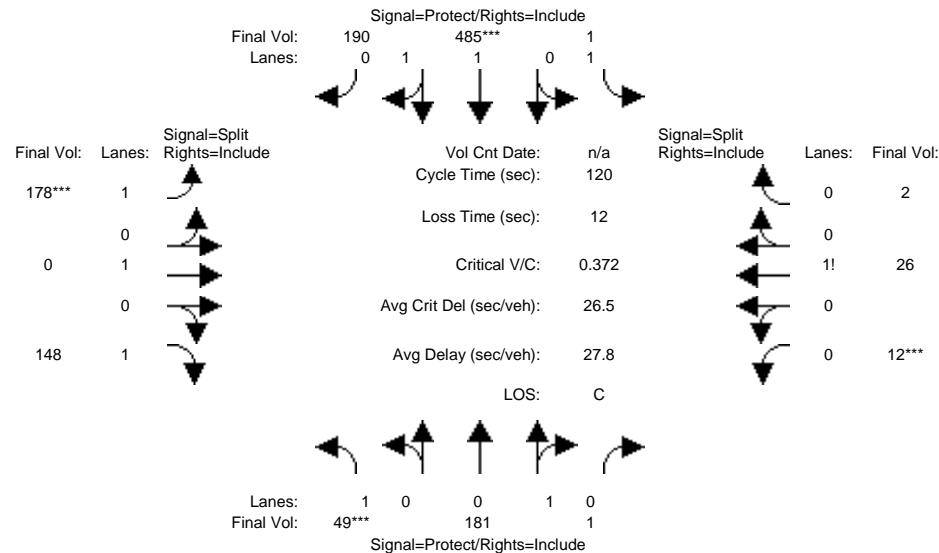
Capacity Analysis Module:

Vol/Sat:	0.09	0.00	0.04	0.01	0.00	0.02	0.01	0.22	0.22	0.06	0.17	0.17
Crit Moves:	****		****		****		****		****		****	
Green Time:	20.7	0.0	20.7	10.0	0.0	10.0	18.6	52.3	52.3	15.0	48.7	48.7
Volume/Cap:	0.46	0.00	0.19	0.11	0.00	0.25	0.09	0.46	0.46	0.46	0.38	0.38
Delay/Veh:	40.7	0.0	37.9	46.1	0.0	47.4	38.7	19.5	19.5	45.2	20.6	20.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	40.7	0.0	37.9	46.1	0.0	47.4	38.7	19.5	19.5	45.2	20.6	20.6
LOS by Move:	D	A	D	D	A	D	D	B	B	D	C	C
HCM2kAvgQ:	5	0	2	1	0	2	1	9	9	4	7	7

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Existing PM

Intersection #31: Tantau Avenue and Vallco Parkway (CUP)



Street Name: Tantau Avenue Vallco Parkway
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 Min. Green: 7 10 10 7 10 10 10 10 10 10 10 10 10
 Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0

Volume Module:

Base Vol:	49	181	1	1	485	190	178	0	148	12	26	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	49	181	1	1	485	190	178	0	148	12	26	2
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	49	181	1	1	485	190	178	0	148	12	26	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	49	181	1	1	485	190	178	0	148	12	26	2
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	49	181	1	1	485	190	178	0	148	12	26	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	49	181	1	1	485	190	178	0	148	12	26	2

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Adjustment:	0.92	0.95	0.95	0.92	0.98	0.95	0.92	1.00	0.92	0.92	0.92	
Lanes:	1.00	0.99	0.01	1.00	1.42	0.58	1.00	1.00	1.00	0.30	0.65	0.05
Final Sat.:	1750	1790	10	1750	2658	1041	1750	1900	1750	525	1138	88

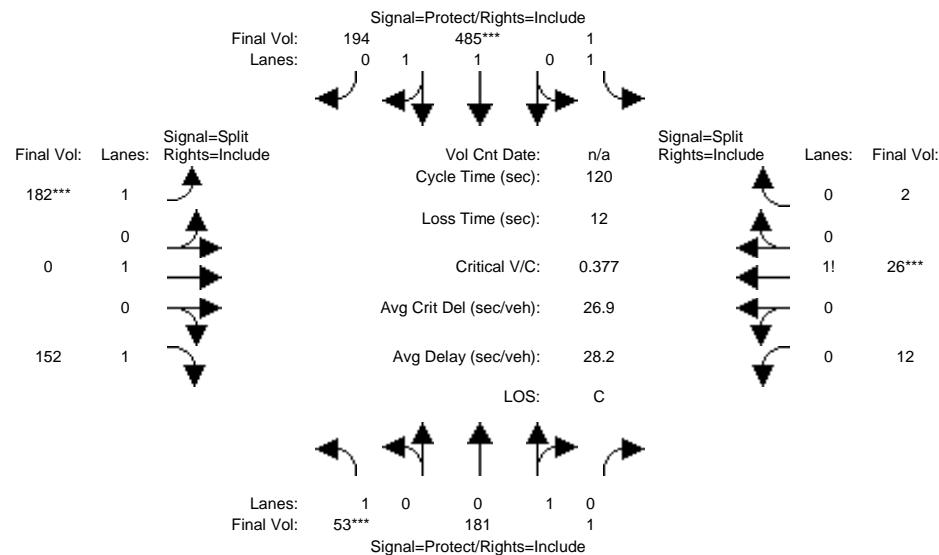
Capacity Analysis Module:

Vol/Sat:	0.03	0.10	0.10	0.00	0.18	0.18	0.10	0.00	0.08	0.02	0.02	0.02
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	8.8	41.9	41.9	24.2	57.3	57.3	31.9	0.0	31.9	10.0	10.0	10.0
Volume/Cap:	0.38	0.29	0.29	0.00	0.38	0.38	0.38	0.00	0.32	0.27	0.27	0.27
Uniform Del:	53.0	28.3	28.3	38.3	20.0	20.0	36.0	0.0	35.3	51.6	51.6	51.6
IncremntDel:	1.9	0.3	0.3	0.0	0.1	0.1	0.5	0.0	0.4	1.0	1.0	1.0
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00
Delay/Veh:	54.9	28.5	28.5	38.3	20.2	20.2	36.5	0.0	35.7	52.6	52.6	52.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	54.9	28.5	28.5	38.3	20.2	20.2	36.5	0.0	35.7	52.6	52.6	52.6
LOS by Move:	D	C	C	D	C	C	D	A	D	D	D	D
HCM2kAvgQ:	2	5	5	0	8	8	6	0	5	2	2	2

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Ex + P PM

Intersection #31: Tantau Avenue and Vallco Parkwy (CUP)



Street Name:	Tantau Avenue				Vallco Parkway										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	10	10	10	10	10	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	49	181	1	1	485	190	178	0	148	12	26	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	49	181	1	1	485	190	178	0	148	12	26	2
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Hyatt:	4	0	0	0	0	4	4	0	4	0	0	0
Initial Fut:	53	181	1	1	485	194	182	0	152	12	26	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	53	181	1	1	485	194	182	0	152	12	26	2
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	53	181	1	1	485	194	182	0	152	12	26	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	53	181	1	1	485	194	182	0	152	12	26	2

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	1.00	0.99	0.01	1.00	1.39	0.61	1.00	1.00	1.00	0.32	0.63	0.05
Final Sat.:	1750	1889	10	1750	2649	1060	1750	1900	1750	553	1199	92

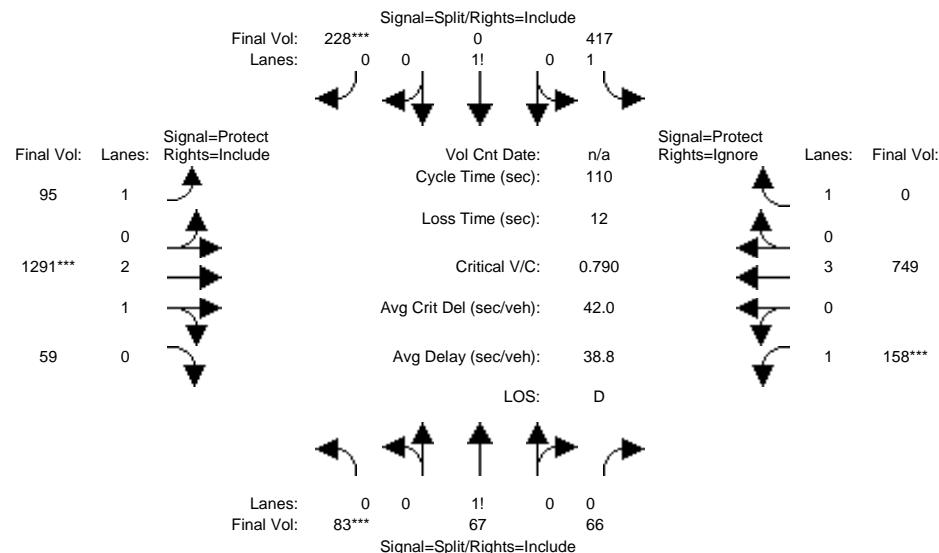
Capacity Analysis Module:

Vol/Sat:	0.03	0.10	0.10	0.00	0.18	0.18	0.10	0.00	0.09	0.02	0.02	0.02
Crit Moves:	****			****		****				****		
Green Time:	9.4	41.0	41.0	24.9	56.5	56.5	32.1	0.0	32.1	10.0	10.0	10.0
Volume/Cap:	0.39	0.28	0.28	0.00	0.39	0.39	0.39	0.00	0.32	0.26	0.26	0.26
Delay/Veh:	54.4	29.0	29.0	37.7	20.7	20.7	36.5	0.0	35.6	52.4	52.4	52.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	54.4	29.0	29.0	37.7	20.7	20.7	36.5	0.0	35.6	52.4	52.4	52.4
LOS by Move:	D	C	C	D	C	C	D	A	D	D	D	D
HCM2kAvgQ:	2	5	5	0	8	8	6	0	5	2	2	2

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Existing PM

Intersection #32: Stevens Creek Blvd and Tantau Avenue (CUP)



Street Name: Tantau Avenue Stevens Creek Blvd

Approach:	North Bound			South Bound			East Bound			West Bound					
	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	7	10	10	7	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	83	67	66	417	0	228	95	1291	59	158	749	69
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	83	67	66	417	0	228	95	1291	59	158	749	69
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	83	67	66	417	0	228	95	1291	59	158	749	69
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Volume:	83	67	66	417	0	228	95	1291	59	158	749	0
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	83	67	66	417	0	228	95	1291	59	158	749	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
FinalVolume:	83	67	66	417	0	228	95	1291	59	158	749	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.92	0.92	0.92	1.00	0.92	0.92	0.98	0.95	0.92	1.00	0.92
Lanes:	0.38	0.31	0.31	1.48	0.00	0.52	1.00	2.86	0.14	1.00	3.00	1.00
Final Sat.:	672	543	535	2586	0	914	1750	5355	245	1750	5700	1750

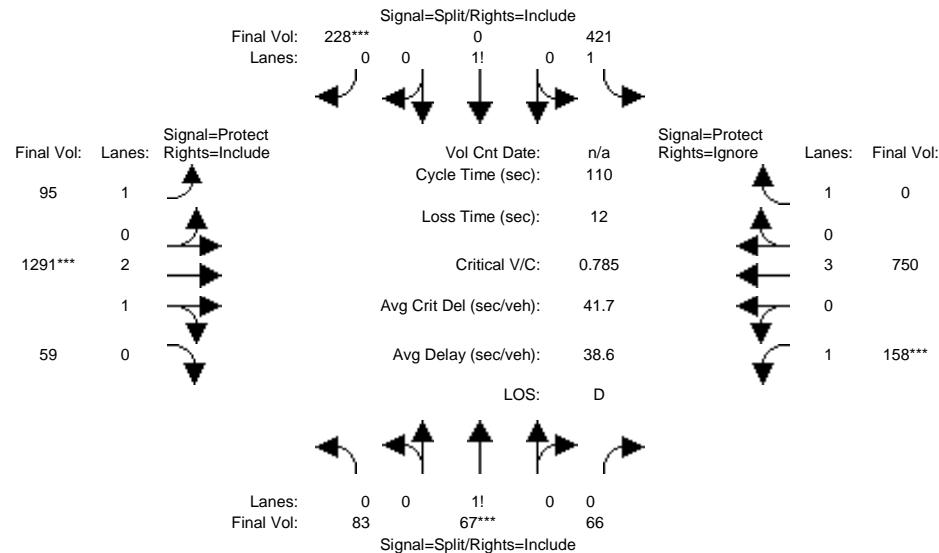
Capacity Analysis Module:

Vol/Sat:	0.12	0.12	0.12	0.16	0.00	0.25	0.05	0.24	0.24	0.09	0.13	0.00
Crit Moves:	****			****		****	****	****	****	****		
Green Time:	17.2	17.2	17.2	34.7	0.0	34.7	15.0	33.5	33.5	12.6	31.1	0.0
Volume/Cap:	0.79	0.79	0.79	0.51	0.00	0.79	0.40	0.79	0.79	0.79	0.47	0.00
Uniform Del:	44.7	44.7	44.7	30.7	0.0	34.3	43.3	35.0	35.0	47.4	32.6	0.0
IncremntDel:	14.4	14.4	14.4	0.4	0.0	5.3	1.1	2.6	2.6	18.9	0.2	0.0
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Delay/Veh:	59.1	59.1	59.1	31.1	0.0	39.6	44.4	37.6	37.6	66.3	32.8	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	59.1	59.1	59.1	31.1	0.0	39.6	44.4	37.6	37.6	66.3	32.8	0.0
LOS by Move:	E	E	E	C	A	D	D	D	D	E	C	A
HCM2kAvgQ:	10	10	10	8	0	15	3	15	15	6	7	0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Ex + P PM

Intersection #32: Stevens Creek Blvd and Tantau Avenue (CUP)

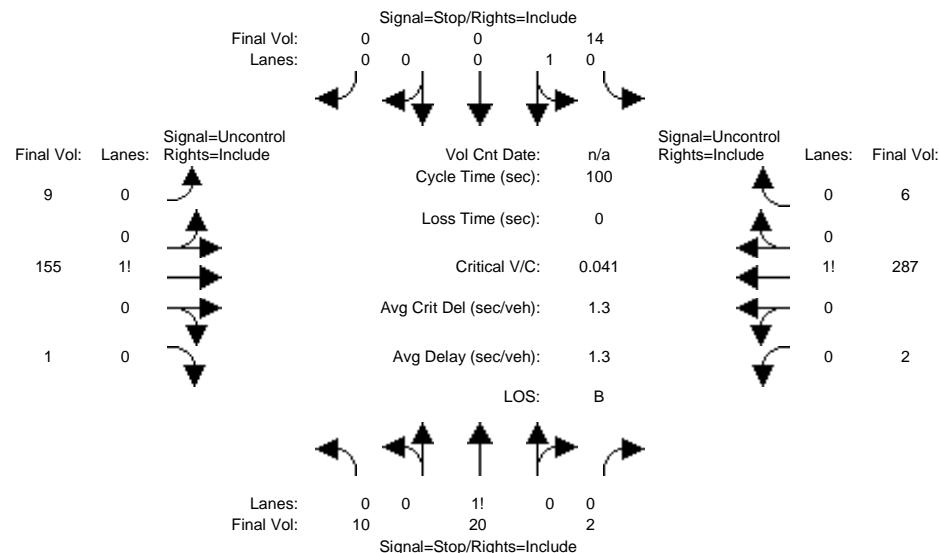


Street Name: Tantau Avenue Stevens Creek Blvd															
Approach:	North Bound			South Bound			East Bound			West Bound					
	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	7	10	10	7	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Volume Module:	<hr/>														
Base Vol:	83	67	66	417	0	228	95	1291	59	158	749	69			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	83	67	66	417	0	228	95	1291	59	158	749	69			
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Hyatt:	0	0	0	4	0	0	0	0	0	0	1	4			
Initial Fut:	83	67	66	421	0	228	95	1291	59	158	750	73			
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00			
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00			
PHF Volume:	83	67	66	421	0	228	95	1291	59	158	750	0			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	83	67	66	421	0	228	95	1291	59	158	750	0			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00			
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00			
FinalVolume:	83	67	66	421	0	228	95	1291	59	158	750	0			
Saturation Flow Module:	<hr/>														
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900			
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92			
Lanes:	0.40	0.29	0.31	1.48	0.00	0.52	1.00	2.86	0.14	1.00	3.00	1.00			
Final Sat.:	689	556	548	2590	0	910	1750	5431	248	1750	5700	1750			
Capacity Analysis Module:	<hr/>														
Vol/Sat:	0.12	0.12	0.12	0.16	0.00	0.25	0.05	0.24	0.24	0.09	0.13	0.00			
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****			
Green Time:	16.9	16.9	16.9	35.1	0.0	35.1	15.0	33.3	33.3	12.7	31.0	0.0			
Volume/Cap:	0.78	0.78	0.78	0.51	0.00	0.78	0.40	0.78	0.78	0.78	0.47	0.00			
Delay/Veh:	58.5	58.5	58.5	30.8	0.0	39.0	44.5	37.5	37.5	65.4	32.9	0.0			
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
AdjDel/Veh:	58.5	58.5	58.5	30.8	0.0	39.0	44.5	37.5	37.5	65.4	32.9	0.0			
LOS by Move:	E	E	E	C	A	D	D	D	D	E	C	A			
HCM2kAvgQ:	9	9	9	8	0	15	3	14	14	6	7	0			

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Existing PM

Intersection #501: Finch Road and Vallco Pkwy



Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R

Volume Module:

Base Vol:	10	20	2	14	0	0	9	155	1	2	287	6
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	10	20	2	14	0	0	9	155	1	2	287	6
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	10	20	2	14	0	0	9	155	1	2	287	6
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	10	20	2	14	0	0	9	155	1	2	287	6
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	10	20	2	14	0	0	9	155	1	2	287	6

Critical Gap Module:

Critical Gp:	7.1	6.5	6.2	7.1	xxxxx	xxxxxx	4.1	xxxx	xxxxxx	4.1	xxxx	xxxxxx
FollowUpTim:	3.5	4.0	3.3	3.5	xxxxx	xxxxxx	2.2	xxxx	xxxxxx	2.2	xxxx	xxxxxx

Capacity Module:

Cnflict Vol:	468	471	156	479	xxxxx	xxxxxx	293	xxxx	xxxxxx	156	xxxx	xxxxxx
Potent Cap.:	509	494	896	501	xxxxx	xxxxxx	1280	xxxx	xxxxxx	1436	xxxx	xxxxxx
Move Cap.:	506	490	896	481	xxxxx	xxxxxx	1280	xxxx	xxxxxx	1436	xxxx	xxxxxx
Volume/Cap:	0.02	0.04	0.00	0.03	xxxxx	xxxx	0.01	xxxx	xxxx	0.00	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	0.1	xxxx	xxxxxx	0.0	xxxx	xxxxxx	0.0	xxxx	xxxxxx			
Control Del:	xxxxx	xxxx	xxxxxx	12.7	xxxx	xxxxxx	7.8	xxxx	xxxxxx	7.5	xxxx	xxxxxx			
LOS by Move:	*	*	*	B	*	*	A	*	*	A	*	*			
Movement:	LT	-	LTR	-	RT	LT	-	LTR	-	RT	LT	-	LTR	-	RT
Shared Cap.:	xxxx	509	xxxxx	xxxx	xxxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx			
SharedQueue:	xxxxx	0.2	xxxxx	xxxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx			
Shrd ConDel:	xxxxx	12.5	xxxxx	xxxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx			
Shared LOS:	*	B	*	*	*	*	*	*	*	*	*	*			
ApproachDel:	12.5			12.7			xxxxxx			xxxxxx					
ApproachLOS:	B			B			*			*					

Note: Queue reported is the number of cars per lane.

Peak Hour Delay Signal Warrant Report

Intersection #501 Finch Road and Vallco Pkwy

Future Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
-----------	-------------	-------------	------------	------------

Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R	
Control:	Stop Sign				Stop Sign				Uncontrolled				Uncontrolled			
Lanes:	0	0	1!	0	0	1	0	0	0	0	0	1!	0	0	0	0
Initial Vol:	10	20	2	14	0	0	9	155	1	2	287	6				
ApproachDel:	12.5				12.7				xxxxxx				xxxxxx			

Approach[northbound] [lanes=1] [control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.1]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=32]
FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=4][total volume=506]
FAIL - Total volume less than 650 for intersection
with less than four approaches.

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #501 Finch Road and Vallco Pkwy

Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
-----|-----|-----|-----|-----|
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
Lanes: 0 0 1! 0 0 1 0 0 0 0 0 1! 0 0 0 0 1! 0 0
Initial Vol: 10 20 2 14 0 0 9 155 1 2 287 6
-----|-----|-----|-----|-----|
Major Street Volume: 460
Minor Approach Volume: 32
Minor Approach Volume Threshold: 427

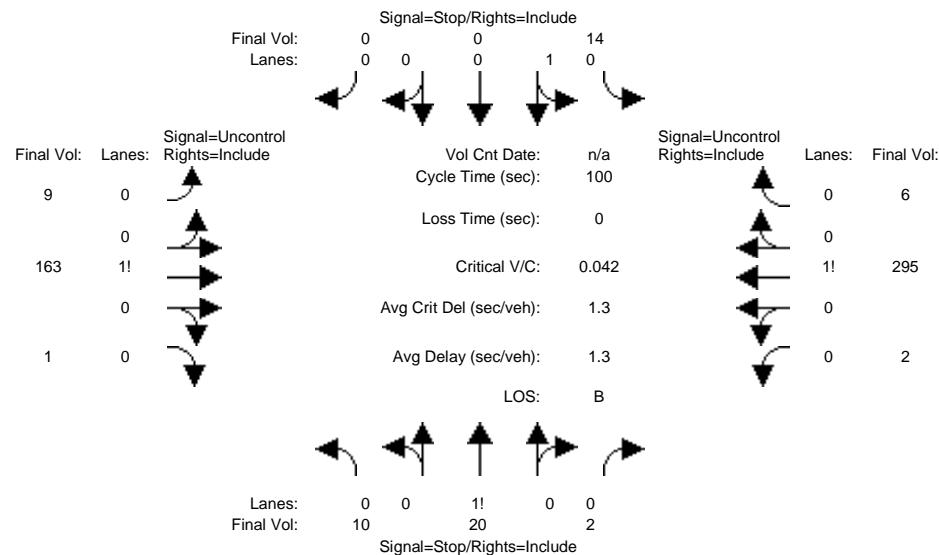
SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Level Of Service Computation Report
2000 HCM Unsignalized (Future Volume Alternative)
Ex + P PM

Intersection #501: Finch Road and Vallco Pkwy



Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R

Volume Module:

Base Vol:	10	20	2	14	0	0	9	155	1	2	287	6
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	10	20	2	14	0	0	9	155	1	2	287	6
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Hyatt:	0	0	0	0	0	0	0	8	0	0	8	0
Initial Fut:	10	20	2	14	0	0	9	163	1	2	295	6
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	10	20	2	14	0	0	9	163	1	2	295	6
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	10	20	2	14	0	0	9	163	1	2	295	6

Critical Gap Module:

Critical Gp:	7.1	6.5	6.2	7.1	xxxxx	xxxxxx	4.1	xxxx	xxxxxx	4.1	xxxx	xxxxxx
FollowUpTim:	3.5	4.0	3.3	3.5	xxxxx	xxxxxx	2.2	xxxx	xxxxxx	2.2	xxxx	xxxxxx

Capacity Module:

Cnflict Vol:	484	487	164	495	xxxxx	xxxxxx	301	xxxx	xxxxxx	164	xxxx	xxxxxx
Potent Cap.:	497	484	886	489	xxxxx	xxxxxx	1272	xxxx	xxxxxx	1427	xxxx	xxxxxx
Move Cap.:	494	480	886	469	xxxxx	xxxxxx	1272	xxxx	xxxxxx	1427	xxxx	xxxxxx
Volume/Cap:	0.02	0.04	0.00	0.03	xxxxx	xxxx	0.01	xxxx	xxxx	0.00	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	0.1	xxxx	xxxxxx	0.0	xxxx	xxxxxx	0.0	xxxx	xxxxxx
Control Del:	xxxxx	xxxx	xxxxxx	12.9	xxxx	xxxxxx	7.9	xxxx	xxxxxx	7.5	xxxx	xxxxxx

LOS by Move: * * * B * * * A * * * A * *

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxx 499 xxxx xxxx

SharedQueue:xxxxx 0.2 xxxx xxxx

Shrd ConDel:xxxxx 12.7 xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx

Shared LOS: * B * * * * * * * * * * * *

ApproachDel: 12.7 12.9 xxxxxxxx xxxxxxxx

ApproachLOS: B B * * *

Note: Queue reported is the number of cars per lane.

Peak Hour Delay Signal Warrant Report

Intersection #501 Finch Road and Vallco Pkwy

Future Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound

Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 1! 0 0	1 0 0 0 0	0 0 1! 0 0	0 0 1! 0 0
Initial Vol:	10 20 2 14 0 0	9 163 1	2 295 6	
ApproachDel:	12.7	12.9	xxxxxx	xxxxxx

Approach[northbound] [lanes=1] [control=Stop Sign]
Signal Warrant Rule #1: [vehicle-hours=0.1]
FAIL - Vehicle-hours less than 4 for one lane approach.
Signal Warrant Rule #2: [approach volume=32]
FAIL - Approach volume less than 100 for one lane approach.
Signal Warrant Rule #3: [approach count=4][total volume=522]
FAIL - Total volume less than 650 for intersection
with less than four approaches.

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #501 Finch Road and Vallco Pkwy

Future Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|-----|
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
-----|-----|-----|-----|-----|
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
Lanes: 0 0 1! 0 0 1 0 0 0 0 0 1! 0 0 0 0 1! 0 0
Initial Vol: 10 20 2 14 0 0 9 163 1 2 295 6
-----|-----|-----|-----|-----|
Major Street Volume: 476
Minor Approach Volume: 32
Minor Approach Volume Threshold: 417

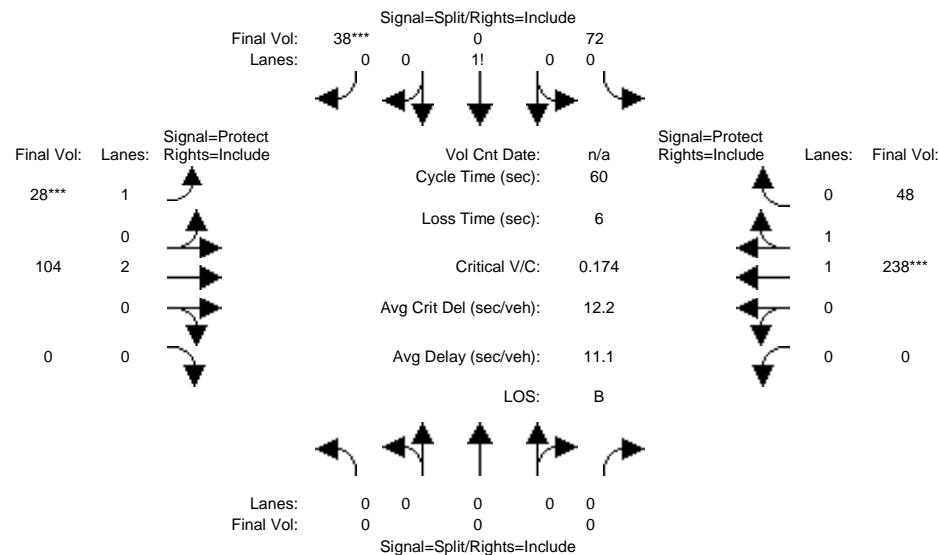
SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Existing PM

Intersection #504: Perimeter Road and Vallco Pkwy (CUP)



Street Name:		Perimeter Rd				Vallco Pkwy						
Approach:	North Bound	South Bound		East Bound		West Bound						
Movement:	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R						
Min. Green:	7	10	10	7	10	10	7	10	10	7	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	0	0	72	0	38	28	104	0	0	238	48
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	72	0	38	28	104	0	0	238	48
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	72	0	38	28	104	0	0	238	48
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	72	0	38	28	104	0	0	238	48
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	72	0	38	28	104	0	0	238	48
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	72	0	38	28	104	0	0	238	48

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	0.92	0.92	0.92	1.00	0.92	0.92	0.98	0.95
Lanes:	0.00	0.00	0.00	0.65	0.00	0.35	1.00	2.00	0.00	0.00	1.66	0.34
Final Sat.:	0	0	0	1145	0	605	1750	3800	0	0	3079	621

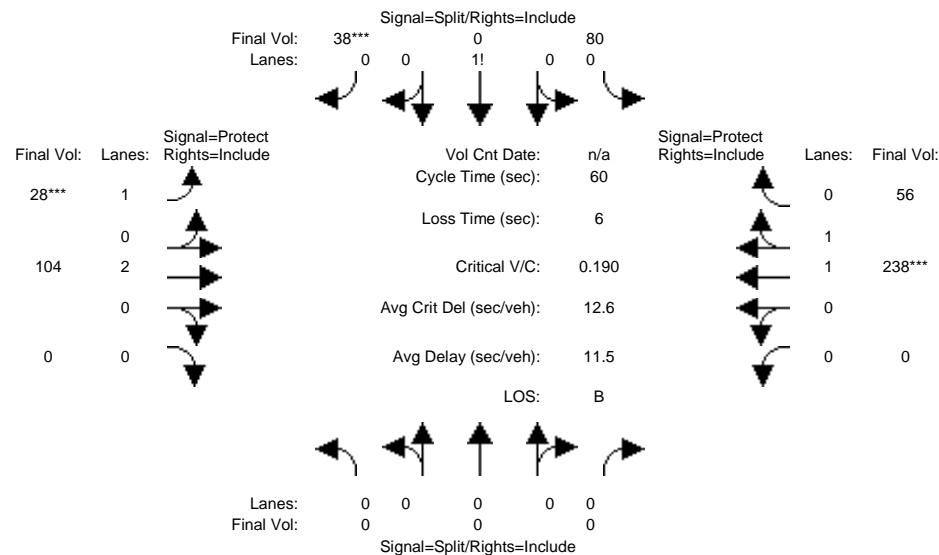
Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.06	0.00	0.06	0.02	0.03	0.00	0.00	0.08	0.08
Crit Moves:						*****	*****				*****	
Green Time:	0.0	0.0	0.0	21.1	0.0	21.1	7.0	32.9	0.0	0.0	25.9	25.9
Volume/Cap:	0.00	0.00	0.00	0.18	0.00	0.18	0.14	0.05	0.00	0.00	0.18	0.18
Uniform Del:	0.0	0.0	0.0	13.5	0.0	13.5	23.8	6.3	0.0	0.0	10.5	10.5
IncremntDel:	0.0	0.0	0.0	0.1	0.0	0.1	0.3	0.0	0.0	0.0	0.1	0.1
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00
Delay/Veh:	0.0	0.0	0.0	13.6	0.0	13.6	24.1	6.3	0.0	0.0	10.5	10.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	13.6	0.0	13.6	24.1	6.3	0.0	0.0	10.5	10.5
LOS by Move:	A	A	A	B	A	B	C	A	A	A	B	B
HCM2kAvgQ:	0	0	0	2	0	2	0	0	0	0	2	2

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Ex + P PM

Intersection #504: Perimeter Road and Vallco Pkwy (CUP)



Street Name:	Perimeter Rd				Vallco Pkwy										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	7	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	0	0	72	0	38	28	104	0	0	238	48
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	72	0	38	28	104	0	0	238	48
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Hyatt:	0	0	0	8	0	0	0	0	0	0	0	8
Initial Fut:	0	0	0	80	0	38	28	104	0	0	238	56
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	80	0	38	28	104	0	0	238	56
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	80	0	38	28	104	0	0	238	56
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	80	0	38	28	104	0	0	238	56

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.85	1.00	0.85	0.88	1.00	0.92	0.92	0.97	0.90
Lanes:	0.00	0.00	0.00	0.68	0.00	0.32	1.00	2.00	0.00	0.00	1.59	0.41
Final Sat.:	0	0	0	1098	0	522	1663	3800	0	0	2942	692

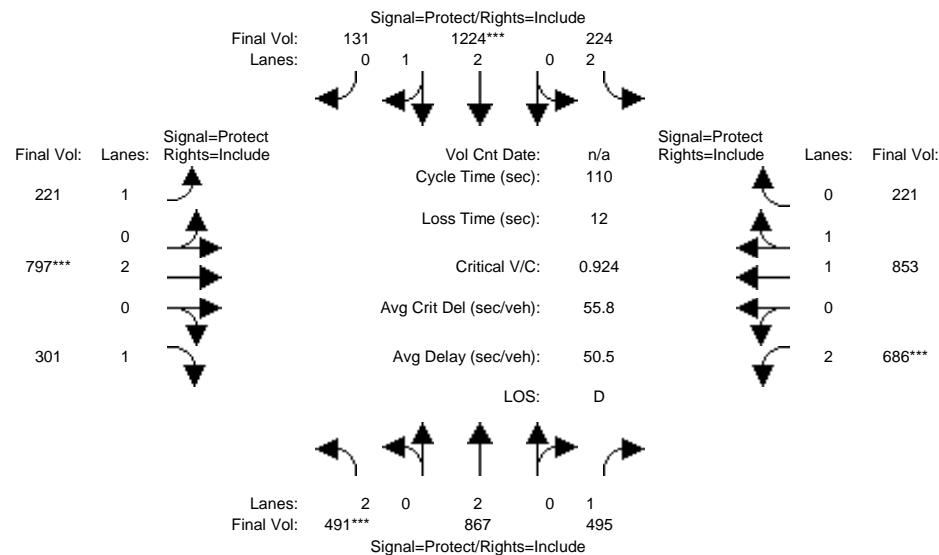
Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.07	0.00	0.07	0.02	0.03	0.00	0.00	0.08	0.08
Crit Moves:						****	****				****	
Green Time:	0.0	0.0	0.0	22.3	0.0	22.3	7.0	31.7	0.0	0.0	24.7	24.7
Volume/Cap:	0.00	0.00	0.00	0.20	0.00	0.20	0.14	0.05	0.00	0.00	0.20	0.20
Delay/Veh:	0.0	0.0	0.0	13.0	0.0	13.0	24.2	6.9	0.0	0.0	11.3	11.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	13.0	0.0	13.0	24.2	6.9	0.0	0.0	11.3	11.3
LOS by Move:	A	A	A	B	A	B	C	A	A	A	B	B
HCM2kAvgQ:	0	0	0	2	0	2	0	0	0	0	2	2

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd PM

Intersection #18: Wolfe Road and Homestead Road (CUP)



Street Name:														
Wolfe Road						Homestead Road								
Approach:	North Bound			South Bound			East Bound			West Bound				
	L	-	T	-	R	L	-	T	-	R	L	-	T	-
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	

Volume Module:

Base Vol:	322	758	189	159	1113	121	201	713	251	444	773	108
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	322	758	189	159	1113	121	201	713	251	444	773	108
Added Vol:	123	67	106	11	22	0	0	17	28	61	52	33
App+Div:	46	42	200	54	89	10	20	67	22	181	28	80
Initial Fut:	491	867	495	224	1224	131	221	797	301	686	853	221
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	491	867	495	224	1224	131	221	797	301	686	853	221
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	491	867	495	224	1224	131	221	797	301	686	853	221
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	491	867	495	224	1224	131	221	797	301	686	853	221

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Adjustment:	0.83	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92
Lanes:	2.00	2.00	1.00	2.00	2.69	0.31	1.00	2.00	1.00	2.00	1.56	0.44
Final Sat.:	3150	3800	1750	3150	5107	547	1750	3800	1750	3150	2966	768

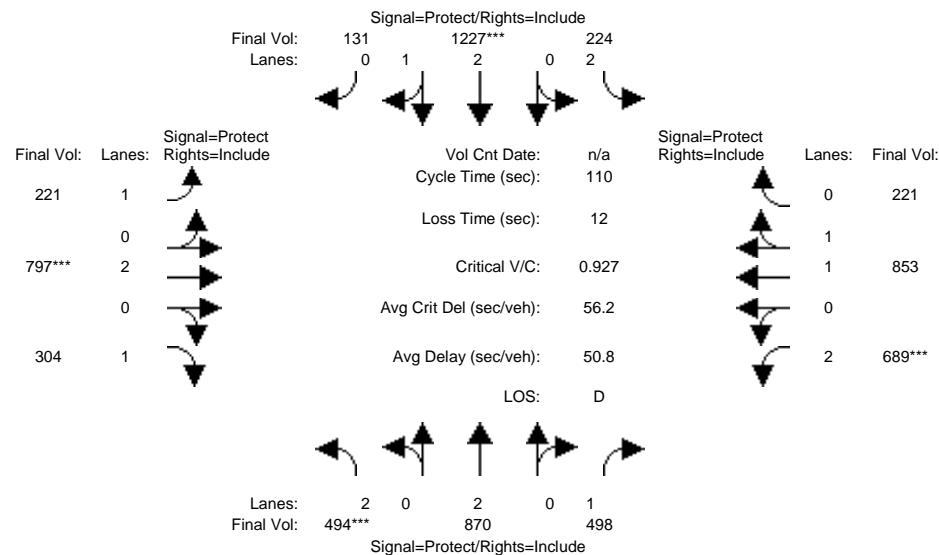
Capacity Analysis Module:

Vol/Sat:	0.16	0.23	0.28	0.07	0.24	0.24	0.13	0.21	0.17	0.22	0.29	0.29
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	18.6	37.6	37.6	9.5	28.5	28.5	15.5	25.0	25.0	25.9	35.4	35.4
Volume/Cap:	0.92	0.67	0.83	0.83	0.92	0.92	0.89	0.92	0.76	0.92	0.89	0.89
Uniform Del:	45.0	30.8	33.2	49.5	39.7	39.7	46.4	41.6	39.7	41.1	35.5	35.5
IncremntDel:	22.0	1.3	9.3	18.5	10.1	10.1	31.0	15.4	8.2	17.2	8.9	8.9
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	67.0	32.2	42.5	68.0	49.8	49.8	77.5	56.9	47.9	58.3	44.4	44.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	67.0	32.2	42.5	68.0	49.8	49.8	77.5	56.9	47.9	58.3	44.4	44.4
LOS by Move:	E	C	D	E	D	D	E	E	D	E	D	D
HCM2kAvgQ:	9	11	15	5	16	16	9	14	10	14	18	18

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + P PM

Intersection #18: Wolfe Road and Homestead Road (CUP)



Street Name:															
Wolfe Road						Homestead Road									
Approach:		North Bound			South Bound			East Bound			West Bound				
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	10	
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	

Volume Module:

Base Vol:	368	800	389	213	1202	131	221	780	273	625	801	188
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	368	800	389	213	1202	131	221	780	273	625	801	188
Added Vol:	123	67	106	11	22	0	0	17	28	61	52	33
Hyatt:	3	3	3	0	3	0	0	0	3	3	0	0
Initial Fut:	494	870	498	224	1227	131	221	797	304	689	853	221
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	494	870	498	224	1227	131	221	797	304	689	853	221
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	494	870	498	224	1227	131	221	797	304	689	853	221
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	494	870	498	224	1227	131	221	797	304	689	853	221

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.83	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92
Lanes:	2.00	2.00	1.00	2.00	2.69	0.31	1.00	2.00	1.00	2.00	1.56	0.44
Final Sat.:	3150	3800	1750	3150	5108	545	1750	3800	1750	3150	2966	768

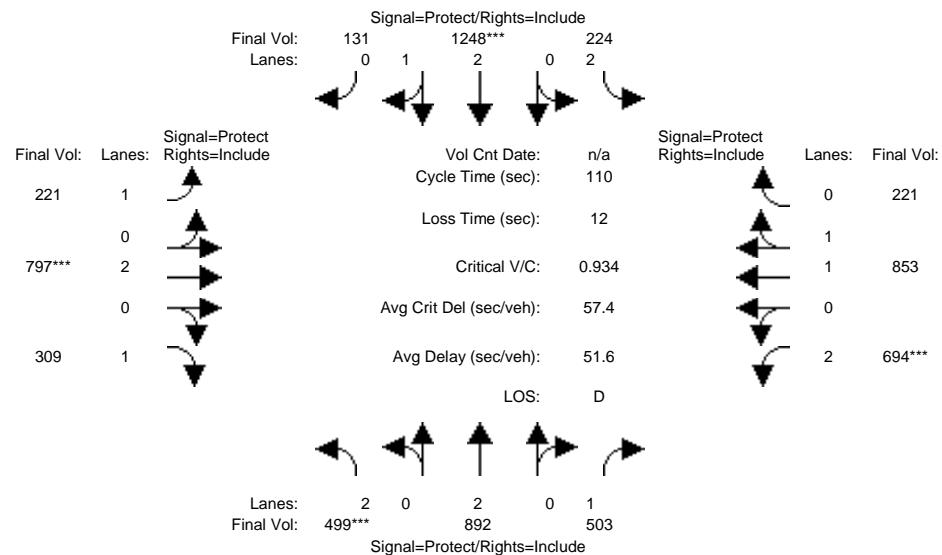
Capacity Analysis Module:

Vol/Sat:	0.16	0.23	0.28	0.07	0.24	0.24	0.13	0.21	0.17	0.22	0.29	0.29
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	18.6	37.7	37.7	9.4	28.5	28.5	15.5	24.9	24.9	26.0	35.3	35.3
Volume/Cap:	0.93	0.67	0.83	0.83	0.93	0.93	0.90	0.93	0.77	0.93	0.90	0.90
Uniform Del:	45.0	30.8	33.2	49.5	39.7	39.7	46.4	41.7	39.8	41.1	35.6	35.6
IncremntDel:	22.4	1.3	9.5	19.0	10.4	10.4	31.2	15.8	8.8	17.6	9.0	9.0
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	67.4	32.2	42.7	68.5	50.1	50.1	77.6	57.4	48.6	58.7	44.5	44.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	67.4	32.2	42.7	68.5	50.1	50.1	77.6	57.4	48.6	58.7	44.5	44.5
LOS by Move:	E	C	D	E	D	D	E	E	D	E	D	D
HCM2kAvgQ:	10	11	16	5	16	16	9	14	10	14	18	18

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative PM

Intersection #18: Wolfe Road and Homestead Road (CUP)

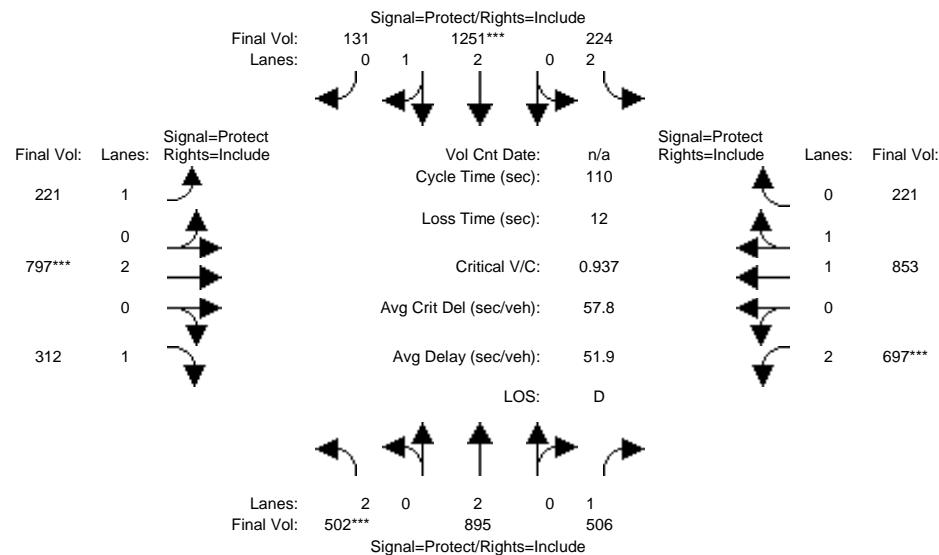


Street Name: Wolfe Road Homestead Road														
Approach:	North Bound			South Bound			East Bound			West Bound				
	L	-	T	-	R	L	-	T	-	R	L	-	T	-
Min. Green:	7 10		10 7		10 10		10 7		10 10		7 10		10 10	
Y+R:	4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0	
Volume Module:														
Base Vol:	322	758	189	159	1113	121	201	713	251	444	773	108		
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Initial Bse:	322	758	189	159	1113	121	201	713	251	444	773	108		
Added Vol:	123	67	106	11	22	0	0	17	28	61	52	33		
A+P+D:	54	67	208	54	113	10	20	67	30	189	28	80		
Initial Fut:	499	892	503	224	1248	131	221	797	309	694	853	221		
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
PHF Volume:	499	892	503	224	1248	131	221	797	309	694	853	221		
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0		
Reduced Vol:	499	892	503	224	1248	131	221	797	309	694	853	221		
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Final Volume:	499	892	503	224	1248	131	221	797	309	694	853	221		
Saturation Flow Module:														
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Adjustment:	0.83	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92		
Lanes:	2.00	2.00	1.00	2.00	2.69	0.31	1.00	2.00	1.00	2.00	1.56	0.44		
Final Sat.:	3150	3800	1750	3150	5117	537	1750	3800	1750	3150	2966	768		
Capacity Analysis Module:														
Vol/Sat:	0.16	0.23	0.29	0.07	0.24	0.24	0.13	0.21	0.18	0.22	0.29	0.29		
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****		
Green Time:	18.7	38.0	38.0	9.4	28.7	28.7	15.4	24.7	24.7	25.9	35.2	35.2		
Volume/Cap:	0.93	0.68	0.83	0.83	0.93	0.93	0.90	0.93	0.79	0.93	0.90	0.90		
Delay/Veh:	68.8	32.3	42.7	68.9	51.0	51.0	78.6	58.9	50.3	60.0	45.1	45.1		
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
AdjDel/Veh:	68.8	32.3	42.7	68.9	51.0	51.0	78.6	58.9	50.3	60.0	45.1	45.1		
LOS by Move:	E	C	D	E	D	D	E	E	D	E	D	D		
HCM2kAvgQ:	10	12	16	5	16	16	8	14	10	15	18	18		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + P PM

Intersection #18: Wolfe Road and Homestead Road (CUP)

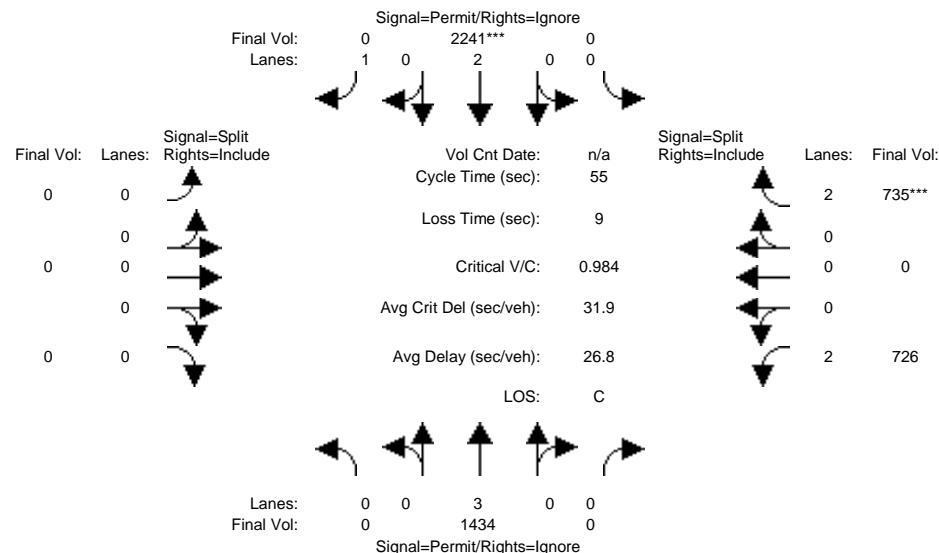


Street Name: Wolfe Road Homestead Road														
Approach:	North Bound			South Bound			East Bound			West Bound				
	L	-	T	-	R	L	-	T	-	R	L	-	T	-
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Volume Module:														
Base Vol:	376	825	397	213	1226	131	221	780	281	633	801	188		
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Initial Bse:	376	825	397	213	1226	131	221	780	281	633	801	188		
Added Vol:	123	67	106	11	22	0	0	17	28	61	52	33		
Hyatt:	3	3	3	0	3	0	0	0	3	3	0	0		
Initial Fut:	502	895	506	224	1251	131	221	797	312	697	853	221		
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
PHF Volume:	502	895	506	224	1251	131	221	797	312	697	853	221		
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0		
Reduced Vol:	502	895	506	224	1251	131	221	797	312	697	853	221		
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
FinalVolume:	502	895	506	224	1251	131	221	797	312	697	853	221		
Saturation Flow Module:														
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Adjustment:	0.83	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92		
Lanes:	2.00	2.00	1.00	2.00	2.69	0.31	1.00	2.00	1.00	2.00	1.56	0.44		
Final Sat.:	3150	3800	1750	3150	5118	536	1750	3800	1750	3150	2966	768		
Capacity Analysis Module:														
Vol/Sat:	0.16	0.24	0.29	0.07	0.24	0.24	0.13	0.21	0.18	0.22	0.29	0.29		
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****		
Green Time:	18.7	38.0	38.0	9.4	28.7	28.7	15.4	24.6	24.6	26.0	35.2	35.2		
Volume/Cap:	0.94	0.68	0.84	0.84	0.94	0.94	0.90	0.94	0.80	0.94	0.90	0.90		
Delay/Veh:	69.3	32.3	43.0	69.5	51.4	51.4	78.8	59.4	51.2	60.5	45.1	45.1		
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
AdjDel/Veh:	69.3	32.3	43.0	69.5	51.4	51.4	78.8	59.4	51.2	60.5	45.1	45.1		
LOS by Move:	E	C	D	E	D	D	E	E	D	E	D	D		
HCM2kAvgQ:	10	12	16	5	16	16	8	14	11	15	18	18		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd PM

Intersection #21: Wolfe Road and I-280 NB Ramps (CUP/CMP)



		Wolfe Road				I-280 NB Ramps			
Approach:	North Bound	South Bound		East Bound		West Bound			
Movement:	L - T - R	L - T - R		L - T - R		L - T - R		L - T - R	
Min. Green:	7 10	10	7 10	10	0 0	0 0	0 0	10 10	10
Y+R:	4.0 4.0	4.0	4.0 4.0	4.0	4.0 4.0	4.0 4.0	4.0 4.0	4.0 4.0	4.0

Volume Module:

Base Vol:	0	1171	663	0	1670	719	0	0	0	720	0	634
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1171	663	0	1670	719	0	0	0	720	0	634
Added Vol:	0	263	215	0	571	490	0	0	0	6	0	101
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1434	878	0	2241	1209	0	0	0	726	0	735
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1434	0	0	2241	0	0	0	0	726	0	735
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1434	0	0	2241	0	0	0	0	726	0	735
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1434	0	0	2241	0	0	0	0	726	0	735

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00
Lanes:	0.00	3.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	2.00	0.00
Final Sat.:	0	5700	0	0	3800	1750	0	0	0	3150	0

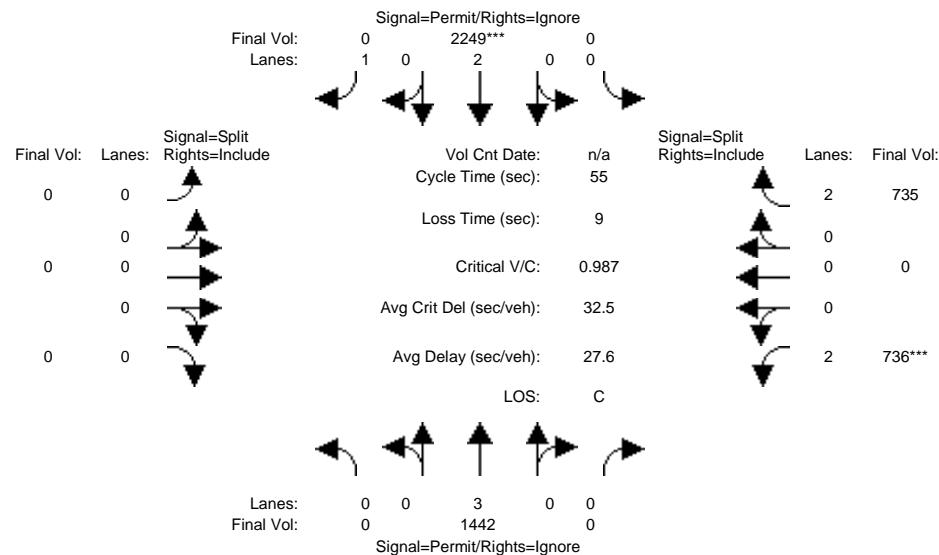
Capacity Analysis Module:

Vol/Sat:	0.00	0.25	0.00	0.00	0.59	0.00	0.00	0.00	0.00	0.23	0.00	0.23
Crit Moves:					****							****
Green Time:	0.0	33.0	0.0	0.0	33.0	0.0	0.0	0.0	0.0	13.0	0.0	13.0
Volume/Cap:	0.00	0.42	0.00	0.00	0.98	0.00	0.00	0.00	0.00	0.97	0.00	0.98
Uniform Del:	0.0	5.9	0.0	0.0	10.8	0.0	0.0	0.0	0.0	20.8	0.0	20.9
IncremntDel:	0.0	0.1	0.0	0.0	15.2	0.0	0.0	0.0	0.0	26.1	0.0	28.9
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	0.00	1.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	0.00	1.00
Delay/Veh:	0.0	6.0	0.0	0.0	26.0	0.0	0.0	0.0	0.0	46.9	0.0	49.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	6.0	0.0	0.0	26.0	0.0	0.0	0.0	0.0	46.9	0.0	49.7
LOS by Move:	A	A	A	A	C	A	A	A	A	D	A	D
HCM2kAvgQ:	0	4	0	0	20	0	0	0	0	13	0	14

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + P PM

Intersection #21: Wolfe Road and I-280 NB Ramps (CUP/CMP)



		Wolfe Road				I-280 NB Ramps			
Approach:	North Bound	South Bound		East Bound		West Bound			
Movement:	L - T - R	L - T - R		L - T - R		L - T - R		L - T - R	
Min. Green:	7 10	10	7 10	10	0 0	0 0	0 0	10 10	10
Y+R:	4.0 4.0	4.0	4.0 4.0	4.0	4.0 4.0	4.0 4.0	4.0 4.0	4.0 4.0	4.0

Volume Module:

Base Vol:	0 1171	663	0 1670	719	0 0	0 0	720	0	634
Growth Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00
Initial Bse:	0 1171	663	0 1670	719	0 0	0 0	720	0	634
Added Vol:	0 263	215	0 571	490	0 0	0 0	0 6	0	101
Hyatt:	0 8	0	0 8	0	0 0	0 0	10	0	0
Initial Fut:	0 1442	878	0 2249	1209	0 0	0 0	736	0	735
User Adj:	1.00 1.00	0.00	1.00 1.00	0.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00
PHF Adj:	1.00 1.00	0.00	1.00 1.00	0.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00
PHF Volume:	0 1442	0	0 2249	0	0 0	0 0	736	0	735
Reduc Vol:	0 0	0	0 0	0	0 0	0 0	0 0	0	0
Reduced Vol:	0 1442	0	0 2249	0	0 0	0 0	736	0	735
PCE Adj:	1.00 1.00	0.00	1.00 1.00	0.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00
MLF Adj:	1.00 1.00	0.00	1.00 1.00	0.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00
FinalVolume:	0 1442	0	0 2249	0	0 0	0 0	736	0	735

Saturation Flow Module:

Sat/Lane:	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900
Adjustment:	0.92 1.00	0.92	0.92 1.00	0.92	0.92 1.00	0.92	0.83 1.00	0.83
Lanes:	0.00 3.00	0.00	0.00 2.00	1.00	0.00 0.00	0.00	2.00 0.00	2.00
Final Sat.:	0 5700	0	0 3800	1750	0 0	0	3150 0	3150

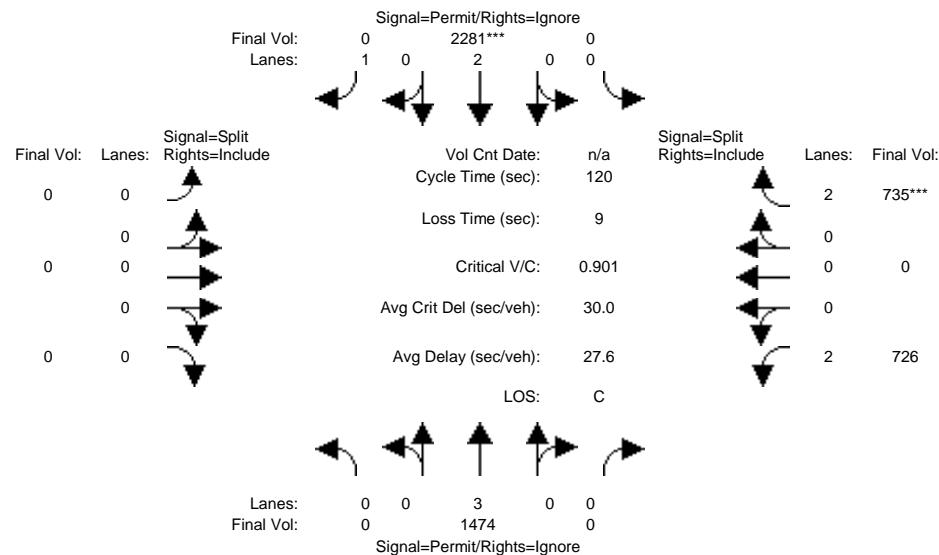
Capacity Analysis Module:

Vol/Sat:	0.00 0.25	0.00	0.00 0.59	0.00	0.00 0.00	0.00	0.23 0.00	0.23
Crit Moves:	*****							
Green Time:	0.0 33.0	0.0	0.0 33.0	0.0	0.0 0.0	0.0	13.0 0.0	13.0
Volume/Cap:	0.00 0.42	0.00	0.00 0.99	0.00	0.00 0.00	0.00	0.99 0.00	0.99
Uniform Del:	0.0 5.9	0.0	0.0 10.8	0.0	0.0 0.0	0.0	20.9 0.0	20.9
IncremntDel:	0.0 0.1	0.0	0.0 15.8	0.0	0.0 0.0	0.0	29.6 0.0	29.3
InitQueueDel:	0.0 0.0	0.0	0.0 0.0	0.0	0.0 0.0	0.0	0.0 0.0	0.0
Delay Adj:	0.00 1.00	0.00	0.00 1.00	0.00	0.00 0.00	0.00	1.00 0.00	1.00
Delay/Veh:	0.0 6.0	0.0	0.0 26.6	0.0	0.0 0.0	0.0	50.5 0.0	50.2
User DelAdj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00
AdjDel/Veh:	0.0 6.0	0.0	0.0 26.6	0.0	0.0 0.0	0.0	50.5 0.0	50.2
LOS by Move:	A A	A A	C A	A A	A A	A D	A A	D
HCM2kAvgQ:	0 4	0 0	20 0	0 0	0 0	0 14	0 0	14

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative PM

Intersection #21: Wolfe Road and I-280 NB Ramps (CUP/CMP)



Street Name:		Wolfe Road				I-280 NB Ramps										
Approach:		North Bound		South Bound		East Bound		West Bound								
Movement:		L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:		7	10	10	7	10	10	0	0	0	0	10	10	10	10	
Y+R:		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	1211	985	0	1710	719	0	0	0	720	0	634
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1211	985	0	1710	719	0	0	0	720	0	634
Added Vol:	0	263	215	0	571	490	0	0	0	6	0	101
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1474	1200	0	2281	1209	0	0	0	726	0	735
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1474	0	0	2281	0	0	0	0	726	0	735
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1474	0	0	2281	0	0	0	0	726	0	735
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1474	0	0	2281	0	0	0	0	726	0	735

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.83
Lanes:	0.00	3.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	2.00	0.00	2.00
Final Sat.:	0	5700	0	0	3800	1750	0	0	0	3150	0	3150

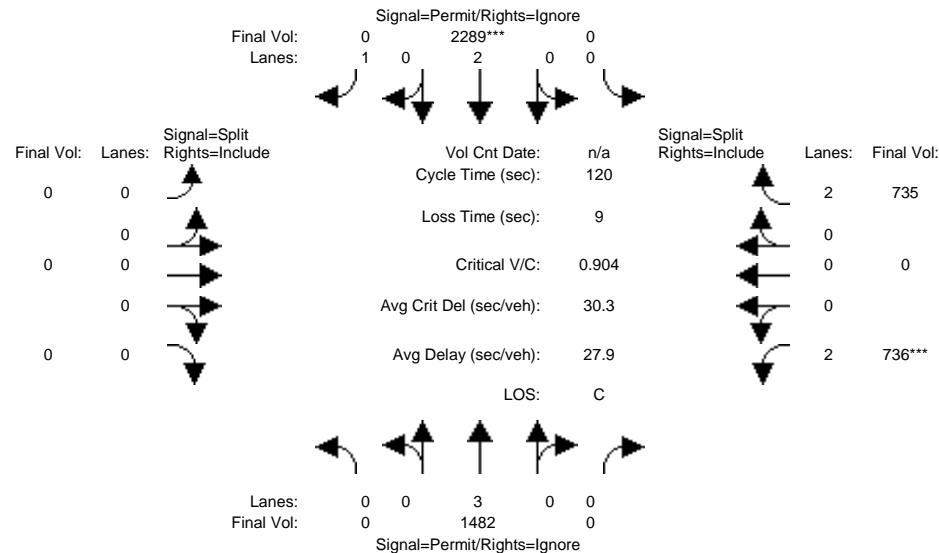
Capacity Analysis Module:

Vol/Sat:	0.00	0.26	0.00	0.00	0.60	0.00	0.00	0.00	0.00	0.23	0.00	0.23
Crit Moves:	*****											
Green Time:	0.0	79.9	0.0	0.0	79.9	0.0	0.0	0.0	0.0	31.1	0.0	31.1
Volume/Cap:	0.00	0.39	0.00	0.00	0.90	0.00	0.00	0.00	0.00	0.89	0.00	0.90
Delay/Veh:	0.0	9.1	0.0	0.0	21.7	0.0	0.0	0.0	0.0	54.7	0.0	56.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	9.1	0.0	0.0	21.7	0.0	0.0	0.0	0.0	54.7	0.0	56.1
LOS by Move:	A	A	A	A	C	A	A	A	A	D	A	E
HCM2kAvgQ:	0	7	0	0	34	0	0	0	0	19	0	19

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + P PM

Intersection #21: Wolfe Road and I-280 NB Ramps (CUP/CMP)



		Wolfe Road				I-280 NB Ramps			
Approach:	North Bound	South Bound		East Bound		West Bound			
Movement:	L - T - R	L - T - R		L - T - R		L - T - R		L - T - R	
Min. Green:	7 10	10	7 10	10	0 0	0 0	0 0	10 10	10
Y+R:	4.0 4.0	4.0	4.0 4.0	4.0	4.0 4.0	4.0 4.0	4.0 4.0	4.0 4.0	4.0

Volume Module:

Base Vol:	0	1211	985	0	1710	719	0	0	0	720	0	634
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1211	985	0	1710	719	0	0	0	720	0	634
Added Vol:	0	263	215	0	571	490	0	0	0	6	0	101
Hyatt:	0	8	0	0	8	0	0	0	0	10	0	0
Initial Fut:	0	1482	1200	0	2289	1209	0	0	0	736	0	735
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1482	0	0	2289	0	0	0	0	736	0	735
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1482	0	0	2289	0	0	0	0	736	0	735
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1482	0	0	2289	0	0	0	0	736	0	735

Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.83
Lanes:	0.00	3.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	2.00	0.00	2.00

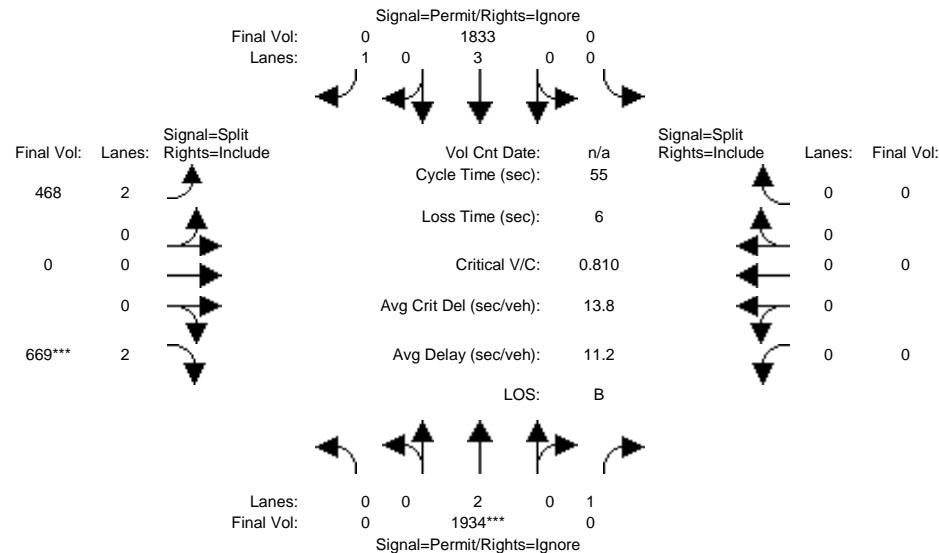
Final Sat.:	0	5700	0	0	3800	1750	0	0	0	3150	0	3150
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Capacity Analysis Module:												
Vol/Sat:	0.00	0.26	0.00	0.00	0.60	0.00	0.00	0.00	0.00	0.23	0.00	0.23
Crit Moves:					****					****		
Green Time:	0.0	80.0	0.0	0.0	80.0	0.0	0.0	0.0	0.0	31.0	0.0	31.0
Volume/Cap:	0.00	0.39	0.00	0.00	0.90	0.00	0.00	0.00	0.00	0.90	0.00	0.90
Delay/Veh:	0.0	9.1	0.0	0.0	21.8	0.0	0.0	0.0	0.0	56.5	0.0	56.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	9.1	0.0	0.0	21.8	0.0	0.0	0.0	0.0	56.5	0.0	56.3
LOS by Move:	A	A	A	A	C	A	A	A	A	E	A	E
HCM2kAvgQ:	0	7	0	0	34	0	0	0	0	19	0	19

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd PM

Intersection #22: Wolfe Road and I-280 SB Ramps (CUP/CMP)



Street Name:		Wolfe Road				I-280 SB Ramps			
Approach:	North Bound	South Bound		East Bound		West Bound			
Movement:	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R				
Min. Green:	7 10	10	7 10	10	10 10	10	0 0 0		
Y+R:	4.0 4.0	4.0	4.0 4.0	4.0	4.0 4.0	4.0	4.0 4.0 4.0		

Volume Module:

Base Vol:	0 1641	642	0 1662	730	283	0	585	0 0 0
Growth Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00
Initial Bse:	0 1641	642	0 1662	730	283	0	585	0 0 0
Added Vol:	0 293	6	0 171	406	185	0	84	0 0 0
PasserByVol:	0 0	0	0 0	0	0 0	0	0	0 0 0
Initial Fut:	0 1934	648	0 1833	1136	468	0	669	0 0 0
User Adj:	1.00 1.00	0.00	1.00 1.00	0.00	1.00 1.00	1.00	1.00 1.00	1.00
PHF Adj:	1.00 1.00	0.00	1.00 1.00	0.00	1.00 1.00	1.00	1.00 1.00	1.00
PHF Volume:	0 1934	0	0 1833	0	468	0	669	0 0 0
Reduc Vol:	0 0	0	0 0	0	0 0	0	0	0 0 0
Reduced Vol:	0 1934	0	0 1833	0	468	0	669	0 0 0
PCE Adj:	1.00 1.00	0.00	1.00 1.00	0.00	1.00 1.00	1.00	1.00 1.00	1.00
MLF Adj:	1.00 1.00	0.00	1.00 1.00	0.00	1.00 1.00	1.00	1.00 1.00	1.00
FinalVolume:	0 1934	0	0 1833	0	468	0	669	0 0 0

Saturation Flow Module:

Sat/Lane:	1900 1900	1900	1900 1900	1900	1900 1900	1900	1900 1900	1900
Adjustment:	0.92 1.00	0.92	0.92 1.00	0.92	0.83 1.00	0.83	0.92 1.00	0.92
Lanes:	0.00 2.00	1.00	0.00 3.00	1.00	2.00 0.00	2.00	0.00 0.00	0.00
Final Sat.:	0 3800	1750	0 5700	1750	3150 0	3150	0 0 0	0

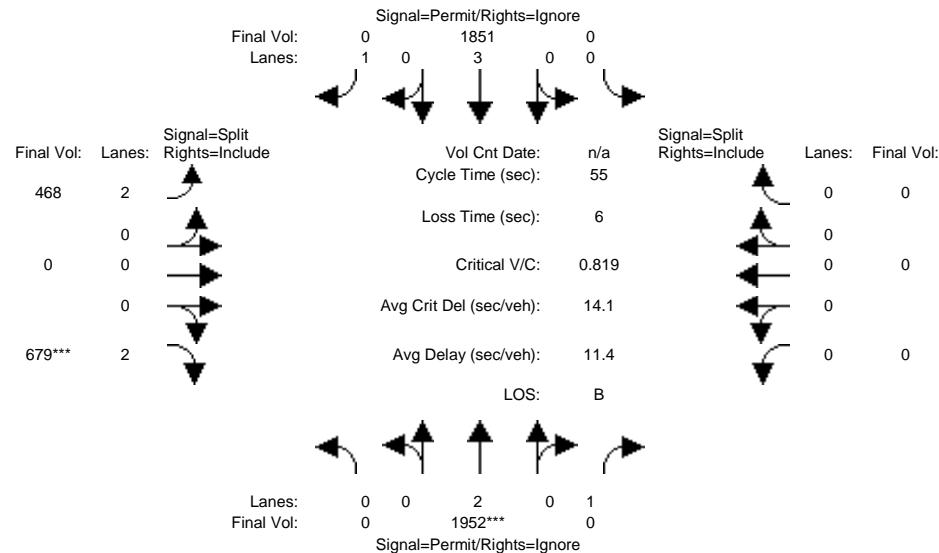
Capacity Analysis Module:

Vol/Sat:	0.00 0.51	0.00	0.00 0.32	0.00	0.15 0.00	0.21	0.00 0.00	0.00
Crit Moves:	****				****			
Green Time:	0.0 34.6	0.0	0.0 34.6	0.0	14.4 0.0	14.4	0.0 0.0	0.0
Volume/Cap:	0.00 0.81	0.00	0.00 0.51	0.00	0.57 0.00	0.81	0.00 0.00	0.00
Uniform Del:	0.0 7.7	0.0	0.0 5.6	0.0	17.6 0.0	19.0	0.0 0.0	0.0
IncremntDel:	0.0 2.2	0.0	0.0 0.1	0.0	0.9 0.0	6.0	0.0 0.0	0.0
InitQueueDel:	0.0 0.0	0.0	0.0 0.0	0.0	0.0 0.0	0.0	0.0 0.0	0.0
Delay Adj:	0.00 1.00	0.00	0.00 1.00	0.00	1.00 0.00	1.00	0.00 0.00	0.00
Delay/Veh:	0.0 9.9	0.0	0.0 5.7	0.0	18.5 0.0	25.0	0.0 0.0	0.0
User DelAdj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00
AdjDel/Veh:	0.0 9.9	0.0	0.0 5.7	0.0	18.5 0.0	25.0	0.0 0.0	0.0
LOS by Move:	A A	A A	A A	B A	C A	A A	A A	A A
HCM2kAvgQ:	0 12	0	0 5	0 5	0 9	0 0	0 0	0 0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + P PM

Intersection #22: Wolfe Road and I-280 SB Ramps (CUP/CMP)



Street Name:		Wolfe Road								I-280 SB Ramps			
Approach:	North Bound	South Bound				East Bound				West Bound			
Movement:	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R
Min. Green:	7	10	10	7	10	10	10	10	10	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	1641	642	0	1662	730	283	0	585	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1641	642	0	1662	730	283	0	585	0	0	0	0
Added Vol:	0	293	6	0	171	406	185	0	84	0	0	0	0
Hyatt:	0	18	9	0	18	0	0	0	10	0	0	0	0
Initial Fut:	0	1952	657	0	1851	1136	468	0	679	0	0	0	0
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1952	0	0	1851	0	468	0	679	0	0	0	0
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1952	0	0	1851	0	468	0	679	0	0	0	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1952	0	0	1851	0	468	0	679	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.83	0.92	1.00	0.92	0.92
Lanes:	0.00	2.00	1.00	0.00	3.00	1.00	2.00	0.00	2.00	0.00	0.00	0.00	0.00
Final Sat.:	0	3800	1750	0	5700	1750	3150	0	3150	0	0	0	0

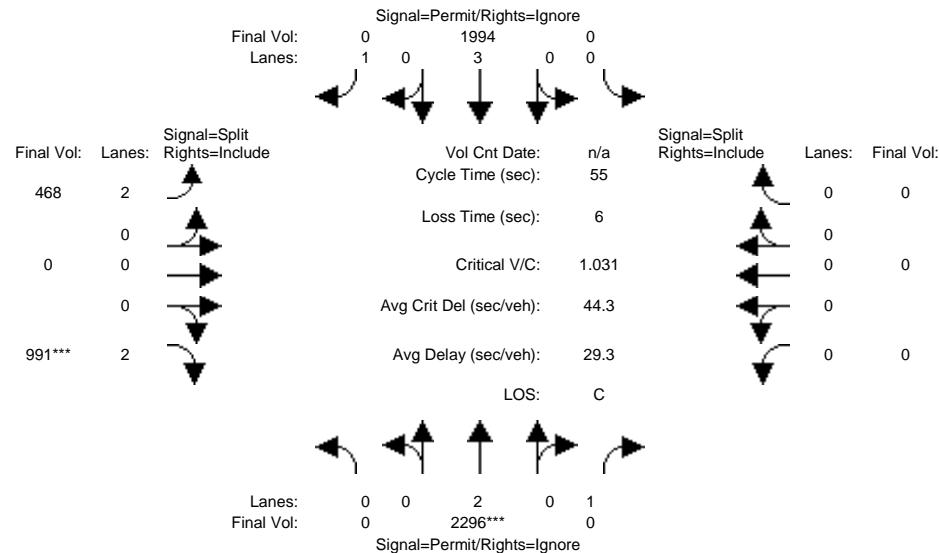
Capacity Analysis Module:

Vol/Sat:	0.00	0.51	0.00	0.00	0.32	0.00	0.15	0.00	0.22	0.00	0.00	0.00	0.00
Crit Moves:	****						****						
Green Time:	0.0	34.5	0.0	0.0	34.5	0.0	14.5	0.0	14.5	0.0	0.0	0.0	0.0
Volume/Cap:	0.00	0.82	0.00	0.00	0.52	0.00	0.56	0.00	0.82	0.00	0.00	0.00	0.00
Uniform Del:	0.0	7.8	0.0	0.0	5.6	0.0	17.5	0.0	19.0	0.0	0.0	0.0	0.0
IncremntDel:	0.0	2.3	0.0	0.0	0.1	0.0	0.9	0.0	6.4	0.0	0.0	0.0	0.0
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	0.00	1.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00	0.00	0.00
Delay/Veh:	0.0	10.2	0.0	0.0	5.8	0.0	18.4	0.0	25.5	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	10.2	0.0	0.0	5.8	0.0	18.4	0.0	25.5	0.0	0.0	0.0	0.0
LOS by Move:	A	B	A	A	A	A	B	A	C	A	A	A	A
HCM2kAvgQ:	0	12	0	0	5	0	5	0	9	0	0	0	0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative PM

Intersection #22: Wolfe Road and I-280 SB Ramps (CUP/CMP)

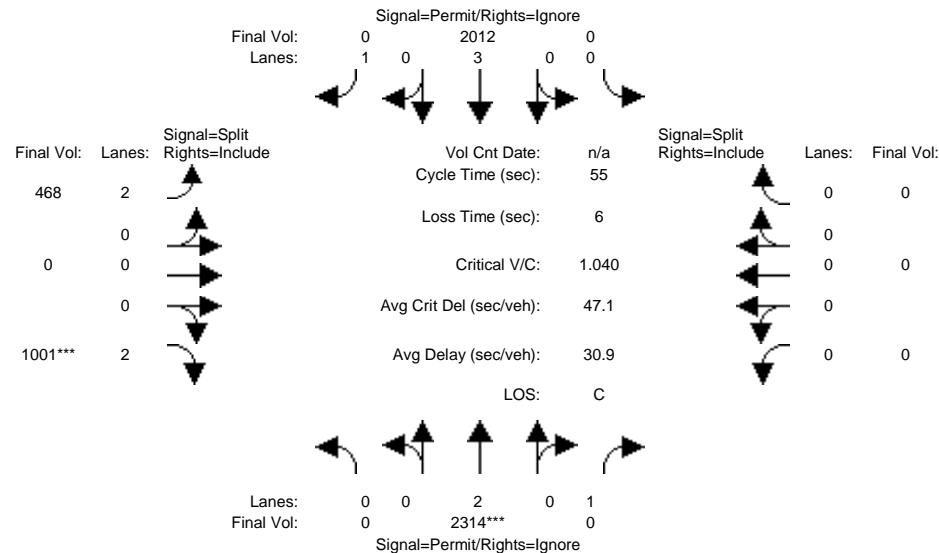


Street Name: Wolfe Road I-280 SB Ramps															
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7 10		10 7		10 4.0		10 4.0		10 4.0		0 0		0 0		
Y+R:	4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		
Volume Module:	<hr/>														
Base Vol:	0	2003	763	0	1823	730	283	0	907	0	0	0	0		
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	0	2003	763	0	1823	730	283	0	907	0	0	0			
Added Vol:	0	293	6	0	171	406	185	0	84	0	0	0			
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0			
Initial Fut:	0	2296	769	0	1994	1136	468	0	991	0	0	0			
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Volume:	0	2296	0	0	1994	0	468	0	991	0	0	0			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	0	2296	0	0	1994	0	468	0	991	0	0	0			
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
FinalVolume:	0	2296	0	0	1994	0	468	0	991	0	0	0			
Saturation Flow Module:	<hr/>														
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900			
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.83	0.92	1.00	0.92			
Lanes:	0.00	2.00	1.00	0.00	3.00	1.00	2.00	0.00	2.00	0.00	0.00	0.00			
Final Sat.:	0	3800	1750	0	5700	1750	3150	0	3150	0	0	0			
Capacity Analysis Module:	<hr/>														
Vol/Sat:	0.00	0.60	0.00	0.00	0.35	0.00	0.15	0.00	0.31	0.00	0.00	0.00			
Crit Moves:	****														
Green Time:	0.0	32.2	0.0	0.0	32.2	0.0	16.8	0.0	16.8	0.0	0.0	0.0			
Volume/Cap:	0.00	1.03	0.00	0.00	0.60	0.00	0.49	0.00	1.03	0.00	0.00	0.00			
Delay/Veh:	0.0	39.0	0.0	0.0	7.6	0.0	16.0	0.0	56.5	0.0	0.0	0.0			
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
AdjDel/Veh:	0.0	39.0	0.0	0.0	7.6	0.0	16.0	0.0	56.5	0.0	0.0	0.0			
LOS by Move:	A	D	A	A	A	A	B	A	E	A	A	A			
HCM2kAvgQ:	0	26	0	0	7	0	4	0	19	0	0	0			

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + P PM

Intersection #22: Wolfe Road and I-280 SB Ramps (CUP/CMP)



Street Name:		Wolfe Road										I-280 SB Ramps					
Approach:	North Bound	South Bound				East Bound				West Bound							
Movement:	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R
Min. Green:	7	10	10	7	10	10	10	10	10	10	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	2003	763	0	1823	730	283	0	907	0	0	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	2003	763	0	1823	730	283	0	907	0	0	0	0	0	0	0	0
Added Vol:	0	293	6	0	171	406	185	0	84	0	0	0	0	0	0	0	0
Hyatt:	0	18	9	0	18	0	0	0	10	0	0	0	0	0	0	0	0
Initial Fut:	0	2314	778	0	2012	1136	468	0	1001	0	0	0	0	0	0	0	0
User Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	2314	0	0	2012	0	468	0	1001	0	0	0	0	0	0	0	0
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	2314	0	0	2012	0	468	0	1001	0	0	0	0	0	0	0	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	2314	0	0	2012	0	468	0	1001	0	0	0	0	0	0	0	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.83	0.92	1.00	0.92	1.00	0.92	1.00	0.92
Lanes:	0.00	2.00	1.00	0.00	3.00	1.00	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Final Sat.:	0	3800	1750	0	5700	1750	3150	0	3150	0	0	0	0	0	0	0

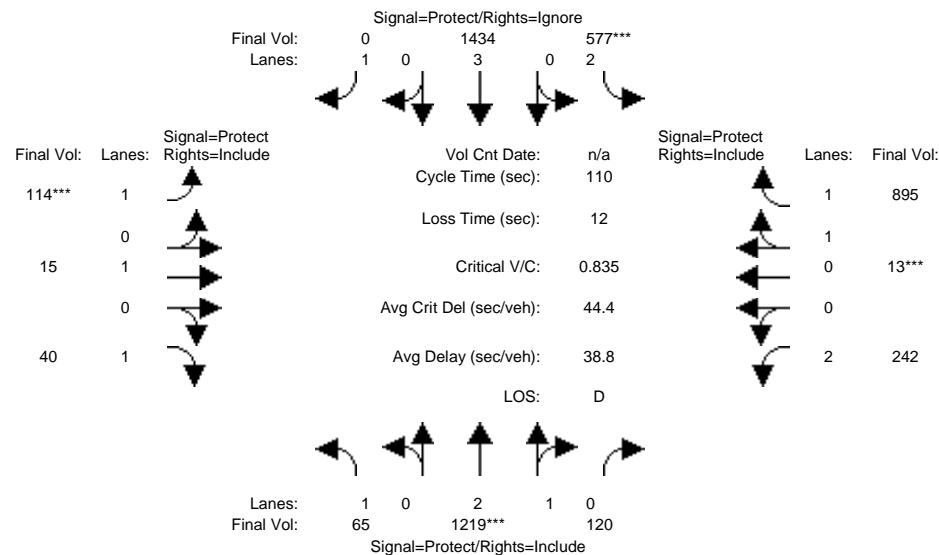
Capacity Analysis Module:

Vol/Sat:	0.00	0.61	0.00	0.00	0.35	0.00	0.15	0.00	0.32	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****						****									
Green Time:	0.0	32.2	0.0	0.0	32.2	0.0	16.8	0.0	16.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Volume/Cap:	0.00	1.04	0.00	0.00	0.60	0.00	0.49	0.00	1.04	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	0.0	41.9	0.0	0.0	7.6	0.0	16.0	0.0	59.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	41.9	0.0	0.0	7.6	0.0	16.0	0.0	59.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0
LOS by Move:	A	D	A	A	A	A	B	A	E	A	A	A				
HCM2kAvgQ:	0	27	0	0	7	0	4	0	19	0	0	0				

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd PM

Intersection #23: Wolfe Road and Vallco Parkway (CUP)



Street Name:	Wolfe Road					Vallco Parkway									
Approach:	North Bound			South Bound		East Bound			West Bound						
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	7	10	10	7	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	65	1141	120	487	1269	166	114	15	40	242	13	674
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	65	1141	120	487	1269	166	114	15	40	242	13	674
Added Vol:	0	78	0	90	165	0	0	0	0	0	0	221
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	65	1219	120	577	1434	166	114	15	40	242	13	895
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	65	1219	120	577	1434	0	114	15	40	242	13	895
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	65	1219	120	577	1434	0	114	15	40	242	13	895
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	65	1219	120	577	1434	0	114	15	40	242	13	895

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92
Lanes:	1.00	2.71	0.29	2.00	3.00	1.00	1.00	1.00	1.00	2.00	0.03	1.97
Final Sat.:	1750	5150	507	3150	5700	1750	1750	1900	1750	3150	50	3454

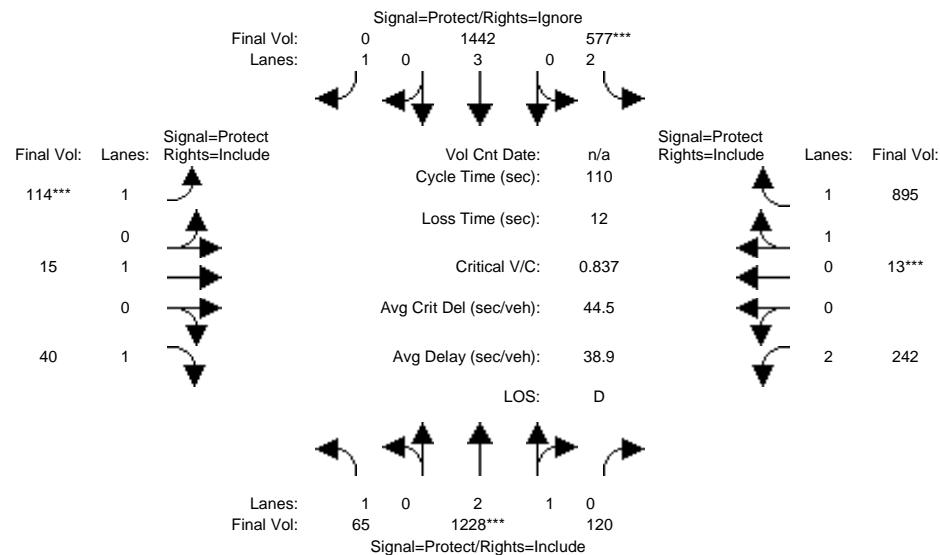
Capacity Analysis Module:

Vol/Sat:	0.04	0.24	0.24	0.18	0.25	0.00	0.07	0.01	0.02	0.08	0.26	0.26
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	11.2	31.2	31.2	24.1	44.1	0.0	8.6	23.1	23.1	19.6	34.1	34.1
Volume/Cap:	0.37	0.84	0.84	0.84	0.63	0.00	0.84	0.04	0.11	0.43	0.84	0.84
Uniform Del:	46.1	37.0	37.0	41.0	26.3	0.0	50.0	34.6	35.1	40.3	35.3	35.3
IncremntDel:	1.3	4.0	4.0	8.7	0.6	0.0	33.8	0.0	0.1	0.5	5.7	5.7
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	47.4	41.0	41.0	49.7	26.9	0.0	83.8	34.6	35.2	40.8	41.1	41.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	47.4	41.0	41.0	49.7	26.9	0.0	83.8	34.6	35.2	40.8	41.1	41.1
LOS by Move:	D	D	D	D	C	A	F	C	D	D	D	D
HCM2kAvgQ:	2	14	14	12	13	0	6	0	1	4	16	16

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + P PM

Intersection #23: Wolfe Road and Vallco Parkway (CUP)



Street Name:		Wolfe Road				Vallco Parkway			
Approach:	North Bound	South Bound		East Bound		West Bound			
Movement:	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R	L - T - R		
Min. Green:	7 10	10 7	10 10	10 7	10 10	10 7	10 10	7 10	10
Y+R:	4.0 4.0	4.0 4.0	4.0 4.0	4.0 4.0	4.0 4.0	4.0 4.0	4.0 4.0	4.0 4.0	4.0

Volume Module:

Base Vol:	65	1141	120	487	1269	166	114	15	40	242	13	674
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	65	1141	120	487	1269	166	114	15	40	242	13	674
Added Vol:	0	78	0	90	165	0	0	0	0	0	0	221
Hyatt:	0	9	0	0	8	0	0	0	0	0	0	0
Initial Fut:	65	1228	120	577	1442	166	114	15	40	242	13	895
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	65	1228	120	577	1442	0	114	15	40	242	13	895
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	65	1228	120	577	1442	0	114	15	40	242	13	895
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	65	1228	120	577	1442	0	114	15	40	242	13	895

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.83	1.00
Lanes:	1.00	2.71	0.29	2.00	3.00	1.00	1.00	1.00	1.00	2.00	0.03
Final Sat.:	1750	5153	504	3150	5700	1750	1750	1900	1750	3150	50

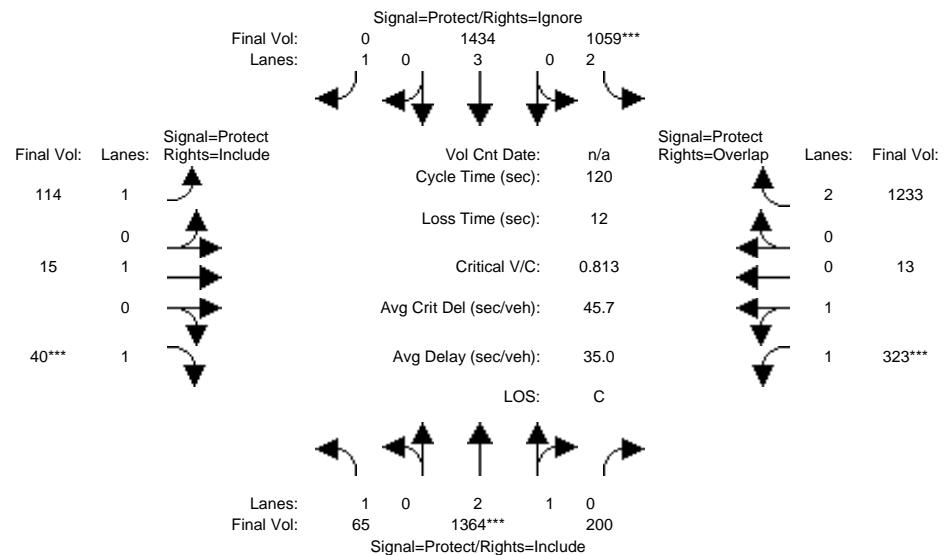
Capacity Analysis Module:

Vol/Sat:	0.04	0.24	0.24	0.18	0.25	0.00	0.07	0.01	0.02	0.08	0.26	0.26
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	11.1	31.3	31.3	24.1	44.3	0.0	8.6	23.1	23.1	19.5	34.1	34.1
Volume/Cap:	0.37	0.84	0.84	0.84	0.63	0.00	0.84	0.04	0.11	0.43	0.84	0.84
Uniform Del:	46.1	36.9	36.9	41.1	26.3	0.0	50.0	34.6	35.1	40.3	35.4	35.4
IncremntDel:	1.3	4.0	4.0	8.8	0.6	0.0	34.2	0.0	0.1	0.5	5.8	5.8
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	47.4	41.0	41.0	49.9	26.9	0.0	84.3	34.6	35.3	40.9	41.2	41.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	47.4	41.0	41.0	49.9	26.9	0.0	84.3	34.6	35.3	40.9	41.2	41.2
LOS by Move:	D	D	D	D	C	A	F	C	D	D	D	D
HCM2kAvgQ:	2	15	15	12	13	0	6	0	1	4	16	16

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative PM

Intersection #23: Wolfe Road and Vallco Parkway (CUP)



Street Name:	Wolfe Road						Vallco Parkway								
	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	10	
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	

Volume Module:

Base Vol:	65	1286	200	969	1269	166	114	15	40	323	13	1012
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	65	1286	200	969	1269	166	114	15	40	323	13	1012
Added Vol:	0	78	0	90	165	0	0	0	0	0	0	221
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	65	1364	200	1059	1434	166	114	15	40	323	13	1233
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	65	1364	200	1059	1434	0	114	15	40	323	13	1233
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	65	1364	200	1059	1434	0	114	15	40	323	13	1233
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	65	1364	200	1059	1434	0	114	15	40	323	13	1233

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.83
Lanes:	1.00	2.59	0.41	2.00	3.00	1.00	1.00	1.00	1.00	1.93	0.07	2.00
Final Sat.:	1750	4917	721	3150	5700	1750	1750	1900	1750	3375	136	3150

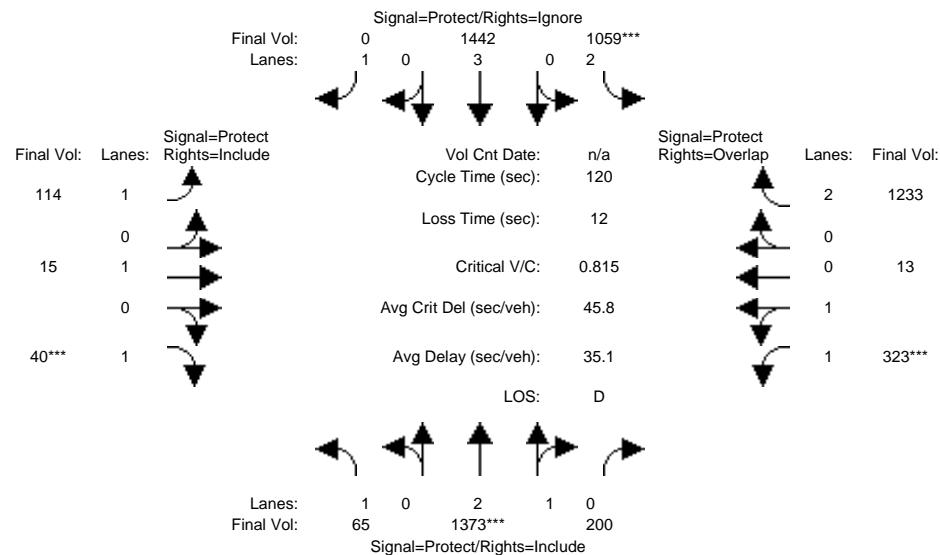
Capacity Analysis Module:

Vol/Sat:	0.04	0.28	0.28	0.34	0.25	0.00	0.07	0.01	0.02	0.10	0.10	0.39
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	16.0	38.3	38.3	46.5	68.8	0.0	9.4	10.0	10.0	13.2	13.8	60.3
Volume/Cap:	0.28	0.87	0.87	0.86	0.44	0.00	0.83	0.09	0.27	0.87	0.83	0.78
Delay/Veh:	47.5	43.3	43.3	40.9	14.7	0.0	87.4	51.1	52.6	71.0	65.5	27.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	47.5	43.3	43.3	40.9	14.7	0.0	87.4	51.1	52.6	71.0	65.5	27.0
LOS by Move:	D	D	D	D	B	A	F	D	D	E	E	C
HCM2kAvgQ:	2	18	18	22	10	0	7	1	2	7	7	22

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + P PM

Intersection #23: Wolfe Road and Vallco Parkway (CUP)



Street Name:	Wolfe Road						Vallco Parkway								
	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10		
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		

Volume Module:

Base Vol:	65	1286	200	969	1269	166	114	15	40	323	13	1012	
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Initial Bse:	65	1286	200	969	1269	166	114	15	40	323	13	1012	
Added Vol:	0	78	0	90	165	0	0	0	0	0	0	0	221
Hyatt:	0	9	0	0	8	0	0	0	0	0	0	0	0
Initial Fut:	65	1373	200	1059	1442	166	114	15	40	323	13	1233	
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	
PHF Volume:	65	1373	200	1059	1442	0	114	15	40	323	13	1233	
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	65	1373	200	1059	1442	0	114	15	40	323	13	1233	
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	
FinalVolume:	65	1373	200	1059	1442	0	114	15	40	323	13	1233	

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.83
Lanes:	1.00	2.59	0.41	2.00	3.00	1.00	1.00	1.00	1.00	1.93	0.07	2.00
Final Sat.:	1750	4922	717	3150	5700	1750	1750	1900	1750	3375	136	3150

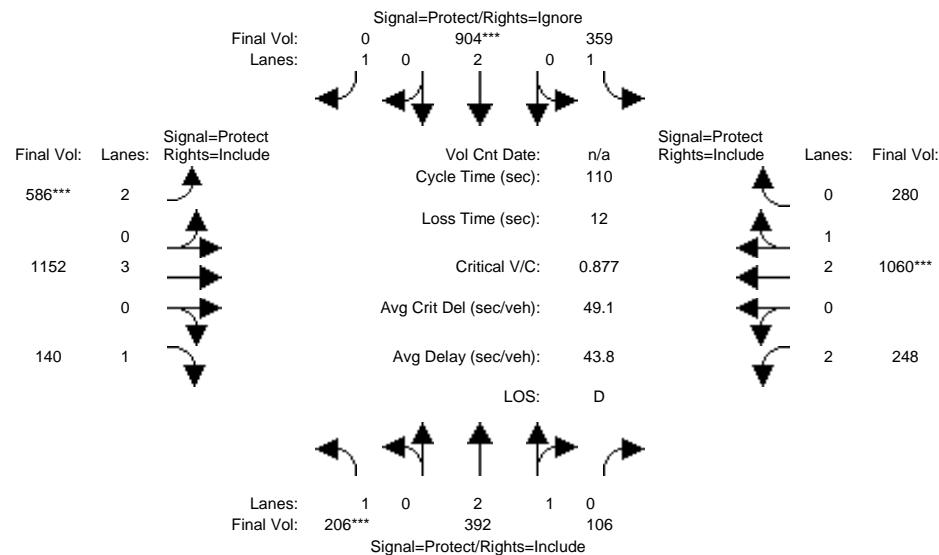
Capacity Analysis Module:

Vol/Sat:	0.04	0.28	0.28	0.34	0.25	0.00	0.07	0.01	0.02	0.10	0.10	0.39
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	15.9	38.5	38.5	46.3	68.9	0.0	9.4	10.0	10.0	13.2	13.8	60.1
Volume/Cap:	0.28	0.87	0.87	0.87	0.44	0.00	0.83	0.09	0.27	0.87	0.83	0.78
Delay/Veh:	47.6	43.3	43.3	41.1	14.7	0.0	87.7	51.1	52.6	71.3	65.7	27.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	47.6	43.3	43.3	41.1	14.7	0.0	87.7	51.1	52.6	71.3	65.7	27.1
LOS by Move:	D	D	D	D	B	A	F	D	D	E	E	C
HCM2kAvgQ:	2	18	18	23	10	0	7	1	2	7	7	23

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkdg PM

Intersection #24: Stevens Creek Blvd and Wolfe Rd/Miller Ave (CUP/CMP)

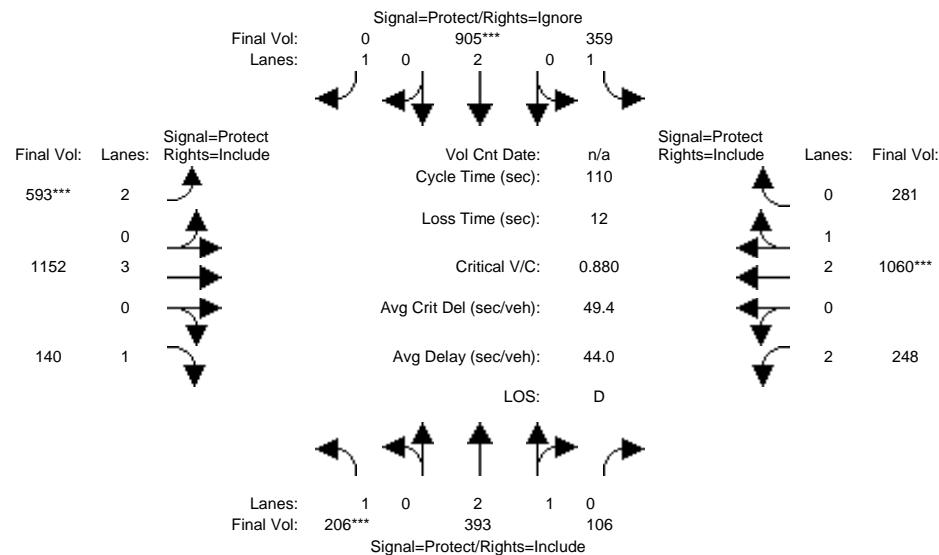


Street Name: Wolfe Rd/Miller Ave Stevens Creek Blvd															
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7 10		10 7		10 10		10 7		10 10		7 10		10 10		
Y+R:	4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		
Volume Module:	<hr/>														
Base Vol:	206	385	105	293	881	526	561	1125	140	234	1021	234			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	206	385	105	293	881	526	561	1125	140	234	1021	234			
Added Vol:	0	7	1	66	23	77	25	27	0	14	39	46			
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0			
Initial Fut:	206	392	106	359	904	603	586	1152	140	248	1060	280			
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Volume:	206	392	106	359	904	0	586	1152	140	248	1060	280			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	206	392	106	359	904	0	586	1152	140	248	1060	280			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
FinalVolume:	206	392	106	359	904	0	586	1152	140	248	1060	280			
Saturation Flow Module:	<hr/>														
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900			
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92	0.83	1.00	0.92			
Lanes:	1.00	2.32	0.68	1.00	2.00	1.00	2.00	3.00	1.00	2.00	2.33	0.67			
Final Sat.:	1750	4406	1192	1750	3800	1750	3150	5700	1750	3150	4430	1170			
Capacity Analysis Module:	<hr/>														
Vol/Sat:	0.12	0.09	0.09	0.21	0.24	0.00	0.19	0.20	0.08	0.08	0.24	0.24			
Crit Moves:	****		****		****		****		****		****				
Green Time:	14.8	13.7	13.7	30.9	29.9	0.0	23.3	38.4	38.4	15.0	30.0	30.0			
Volume/Cap:	0.88	0.71	0.71	0.73	0.88	0.00	0.88	0.58	0.23	0.58	0.88	0.88			
Uniform Del:	46.7	46.3	46.3	35.8	38.3	0.0	41.9	29.2	25.3	44.6	38.2	38.2			
IncremntDel:	28.8	3.5	3.5	5.5	8.6	0.0	12.5	0.4	0.2	2.0	6.1	6.1			
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
Delay Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00			
Delay/Veh:	75.5	49.8	49.8	41.3	46.9	0.0	54.5	29.6	25.5	46.5	44.3	44.3			
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
AdjDel/Veh:	75.5	49.8	49.8	41.3	46.9	0.0	54.5	29.6	25.5	46.5	44.3	44.3			
LOS by Move:	E	D	D	D	D	A	D	C	C	D	D	D			
HCM2kAvgQ:	10	7	7	12	15	0	15	11	4	5	15	15			

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + P PM

Intersection #24: Stevens Creek Blvd and Wolfe Rd/Miller Ave (CUP/CMP)



Street Name:	Wolfe Rd/Miller Ave					Stevens Creek Blvd									
Approach:	North Bound			South Bound		East Bound			West Bound						
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	10	
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	

Volume Module:

Base Vol:	206	385	105	293	881	526	561	1125	140	234	1021	234
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	206	385	105	293	881	526	561	1125	140	234	1021	234
Added Vol:	0	7	1	66	23	77	25	27	0	14	39	46
Hyatt:	0	1	0	0	1	6	7	0	0	0	0	1
Initial Fut:	206	393	106	359	905	609	593	1152	140	248	1060	281
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	206	393	106	359	905	0	593	1152	140	248	1060	281
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	206	393	106	359	905	0	593	1152	140	248	1060	281
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	206	393	106	359	905	0	593	1152	140	248	1060	281

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92	0.83	1.00	0.92
Lanes:	1.00	2.32	0.68	1.00	2.00	1.00	2.00	3.00	1.00	2.00	2.33	0.67
Final Sat.:	1750	4409	1189	1750	3800	1750	3150	5700	1750	3150	4426	1173

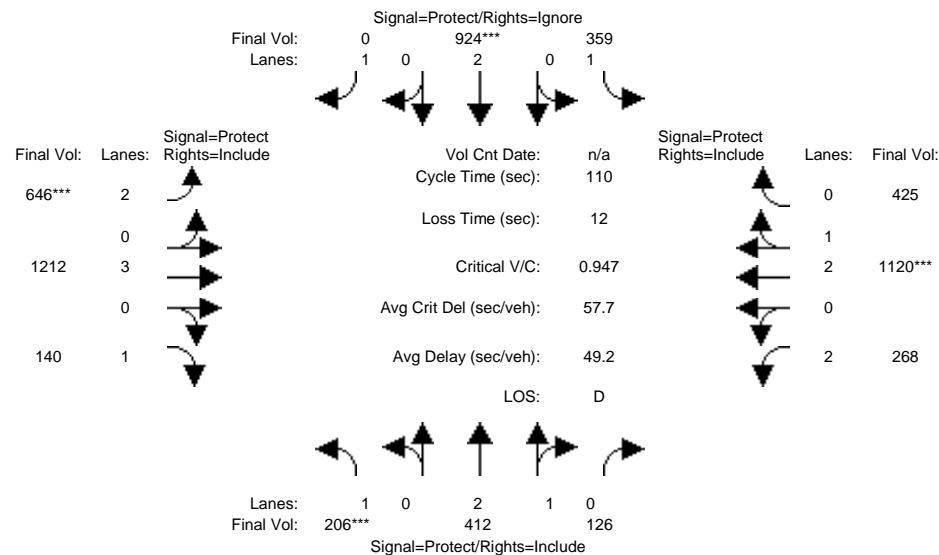
Capacity Analysis Module:

Vol/Sat:	0.12	0.09	0.09	0.21	0.24	0.00	0.19	0.20	0.08	0.08	0.24	0.24
Crit Moves:	****			****		****	****		****		****	
Green Time:	14.7	13.7	13.7	30.8	29.8	0.0	23.5	38.5	38.5	15.0	30.0	30.0
Volume/Cap:	0.88	0.72	0.72	0.73	0.88	0.00	0.88	0.58	0.23	0.58	0.88	0.88
Uniform Del:	46.8	46.3	46.3	35.8	38.4	0.0	41.9	29.1	25.3	44.5	38.3	38.3
IncremntDel:	29.4	3.6	3.6	5.6	8.9	0.0	12.7	0.4	0.2	2.0	6.2	6.2
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	76.2	49.9	49.9	41.4	47.3	0.0	54.6	29.5	25.5	46.5	44.5	44.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	76.2	49.9	49.9	41.4	47.3	0.0	54.6	29.5	25.5	46.5	44.5	44.5
LOS by Move:	E	D	D	D	D	A	D	C	C	D	D	D
HCM2kAvgQ:	10	7	7	12	15	0	15	11	4	5	15	15

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative PM

Intersection #24: Stevens Creek Blvd and Wolfe Rd/Miller Ave (CUP/CMP)



Street Name:	Wolfe Rd/Miller Ave					Stevens Creek Blvd									
Approach:	North Bound			South Bound		East Bound			West Bound						
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	10	
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	

Volume Module:

Base Vol:	206	405	125	293	901	586	621	1185	140	254	1081	379
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	206	405	125	293	901	586	621	1185	140	254	1081	379
Added Vol:	0	7	1	66	23	77	25	27	0	14	39	46
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	206	412	126	359	924	663	646	1212	140	268	1120	425
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	206	412	126	359	924	0	646	1212	140	268	1120	425
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	206	412	126	359	924	0	646	1212	140	268	1120	425
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	206	412	126	359	924	0	646	1212	140	268	1120	425

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92	0.83	1.00	0.92
Lanes:	1.00	2.25	0.75	1.00	2.00	1.00	2.00	3.00	1.00	2.00	2.12	0.88
Final Sat.:	1750	4279	1309	1750	3800	1750	3150	5700	1750	3150	4037	1532

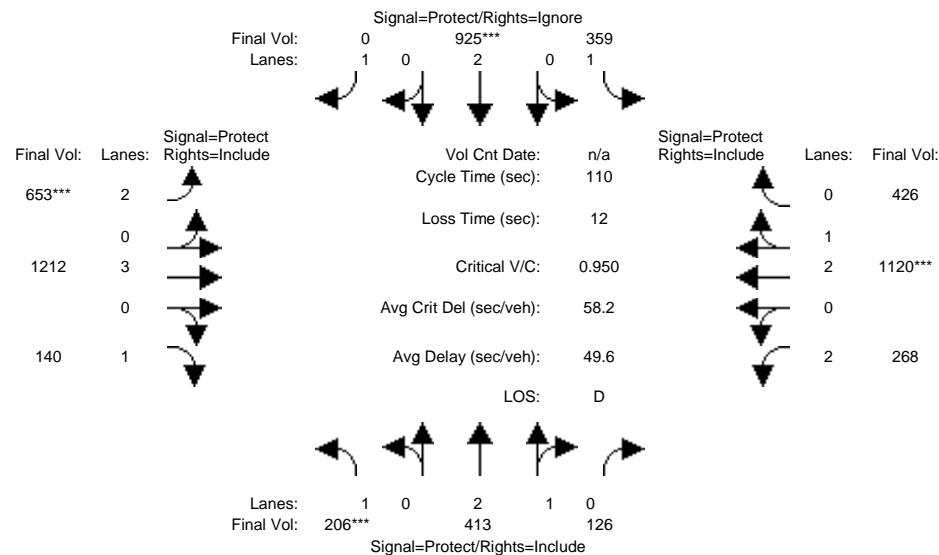
Capacity Analysis Module:

Vol/Sat:	0.12	0.10	0.10	0.21	0.24	0.00	0.21	0.21	0.08	0.09	0.28	0.28
Crit Moves:	****			****		****	****			****		
Green Time:	13.7	13.4	13.4	28.5	28.3	0.0	23.8	40.0	40.0	16.0	32.2	32.2
Volume/Cap:	0.95	0.79	0.79	0.79	0.95	0.00	0.95	0.58	0.22	0.58	0.95	0.95
Delay/Veh:	93.8	53.2	53.2	47.1	57.5	0.0	64.7	28.7	24.4	45.8	50.0	50.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	93.8	53.2	53.2	47.1	57.5	0.0	64.7	28.7	24.4	45.8	50.0	50.0
LOS by Move:	F	D	D	D	E	A	E	C	C	D	D	D
HCM2kAvgQ:	11	8	8	13	18	0	17	11	3	5	17	17

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + P PM

Intersection #24: Stevens Creek Blvd and Wolfe Rd/Miller Ave (CUP/CMP)



Street Name:	Wolfe Rd/Miller Ave					Stevens Creek Blvd									
Approach:	North Bound			South Bound		East Bound			West Bound						
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	10	
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	

Volume Module:

Base Vol:	206	405	125	293	901	586	621	1185	140	254	1081	379
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	206	405	125	293	901	586	621	1185	140	254	1081	379
Added Vol:	0	7	1	66	23	77	25	27	0	14	39	46
Hyatt:	0	1	0	0	1	6	7	0	0	0	0	1
Initial Fut:	206	413	126	359	925	669	653	1212	140	268	1120	426
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	206	413	126	359	925	0	653	1212	140	268	1120	426
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	206	413	126	359	925	0	653	1212	140	268	1120	426
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	206	413	126	359	925	0	653	1212	140	268	1120	426

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.83	1.00	0.92	0.83	1.00	0.92
Lanes:	1.00	2.25	0.75	1.00	2.00	1.00	2.00	3.00	1.00	2.00	2.12	0.88
Final Sat.:	1750	4282	1306	1750	3800	1750	3150	5700	1750	3150	4034	1534

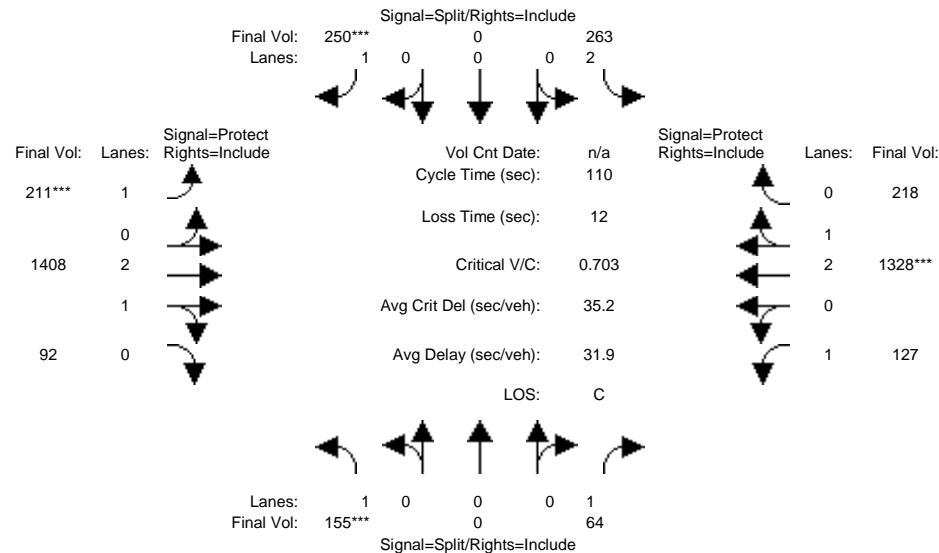
Capacity Analysis Module:

Vol/Sat:	0.12	0.10	0.10	0.21	0.24	0.00	0.21	0.21	0.08	0.09	0.28	0.28
Crit Moves:	****			****		****	****			****		
Green Time:	13.6	13.4	13.4	28.5	28.2	0.0	24.0	40.1	40.1	16.1	32.2	32.2
Volume/Cap:	0.95	0.79	0.79	0.79	0.95	0.00	0.95	0.58	0.22	0.58	0.95	0.95
Delay/Veh:	94.7	53.3	53.3	47.3	58.1	0.0	65.1	28.6	24.3	45.8	50.5	50.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	94.7	53.3	53.3	47.3	58.1	0.0	65.1	28.6	24.3	45.8	50.5	50.5
LOS by Move:	F	D	D	D	E	A	E	C	C	D	D	D
HCM2kAvgQ:	11	8	8	13	18	0	18	11	3	5	17	17

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd PM

Intersection #26: Stevens Creek Blvd and Finch Avenue (CUP)



Street Name:	Finch Avenue					Stevens Creek Blvd									
Approach:	North Bound			South Bound		East Bound			West Bound						
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	10	10	10	10	10	10	7	10	10	7	10	10			
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0			

Volume Module:

Base Vol:	155	0	64	263	0	250	211	1314	92	114	1230	218			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	155	0	64	263	0	250	211	1314	92	114	1230	218			
Added Vol:	0	0	0	0	0	0	0	94	0	13	98	0			
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0			
Initial Fut:	155	0	64	263	0	250	211	1408	92	127	1328	218			
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Volume:	155	0	64	263	0	250	211	1408	92	127	1328	218			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	155	0	64	263	0	250	211	1408	92	127	1328	218			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
FinalVolume:	155	0	64	263	0	250	211	1408	92	127	1328	218			

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92		
Lanes:	1.00	0.00	1.00	2.00	0.00	1.00	1.00	2.80	0.20	1.00	2.55	0.45		
Final Sat.:	1750	0	1750	3150	0	1750	1750	5322	348	1750	4838	794		

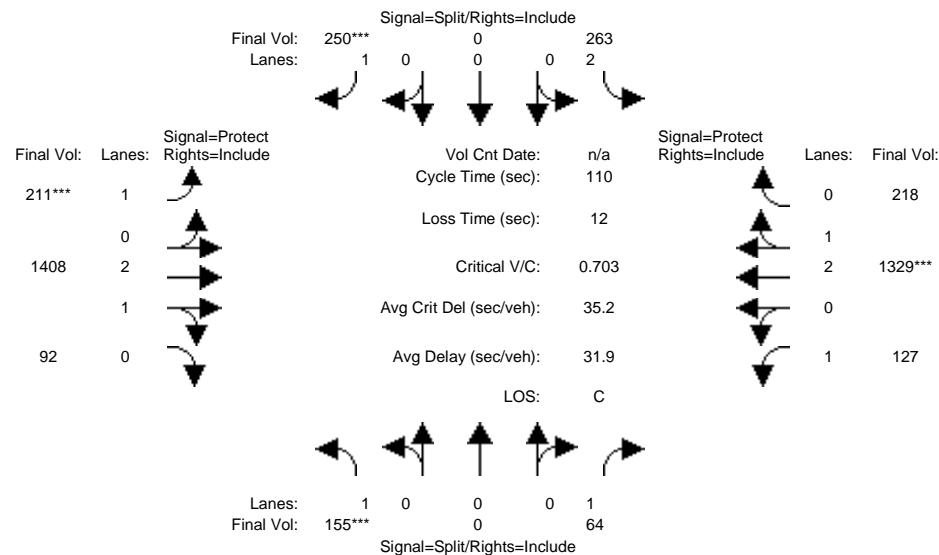
Capacity Analysis Module:

Vol/Sat:	0.09	0.00	0.04	0.08	0.00	0.14	0.12	0.26	0.26	0.07	0.27	0.27		
Crit Moves:	****			****	****	****	****	****	****	****	****	****		
Green Time:	13.9	0.0	13.9	22.3	0.0	22.3	18.9	48.5	48.5	13.3	42.9	42.9		
Volume/Cap:	0.70	0.00	0.29	0.41	0.00	0.70	0.70	0.60	0.60	0.60	0.70	0.70		
Uniform Del:	46.1	0.0	43.6	38.1	0.0	40.7	42.9	23.4	23.4	45.8	28.2	28.2		
IncremntDel:	9.8	0.0	0.7	0.4	0.0	6.2	7.3	0.4	0.4	4.7	1.0	1.0		
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Delay Adj:	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Delay/Veh:	55.9	0.0	44.3	38.5	0.0	47.0	50.3	23.8	23.8	50.6	29.2	29.2		
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
AdjDel/Veh:	55.9	0.0	44.3	38.5	0.0	47.0	50.3	23.8	23.8	50.6	29.2	29.2		
LOS by Move:	E	A	D	D	A	D	D	C	C	D	C	C		
HCM2kAvgQ:	7	0	2	5	0	10	7	12	12	4	14	14		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + P PM

Intersection #26: Stevens Creek Blvd and Finch Avenue (CUP)



Street Name:	Finch Avenue				Stevens Creek Blvd										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	10	10	10	10	10	10	7	10	10	10	7	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	155	0	64	263	0	250	211	1314	92	114	1230	218
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	155	0	64	263	0	250	211	1314	92	114	1230	218
Added Vol:	0	0	0	0	0	0	0	94	0	13	98	0
Hyatt:	0	0	0	0	0	0	0	0	0	0	1	0
Initial Fut:	155	0	64	263	0	250	211	1408	92	127	1329	218
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	155	0	64	263	0	250	211	1408	92	127	1329	218
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	155	0	64	263	0	250	211	1408	92	127	1329	218
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	155	0	64	263	0	250	211	1408	92	127	1329	218

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	1.00	0.00	1.00	2.00	0.00	1.00	1.00	2.80	0.20	1.00	2.55	0.45
Final Sat.:	1750	0	1750	3150	0	1750	1750	5322	348	1750	4838	794

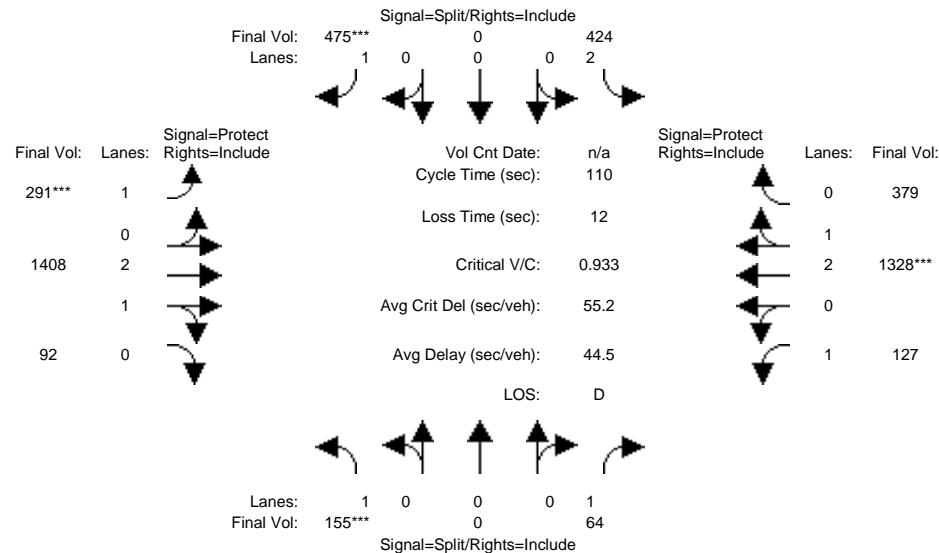
Capacity Analysis Module:

Vol/Sat:	0.09	0.00	0.04	0.08	0.00	0.14	0.12	0.26	0.26	0.07	0.27	0.27
Crit Moves:	****			****	****	****	****			****		
Green Time:	13.9	0.0	13.9	22.3	0.0	22.3	18.9	48.5	48.5	13.3	43.0	43.0
Volume/Cap:	0.70	0.00	0.29	0.41	0.00	0.70	0.70	0.60	0.60	0.60	0.70	0.70
Uniform Del:	46.1	0.0	43.6	38.1	0.0	40.8	42.9	23.4	23.4	45.8	28.2	28.2
IncremntDel:	9.8	0.0	0.7	0.4	0.0	6.3	7.3	0.4	0.4	4.7	1.0	1.0
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	55.9	0.0	44.4	38.5	0.0	47.0	50.3	23.8	23.8	50.5	29.2	29.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	55.9	0.0	44.4	38.5	0.0	47.0	50.3	23.8	23.8	50.5	29.2	29.2
LOS by Move:	E	A	D	D	A	D	D	C	C	D	C	C
HCM2kAvgQ:	7	0	2	5	0	10	7	12	12	4	14	14

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative PM

Intersection #26: Stevens Creek Blvd and Finch Avenue (CUP)



Street Name:	Finch Avenue				Stevens Creek Blvd										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	10	10	10	10	10	10	7	10	10	10	7	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	155	0	64	424	0	475	291	1314	92	114	1230	379
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	155	0	64	424	0	475	291	1314	92	114	1230	379
Added Vol:	0	0	0	0	0	0	0	94	0	13	98	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	155	0	64	424	0	475	291	1408	92	127	1328	379
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	155	0	64	424	0	475	291	1408	92	127	1328	379
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	155	0	64	424	0	475	291	1408	92	127	1328	379
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	155	0	64	424	0	475	291	1408	92	127	1328	379

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	1.00	0.00	1.00	2.00	0.00	1.00	1.00	2.80	0.20	1.00	2.29	0.71
Final Sat.:	1750	0	1750	3150	0	1750	1750	5322	348	1750	4352	1242

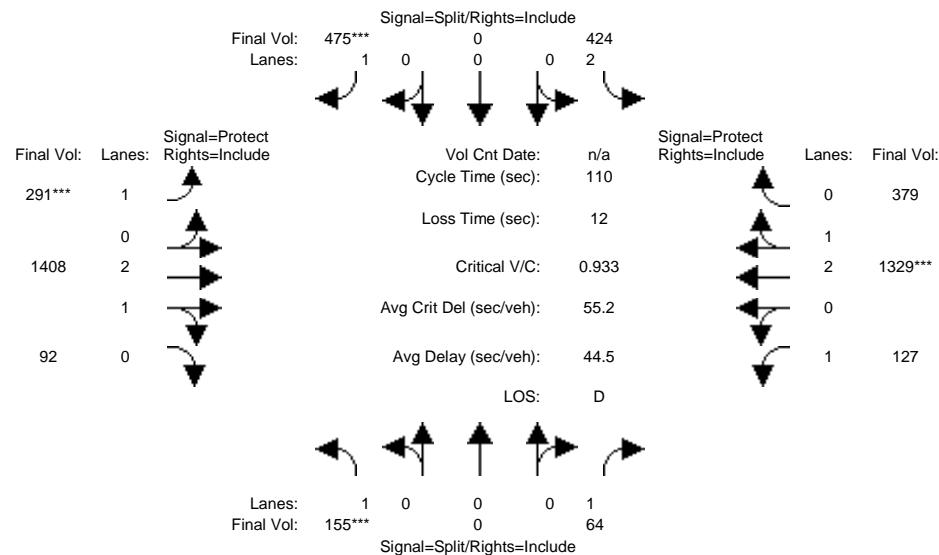
Capacity Analysis Module:

Vol/Sat:	0.09	0.00	0.04	0.13	0.00	0.27	0.17	0.26	0.26	0.07	0.31	0.31
Crit Moves:	****			****		****				****		
Green Time:	10.4	0.0	10.4	32.0	0.0	32.0	19.6	43.6	43.6	12.0	36.0	36.0
Volume/Cap:	0.93	0.00	0.39	0.46	0.00	0.93	0.93	0.67	0.67	0.67	0.93	0.93
Delay/Veh:	99.9	0.0	48.3	32.3	0.0	62.3	78.4	28.0	28.0	55.8	45.2	45.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	99.9	0.0	48.3	32.3	0.0	62.3	78.4	28.0	28.0	55.8	45.2	45.2
LOS by Move:	F	A	D	C	A	E	E	C	C	E	D	D
HCM2kAvgQ:	9	0	3	7	0	21	12	14	14	4	20	20

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + P PM

Intersection #26: Stevens Creek Blvd and Finch Avenue (CUP)



Street Name:	Finch Avenue					Stevens Creek Blvd									
Approach:	North Bound			South Bound		East Bound			West Bound						
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	10	10	10	10	10	10	7	10	10	7	10	10			
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0			

Volume Module:

Base Vol:	155	0	64	424	0	475	291	1314	92	114	1230	379			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	155	0	64	424	0	475	291	1314	92	114	1230	379			
Added Vol:	0	0	0	0	0	0	0	94	0	13	98	0			
Hyatt:	0	0	0	0	0	0	0	0	0	0	1	0			
Initial Fut:	155	0	64	424	0	475	291	1408	92	127	1329	379			
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Volume:	155	0	64	424	0	475	291	1408	92	127	1329	379			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	155	0	64	424	0	475	291	1408	92	127	1329	379			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
FinalVolume:	155	0	64	424	0	475	291	1408	92	127	1329	379			

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92		
Lanes:	1.00	0.00	1.00	2.00	0.00	1.00	1.00	2.80	0.20	1.00	2.29	0.71		
Final Sat.:	1750	0	1750	3150	0	1750	1750	5322	348	1750	4352	1241		

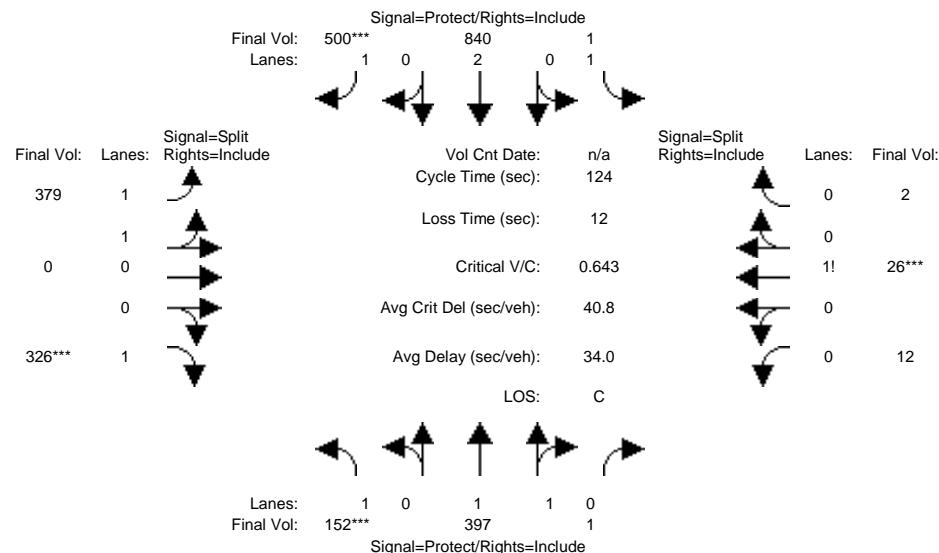
Capacity Analysis Module:

Vol/Sat:	0.09	0.00	0.04	0.13	0.00	0.27	0.17	0.26	0.26	0.07	0.31	0.31		
Crit Moves:	****			****		****	****			****				
Green Time:	10.4	0.0	10.4	32.0	0.0	32.0	19.6	43.6	43.6	12.0	36.0	36.0		
Volume/Cap:	0.93	0.00	0.39	0.46	0.00	0.93	0.93	0.67	0.67	0.67	0.93	0.93		
Delay/Veh:	100.0	0.0	48.3	32.3	0.0	62.4	78.5	28.0	28.0	55.8	45.2	45.2		
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
AdjDel/Veh:	100.0	0.0	48.3	32.3	0.0	62.4	78.5	28.0	28.0	55.8	45.2	45.2		
LOS by Move:	F	A	D	C	A	E	E	C	C	E	D	D		
HCM2kAvgQ:	9	0	3	7	0	21	12	14	14	4	20	20		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd PM

Intersection #31: Tantau Avenue and Vallco Parkway (CUP)



Street Name:	Tantau Avenue					Vallco Parkway									
	North Bound			South Bound		East Bound			West Bound						
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	10	10	10	10	10	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	152	222	1	1	500	279	289	0	326	12	26	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	152	222	1	1	500	279	289	0	326	12	26	2
Added Vol:	0	175	0	0	340	221	90	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	152	397	1	1	840	500	379	0	326	12	26	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	152	397	1	1	840	500	379	0	326	12	26	2
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	152	397	1	1	840	500	379	0	326	12	26	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	152	397	1	1	840	500	379	0	326	12	26	2

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	1.00	1.99	0.01	1.00	2.00	1.00	2.00	0.00	1.00	0.32	0.63	0.05
Final Sat.:	1750	3790	10	1750	3800	1750	3500	0	1750	553	1199	92

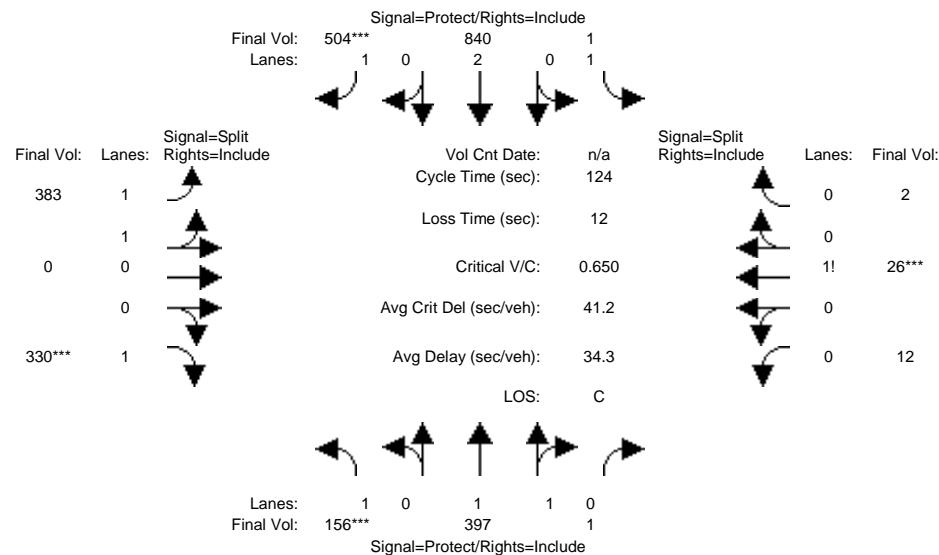
Capacity Analysis Module:

Vol/Sat:	0.09	0.10	0.10	0.00	0.22	0.29	0.11	0.00	0.19	0.02	0.02	0.02
Crit Moves:	****			****		****	****		****	****		****
Green Time:	15.9	44.2	44.2	23.8	52.1	52.1	34.0	0.0	34.0	10.0	10.0	10.0
Volume/Cap:	0.68	0.29	0.29	0.00	0.53	0.68	0.39	0.00	0.68	0.27	0.27	0.27
Uniform Del:	51.6	28.7	28.7	40.5	26.7	29.1	36.6	0.0	40.1	53.6	53.6	53.6
IncremntDel:	8.2	0.1	0.1	0.0	0.3	2.6	0.3	0.0	3.9	1.0	1.0	1.0
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00
Delay/Veh:	59.8	28.8	28.8	40.5	27.1	31.7	36.9	0.0	44.1	54.5	54.5	54.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	59.8	28.8	28.8	40.5	27.1	31.7	36.9	0.0	44.1	54.5	54.5	54.5
LOS by Move:	E	C	C	D	C	C	D	A	D	D	D	D
HCM2kAvgQ:	6	5	5	0	11	16	6	0	11	2	2	2

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + P PM

Intersection #31: Tantau Avenue and Vallco Parkway (CUP)



Street Name:	Tantau Avenue						Vallco Parkway								
	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	10	10	10	10	10	10	10	10	
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	

Volume Module:

Base Vol:	152	222	1	1	500	279	289	0	326	12	26	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	152	222	1	1	500	279	289	0	326	12	26	2
Added Vol:	0	175	0	0	340	221	90	0	0	0	0	0
Hyatt:	4	0	0	0	0	4	4	0	4	0	0	0
Initial Fut:	156	397	1	1	840	504	383	0	330	12	26	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	156	397	1	1	840	504	383	0	330	12	26	2
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	156	397	1	1	840	504	383	0	330	12	26	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	156	397	1	1	840	504	383	0	330	12	26	2

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	1.00	1.99	0.01	1.00	2.00	1.00	2.00	0.00	1.00	0.32	0.63	0.05
Final Sat.:	1750	3790	10	1750	3800	1750	3500	0	1750	553	1199	92

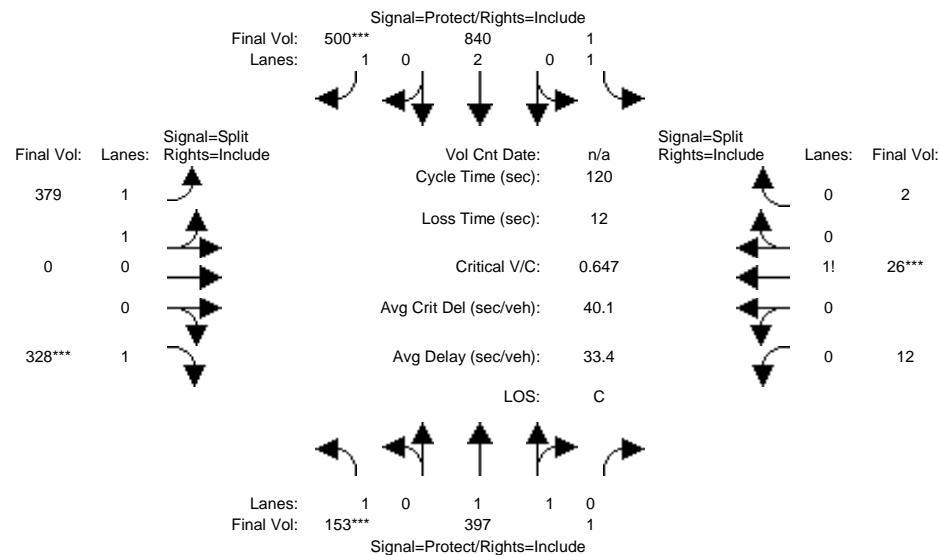
Capacity Analysis Module:

Vol/Sat:	0.09	0.10	0.10	0.00	0.22	0.29	0.11	0.00	0.19	0.02	0.02	0.02
Crit Moves:	****			****		****	****		****	****		****
Green Time:	16.1	44.2	44.2	23.8	51.9	51.9	34.0	0.0	34.0	10.0	10.0	10.0
Volume/Cap:	0.69	0.29	0.29	0.00	0.53	0.69	0.40	0.00	0.69	0.27	0.27	0.27
Uniform Del:	51.6	28.7	28.7	40.5	26.9	29.4	36.7	0.0	40.3	53.6	53.6	53.6
IncremntDel:	8.6	0.1	0.1	0.0	0.3	2.8	0.3	0.0	4.2	1.0	1.0	1.0
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00
Delay/Veh:	60.1	28.8	28.8	40.5	27.2	32.2	36.9	0.0	44.4	54.5	54.5	54.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	60.1	28.8	28.8	40.5	27.2	32.2	36.9	0.0	44.4	54.5	54.5	54.5
LOS by Move:	E	C	C	D	C	C	D	A	D	D	D	D
HCM2kAvgQ:	6	5	5	0	11	16	6	0	11	2	2	2

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative PM

Intersection #31: Tantau Avenue and Vallco Parkway (CUP)



Street Name:	Tantau Avenue					Vallco Parkway									
	North Bound			South Bound		East Bound			West Bound						
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	10	10	10	10	10	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	153	222	1	1	500	279	289	0	328	12	26	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	153	222	1	1	500	279	289	0	328	12	26	2
Added Vol:	0	175	0	0	340	221	90	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	153	397	1	1	840	500	379	0	328	12	26	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	153	397	1	1	840	500	379	0	328	12	26	2
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	153	397	1	1	840	500	379	0	328	12	26	2
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	153	397	1	1	840	500	379	0	328	12	26	2

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	1.00	1.99	0.01	1.00	2.00	1.00	2.00	0.00	1.00	0.32	0.63	0.05
Final Sat.:	1750	3790	10	1750	3800	1750	3500	0	1750	553	1199	92

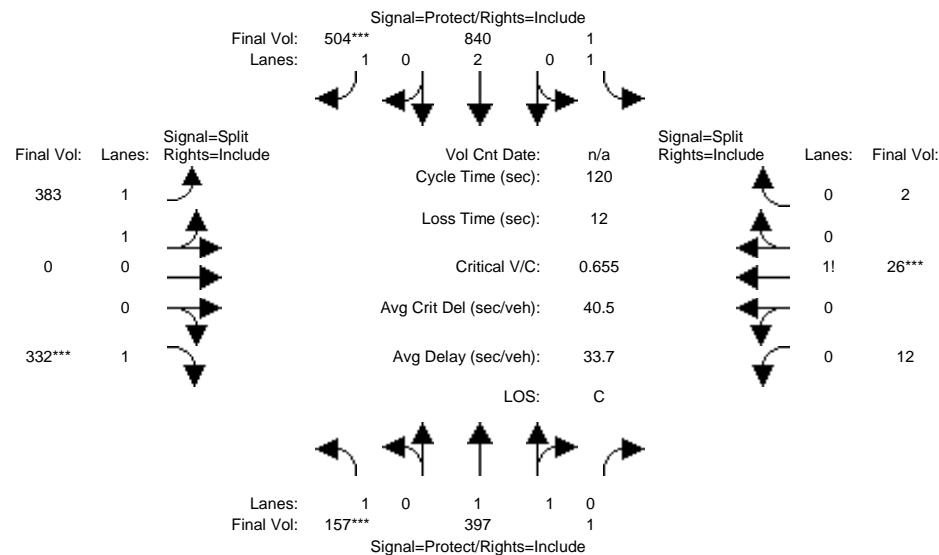
Capacity Analysis Module:

Vol/Sat:	0.09	0.10	0.10	0.00	0.22	0.29	0.11	0.00	0.19	0.02	0.02	0.02
Crit Moves:	****			****		****	****		****	****		****
Green Time:	15.3	41.9	41.9	23.3	49.9	49.9	32.8	0.0	32.8	10.0	10.0	10.0
Volume/Cap:	0.69	0.30	0.30	0.00	0.53	0.69	0.40	0.00	0.69	0.26	0.26	0.26
Delay/Veh:	58.7	28.5	28.5	39.0	26.6	31.4	35.8	0.0	43.2	52.4	52.4	52.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	58.7	28.5	28.5	39.0	26.6	31.4	35.8	0.0	43.2	52.4	52.4	52.4
LOS by Move:	E	C	C	D	C	C	D	A	D	D	D	D
HCM2kAvgQ:	5	5	5	0	11	16	6	0	11	2	2	2

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + P PM

Intersection #31: Tantau Avenue and Vallco Parkway (CUP)

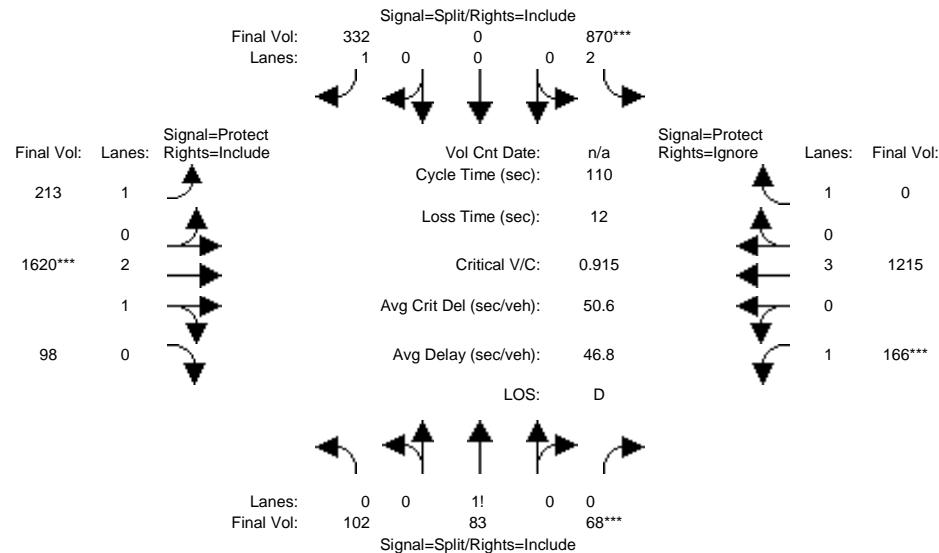


Street Name: Tantau Avenue Vallco Parkway															
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7 10		10 7		10 10		10 10		10 10		10 10		10 10		
Y+R:	4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		4.0 4.0		
Volume Module:	<hr/>														
Base Vol:	153	222	1	1	500	279	289	0	328	12	26	2			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	153	222	1	1	500	279	289	0	328	12	26	2			
Added Vol:	0	175	0	0	340	221	90	0	0	0	0	0			
Hyatt:	4	0	0	0	0	4	4	0	4	0	0	0			
Initial Fut:	157	397	1	1	840	504	383	0	332	12	26	2			
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
PHF Volume:	157	397	1	1	840	504	383	0	332	12	26	2			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	157	397	1	1	840	504	383	0	332	12	26	2			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
FinalVolume:	157	397	1	1	840	504	383	0	332	12	26	2			
Saturation Flow Module:	<hr/>														
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900			
Adjustment:	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92			
Lanes:	1.00	1.99	0.01	1.00	2.00	1.00	2.00	0.00	1.00	0.32	0.63	0.05			
Final Sat.:	1750	3790	10	1750	3800	1750	3500	0	1750	553	1199	92			
Capacity Analysis Module:	<hr/>														
Vol/Sat:	0.09	0.10	0.10	0.00	0.22	0.29	0.11	0.00	0.19	0.02	0.02	0.02			
Crit Moves:	****			****		****	****		****	****		****			
Green Time:	15.5	41.9	41.9	23.3	49.7	49.7	32.8	0.0	32.8	10.0	10.0	10.0			
Volume/Cap:	0.69	0.30	0.30	0.00	0.53	0.69	0.40	0.00	0.69	0.26	0.26	0.26			
Delay/Veh:	59.0	28.5	28.5	39.0	26.8	31.8	35.9	0.0	43.5	52.4	52.4	52.4			
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
AdjDel/Veh:	59.0	28.5	28.5	39.0	26.8	31.8	35.9	0.0	43.5	52.4	52.4	52.4			
LOS by Move:	E	C	C	D	C	C	D	A	D	D	D	D			
HCM2kAvgQ:	6	5	5	0	11	16	6	0	11	2	2	2			

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd PM

Intersection #32: Stevens Creek Blvd and Tantau Avenue (CUP)



Street Name:	Tantau Avenue					Stevens Creek Blvd									
Approach:	North Bound			South Bound		East Bound			West Bound						
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	7	10	10	7	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	102	75	68	595	0	266	185	1554	98	166	1169	147
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	102	75	68	595	0	266	185	1554	98	166	1169	147
Added Vol:	0	8	0	275	0	66	28	66	0	0	46	138
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	102	83	68	870	0	332	213	1620	98	166	1215	285
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Volume:	102	83	68	870	0	332	213	1620	98	166	1215	0
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	102	83	68	870	0	332	213	1620	98	166	1215	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
FinalVolume:	102	83	68	870	0	332	213	1620	98	166	1215	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	0.41	0.31	0.28	2.00	0.00	1.00	1.00	2.82	0.18	1.00	3.00	1.00
Final Sat.:	724	589	483	3150	0	1750	1750	5349	324	1750	5700	1750

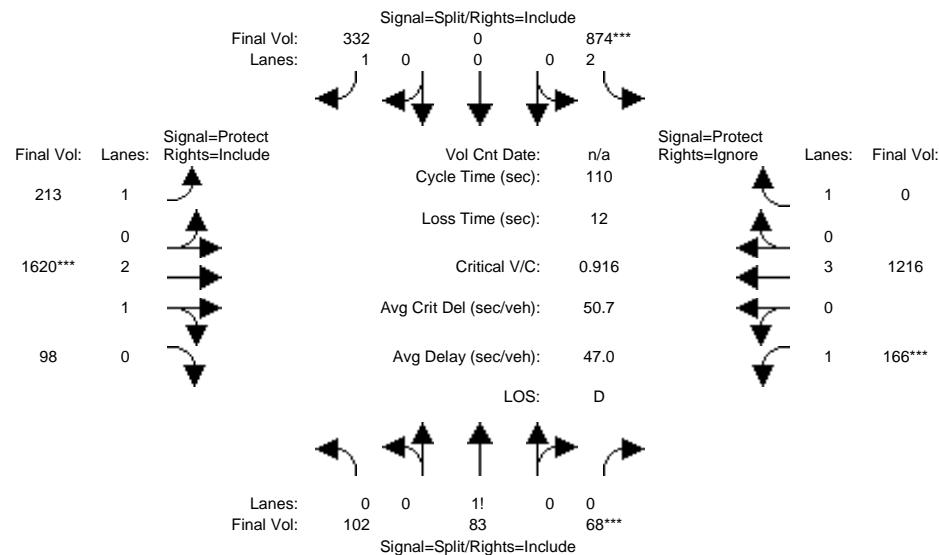
Capacity Analysis Module:

Vol/Sat:	0.14	0.14	0.14	0.28	0.00	0.19	0.12	0.30	0.30	0.09	0.21	0.00
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	16.9	16.9	16.9	33.2	0.0	33.2	17.4	36.4	36.4	11.4	30.5	0.0
Volume/Cap:	0.91	0.91	0.91	0.91	0.00	0.63	0.77	0.91	0.91	0.91	0.77	0.00
Uniform Del:	45.8	45.8	45.8	37.0	0.0	33.1	44.4	35.3	35.3	48.8	36.6	0.0
IncremntDel:	32.5	32.5	32.5	13.0	0.0	2.4	12.4	7.4	7.4	43.2	2.4	0.0
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Delay/Veh:	78.4	78.4	78.4	50.1	0.0	35.5	56.8	42.7	42.7	92.0	38.9	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	78.4	78.4	78.4	50.1	0.0	35.5	56.8	42.7	42.7	92.0	38.9	0.0
LOS by Move:	E	E	E	D	A	D	E	D	D	F	D	A
HCM2kAvgQ:	12	12	12	19	0	10	8	20	20	7	13	0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + P PM

Intersection #32: Stevens Creek Blvd and Tantau Avenue (CUP)



Street Name:	Tantau Avenue				Stevens Creek Blvd				
	North Bound		South Bound		East Bound		West Bound		
Movement:	L -	T -	R	L -	T -	R	L -	T -	R
Min. Green:	7	10	10	7	10	10	7	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	102	75	68	595	0	266	185	1554	98	166	1169	147
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	102	75	68	595	0	266	185	1554	98	166	1169	147
Added Vol:	0	8	0	275	0	66	28	66	0	0	46	138
Hyatt:	0	0	0	4	0	0	0	0	0	0	1	4
Initial Fut:	102	83	68	874	0	332	213	1620	98	166	1216	289
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Volume:	102	83	68	874	0	332	213	1620	98	166	1216	0
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	102	83	68	874	0	332	213	1620	98	166	1216	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
FinalVolume:	102	83	68	874	0	332	213	1620	98	166	1216	0

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	0.41	0.31	0.28	2.00	0.00	1.00	1.00	2.82	0.18	1.00	3.00	1.00
Final Sat.:	724	589	483	3150	0	1750	1750	5349	324	1750	5700	1750

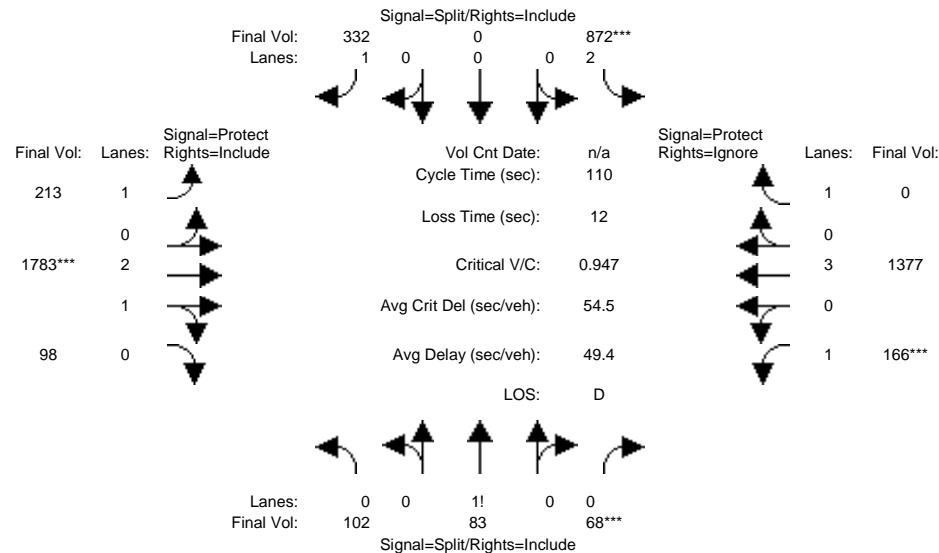
Capacity Analysis Module:

Vol/Sat:	0.14	0.14	0.14	0.28	0.00	0.19	0.12	0.30	0.30	0.09	0.21	0.00
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	16.9	16.9	16.9	33.3	0.0	33.3	17.4	36.4	36.4	11.4	30.4	0.0
Volume/Cap:	0.92	0.92	0.92	0.92	0.00	0.63	0.77	0.92	0.92	0.92	0.77	0.00
Uniform Del:	45.8	45.8	45.8	37.0	0.0	33.0	44.4	35.3	35.3	48.8	36.6	0.0
IncremntDel:	32.9	32.9	32.9	13.2	0.0	2.4	12.6	7.5	7.5	43.5	2.4	0.0
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Delay/Veh:	78.7	78.7	78.7	50.2	0.0	35.4	57.0	42.9	42.9	92.4	39.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	78.7	78.7	78.7	50.2	0.0	35.4	57.0	42.9	42.9	92.4	39.0	0.0
LOS by Move:	E	E	E	D	A	D	E	D	D	F	D	A
HCM2kAvgQ:	13	13	13	19	0	10	8	20	20	7	13	0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative PM

Intersection #32: Stevens Creek Blvd and Tantau Avenue (CUP)



Street Name:	Tantau Avenue				Stevens Creek Blvd										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	102	75	68	597	0	266	185	1717	98	166	1331	148			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	102	75	68	597	0	266	185	1717	98	166	1331	148			
Added Vol:	0	8	0	275	0	66	28	66	0	0	46	138			
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0			
Initial Fut:	102	83	68	872	0	332	213	1783	98	166	1377	286			
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00		
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00		
PHF Volume:	102	83	68	872	0	332	213	1783	98	166	1377	0			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	102	83	68	872	0	332	213	1783	98	166	1377	0			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00		
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00		
FinalVolume:	102	83	68	872	0	332	213	1783	98	166	1377	0			

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	0.41	0.31	0.28	2.00	0.00	1.00	1.00	2.83	0.17	1.00	3.00	1.00
Final Sat.:	724	589	483	3150	0	1750	1750	5379	296	1750	5700	1750

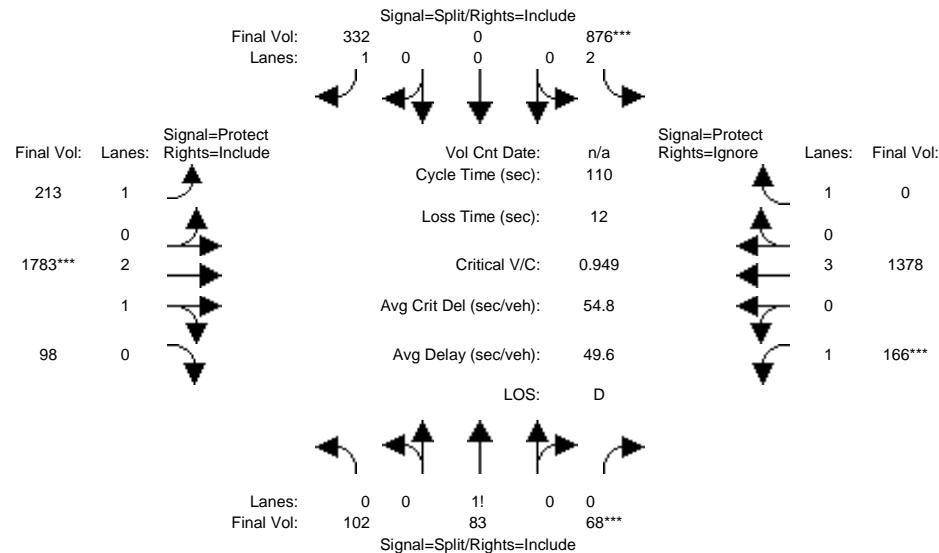
Capacity Analysis Module:

Vol/Sat:	0.14	0.14	0.14	0.28	0.00	0.19	0.12	0.33	0.33	0.09	0.24	0.00
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	16.4	16.4	16.4	32.1	0.0	32.1	16.6	38.5	38.5	11.0	32.9	0.0
Volume/Cap:	0.95	0.95	0.95	0.95	0.00	0.65	0.81	0.95	0.95	0.95	0.81	0.00
Delay/Veh:	87.2	87.2	87.2	56.3	0.0	36.9	61.7	45.2	45.2	101.8	38.6	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	87.2	87.2	87.2	56.3	0.0	36.9	61.7	45.2	45.2	101.8	38.6	0.0
LOS by Move:	F	F	F	E	A	D	E	D	D	F	D	A
HCM2kAvgQ:	13	13	13	20	0	10	8	23	23	7	15	0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + P PM

Intersection #32: Stevens Creek Blvd and Tantau Avenue (CUP)

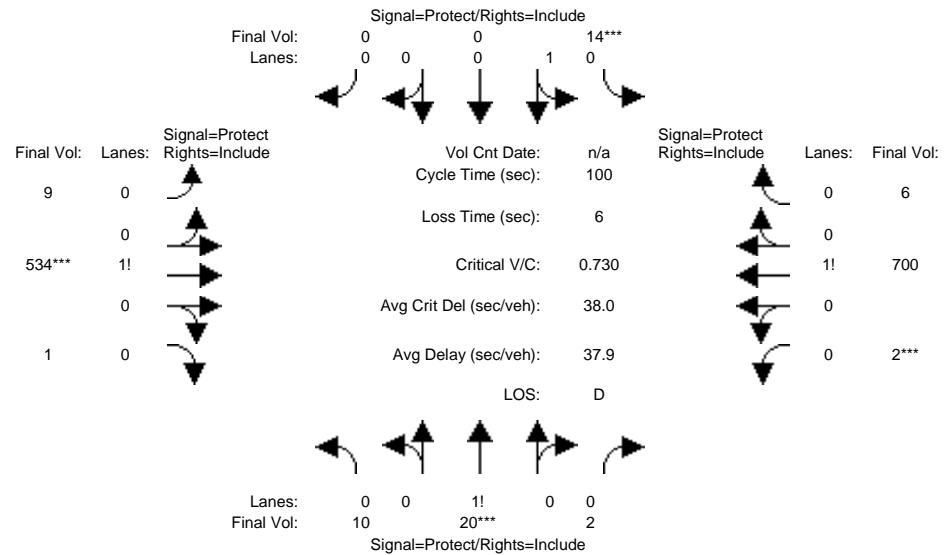


Street Name: Tantau Avenue Stevens Creek Blvd															
Approach:	North Bound			South Bound			East Bound			West Bound					
	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	7	10	10	7	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Volume Module:	<hr/>														
Base Vol:	102	75	68	597	0	266	185	1717	98	166	1331	148			
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Initial Bse:	102	75	68	597	0	266	185	1717	98	166	1331	148			
Added Vol:	0	8	0	275	0	66	28	66	0	0	46	138			
Hyatt:	0	0	0	4	0	0	0	0	0	0	1	4			
Initial Fut:	102	83	68	876	0	332	213	1783	98	166	1378	290			
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00			
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00			
PHF Volume:	102	83	68	876	0	332	213	1783	98	166	1378	0			
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0			
Reduced Vol:	102	83	68	876	0	332	213	1783	98	166	1378	0			
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00			
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00			
FinalVolume:	102	83	68	876	0	332	213	1783	98	166	1378	0			
Saturation Flow Module:	<hr/>														
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900			
Adjustment:	0.92	1.00	0.92	0.83	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92			
Lanes:	0.41	0.31	0.28	2.00	0.00	1.00	1.00	2.83	0.17	1.00	3.00	1.00			
Final Sat.:	724	589	483	3150	0	1750	1750	5379	296	1750	5700	1750			
Capacity Analysis Module:	<hr/>														
Vol/Sat:	0.14	0.14	0.14	0.28	0.00	0.19	0.12	0.33	0.33	0.09	0.24	0.00			
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****			
Green Time:	16.3	16.3	16.3	32.2	0.0	32.2	16.6	38.4	38.4	11.0	32.9	0.0			
Volume/Cap:	0.95	0.95	0.95	0.95	0.00	0.65	0.81	0.95	0.95	0.95	0.81	0.00			
Delay/Veh:	87.6	87.6	87.6	56.5	0.0	36.8	62.0	45.4	45.4	102.2	38.7	0.0			
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
AdjDel/Veh:	87.6	87.6	87.6	56.5	0.0	36.8	62.0	45.4	45.4	102.2	38.7	0.0			
LOS by Move:	F	F	F	E	A	D	E	D	D	F	D	A			
HCM2kAvgQ:	13	13	13	20	0	10	8	23	23	7	15	0			

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd PM

Intersection #501: Finch Road and Vallco Pkwy



Approach:	North Bound			South Bound			East Bound			West Bound			
	Movement:	L -	T -	R	L -	T -	R	L -	T -	R	L -	T -	R
Min. Green:		7	10	10	7	10	10	7	10	10	7	10	10
Y+R:		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Volume Module:													
Base Vol:		10	20	2	14	0	0	9	444	1	2	479	6
Growth Adj:		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:		10	20	2	14	0	0	9	444	1	2	479	6
Added Vol:		0	0	0	0	0	0	0	90	0	0	221	0
PasserByVol:		0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:		10	20	2	14	0	0	9	534	1	2	700	6
User Adj:		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:		10	20	2	14	0	0	9	534	1	2	700	6
Reduc Vol:		0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:		10	20	2	14	0	0	9	534	1	2	700	6
PCE Adj:		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:		10	20	2	14	0	0	9	534	1	2	700	6

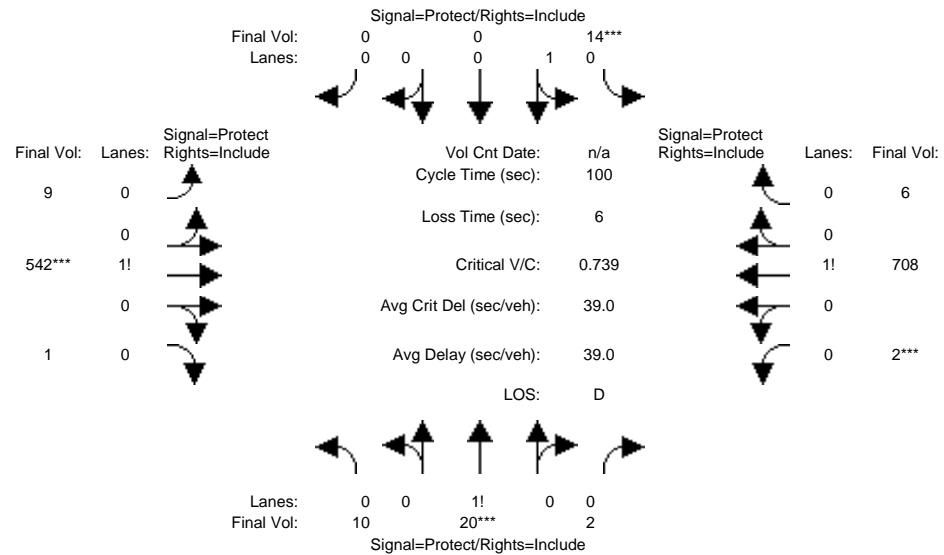
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.90	0.98	0.90	0.88	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92
Lanes:	0.33	0.60	0.07	1.00	0.00	0.00	0.01	0.98	0.01	0.01	0.98	0.01
Final Sat.:	562	1123	112	1663	0	0	31	1860	3	5	1875	16

Capacity Analysis Module:												
Vol/Sat:	0.02	0.02	0.02	0.01	0.00	0.00	0.29	0.29	0.29	0.37	0.37	0.37
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	17.0	10.0	10.0	7.0	0.0	0.0	33.5	33.5	33.5	43.5	43.5	43.5
Volume/Cap:	0.10	0.18	0.18	0.12	0.00	0.00	0.86	0.86	0.86	0.86	0.86	0.86
Uniform Del:	35.1	41.2	41.2	43.6	0.0	0.0	31.0	31.0	31.0	25.4	25.4	25.4
IncremntDel:	0.2	0.5	0.5	0.5	0.0	0.0	11.2	11.2	11.2	8.9	8.9	8.9
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	35.2	41.7	41.7	44.1	0.0	0.0	42.3	42.3	42.3	34.4	34.4	34.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	35.2	41.7	41.7	44.1	0.0	0.0	42.3	42.3	42.3	34.4	34.4	34.4
LOS by Move:	D	D	D	D	A	A	D	D	D	C	C	C
HCM2kAvgQ:	1	1	1	1	0	0	17	17	17	21	21	21

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + P PM

Intersection #501: Finch Road and Vallco Pkwy



Approach:	North Bound			South Bound			East Bound			West Bound					
	Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-
Min. Green:		7	10	10	7	10	10	7	10	10	10	7	10	10	10
Y+R:		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Volume Module:															
Base Vol:		10	20	2	14	0	0	9	444	1	2	479	6		
Growth Adj:		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:		10	20	2	14	0	0	9	444	1	2	479	6		
Added Vol:		0	0	0	0	0	0	0	90	0	0	221	0		
Hyatt:		0	0	0	0	0	0	0	8	0	0	8	0		
Initial Fut:		10	20	2	14	0	0	9	542	1	2	708	6		
User Adj:		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:		10	20	2	14	0	0	9	542	1	2	708	6		
Reduc Vol:		0	0	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:		10	20	2	14	0	0	9	542	1	2	708	6		
PCE Adj:		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:		10	20	2	14	0	0	9	542	1	2	708	6		

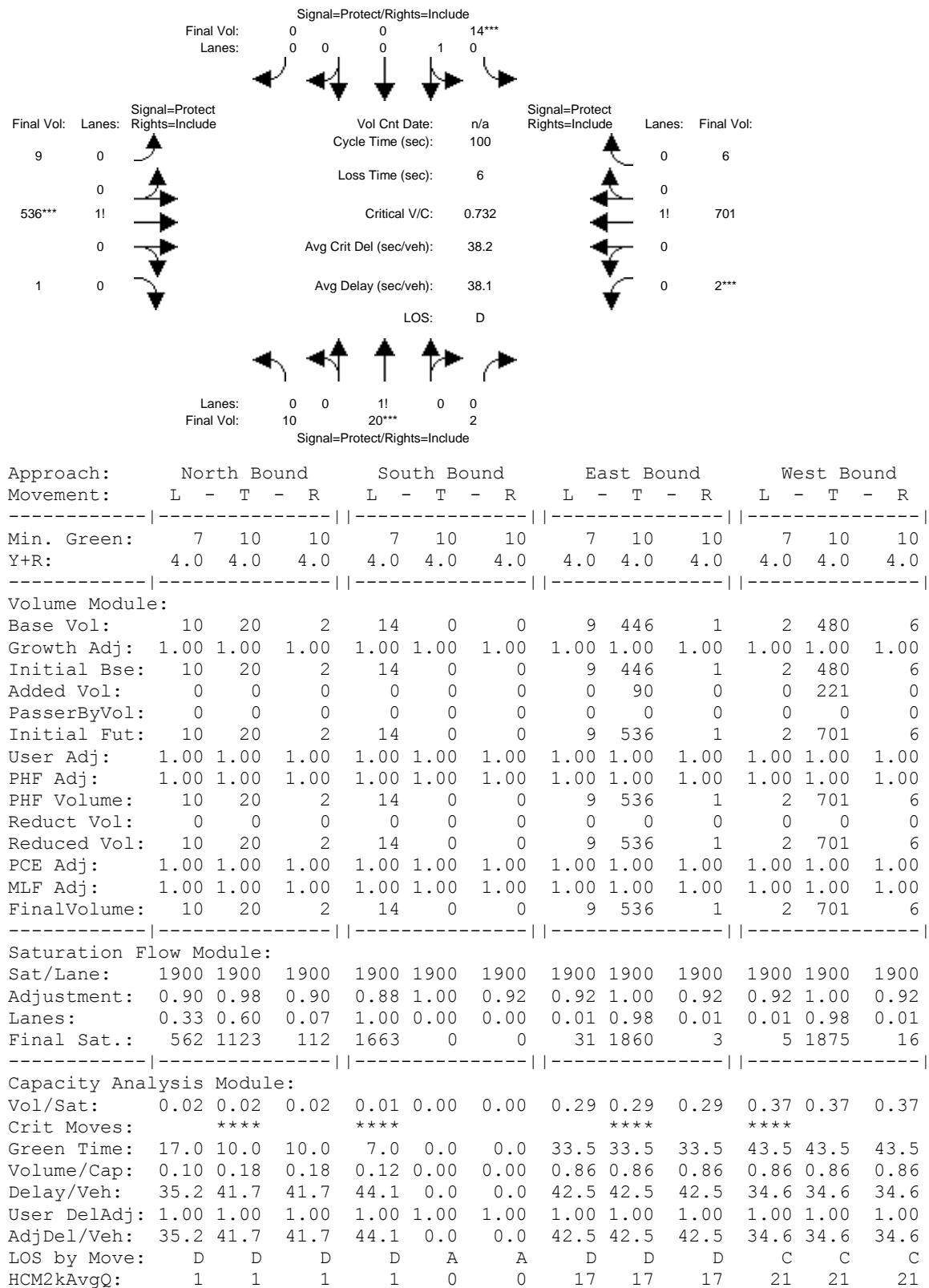
Saturation Flow Module:														
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.90	0.98	0.90	0.88	1.00	0.92	0.92	1.00	0.92	0.92	1.00	0.92	1.00	0.92
Lanes:	0.33	0.60	0.07	1.00	0.00	0.00	0.01	0.98	0.01	0.01	0.98	0.01		
Final Sat.:	562	1123	112	1663	0	0	31	1861	3	5	1875	16		

Capacity Analysis Module:														
Vol/Sat:	0.02	0.02	0.02	0.01	0.00	0.00	0.29	0.29	0.29	0.38	0.38	0.38		
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****		
Green Time:	17.0	10.0	10.0	7.0	0.0	0.0	33.5	33.5	33.5	43.5	43.5	43.5		
Volume/Cap:	0.10	0.18	0.18	0.12	0.00	0.00	0.87	0.87	0.87	0.87	0.87	0.87	0.87	
Uniform Del:	35.1	41.2	41.2	43.6	0.0	0.0	31.2	31.2	31.2	25.7	25.7	25.7		
IncremntDel:	0.2	0.5	0.5	0.5	0.0	0.0	12.3	12.3	12.3	9.8	9.8	9.8		
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Delay Adj:	1.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Delay/Veh:	35.2	41.7	41.7	44.1	0.0	0.0	43.4	43.4	43.4	35.5	35.5	35.5		
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
AdjDel/Veh:	35.2	41.7	41.7	44.1	0.0	0.0	43.4	43.4	43.4	35.5	35.5	35.5		
LOS by Move:	D	D	D	D	A	A	D	D	D	D	D	D		
HCM2kAvgQ:	1	1	1	1	0	0	17	17	17	21	21	21		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative PM

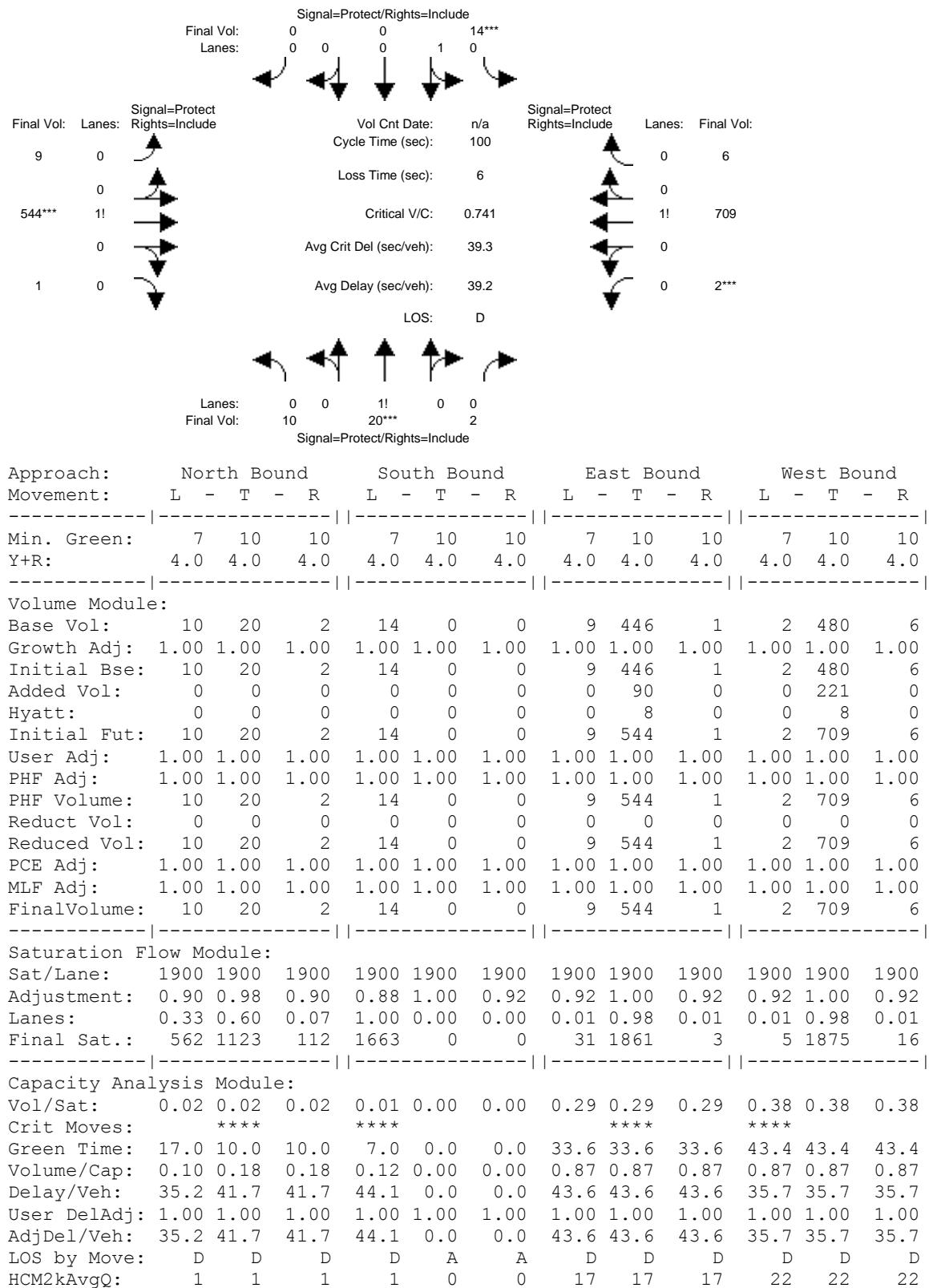
Intersection #501: Finch Road and Vallco Pkwy



Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + P PM

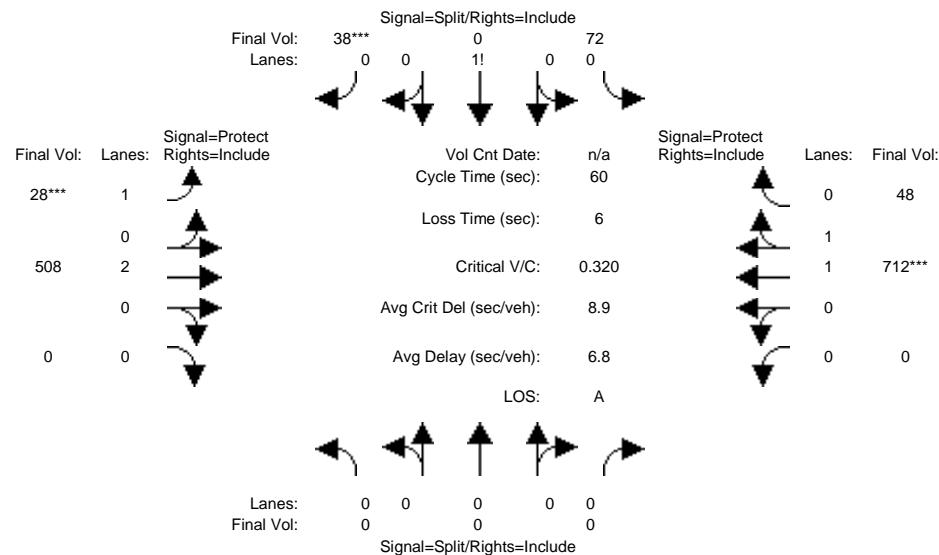
Intersection #501: Finch Road and Vallco Pkwy



Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd PM

Intersection #504: Perimeter Road and Vallco Pkwy (CUP)



Street Name:	Perimeter Rd				Vallco Pkwy										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	7	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	0	0	72	0	38	28	418	0	0	491	48				
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
Initial Bse:	0	0	0	72	0	38	28	418	0	0	491	48				
Added Vol:	0	0	0	0	0	0	0	90	0	0	221	0				
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0				
Initial Fut:	0	0	0	72	0	38	28	508	0	0	712	48				
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
PHF Volume:	0	0	0	72	0	38	28	508	0	0	712	48				
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0				
Reduced Vol:	0	0	0	72	0	38	28	508	0	0	712	48				
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
FinalVolume:	0	0	0	72	0	38	28	508	0	0	712	48				

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.85	1.00	0.85	0.88	1.00	0.92	0.92	0.99	0.91
Lanes:	0.00	0.00	0.00	0.65	0.00	0.35	1.00	2.00	0.00	0.00	1.86	0.14
Final Sat.:	0	0	0	1057	0	558	1663	3800	0	0	3509	237

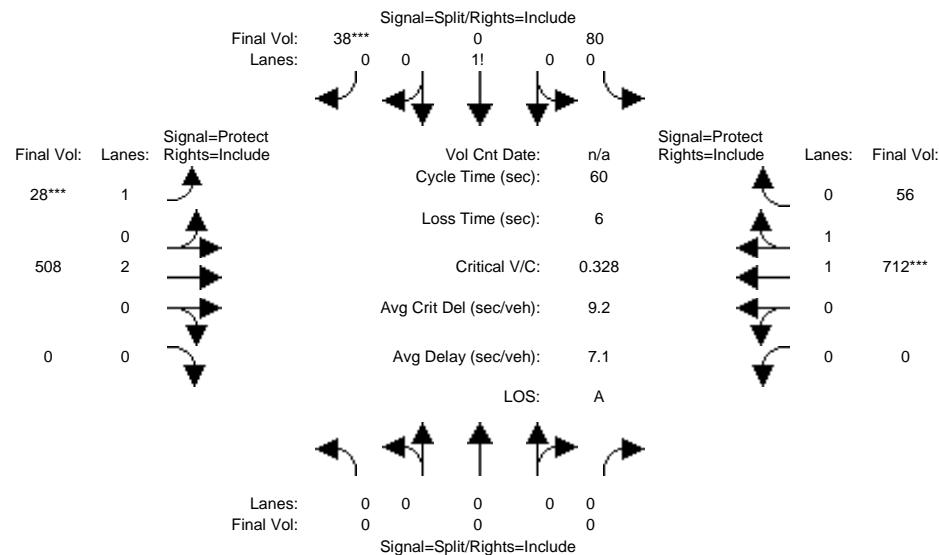
Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.07	0.00	0.07	0.02	0.13	0.00	0.00	0.20	0.20
Crit Moves:						****	****				****	
Green Time:	0.0	0.0	0.0	11.8	0.0	11.8	7.0	42.2	0.0	0.0	35.2	35.2
Volume/Cap:	0.00	0.00	0.00	0.35	0.00	0.35	0.14	0.19	0.00	0.00	0.35	0.35
Uniform Del:	0.0	0.0	0.0	20.8	0.0	20.8	23.8	3.1	0.0	0.0	6.4	6.4
IncremntDel:	0.0	0.0	0.0	0.7	0.0	0.7	0.3	0.0	0.0	0.0	0.1	0.1
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00
Delay/Veh:	0.0	0.0	0.0	21.4	0.0	21.4	24.2	3.1	0.0	0.0	6.5	6.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	21.4	0.0	21.4	24.2	3.1	0.0	0.0	6.5	6.5
LOS by Move:	A	A	A	C	A	C	C	A	A	A	A	A
HCM2kAvgQ:	0	0	0	2	0	2	0	2	0	0	3	3

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Bkgd + P PM

Intersection #504: Perimeter Road and Vallco Pkwy (CUP)



Street Name:	Perimeter Rd				Vallco Pkwy										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	7	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	0	0	72	0	38	28	418	0	0	491	48
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	72	0	38	28	418	0	0	491	48
Added Vol:	0	0	0	0	0	0	0	90	0	0	221	0
Hyatt:	0	0	0	8	0	0	0	0	0	0	0	8
Initial Fut:	0	0	0	80	0	38	28	508	0	0	712	56
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	80	0	38	28	508	0	0	712	56
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	80	0	38	28	508	0	0	712	56
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	80	0	38	28	508	0	0	712	56

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.85	1.00	0.85	0.88	1.00	0.92	0.92	0.99	0.91
Lanes:	0.00	0.00	0.00	0.68	0.00	0.32	1.00	2.00	0.00	0.00	1.84	0.16
Final Sat.:	0	0	0	1098	0	522	1663	3800	0	0	3463	272

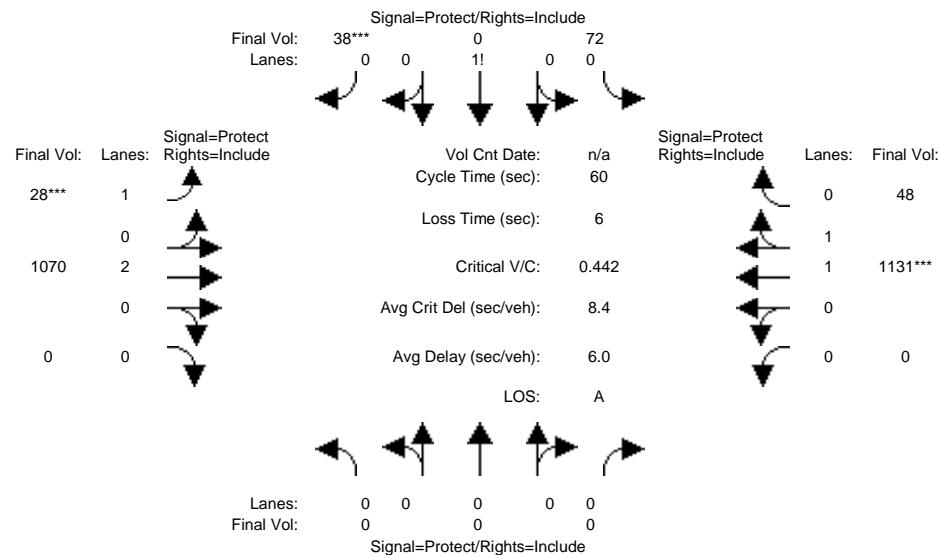
Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.07	0.00	0.07	0.02	0.13	0.00	0.00	0.21	0.21
Crit Moves:						****	****				****	
Green Time:	0.0	0.0	0.0	12.3	0.0	12.3	7.0	41.7	0.0	0.0	34.7	34.7
Volume/Cap:	0.00	0.00	0.00	0.36	0.00	0.36	0.14	0.19	0.00	0.00	0.36	0.36
Uniform Del:	0.0	0.0	0.0	20.5	0.0	20.5	23.8	3.2	0.0	0.0	6.7	6.7
IncremntDel:	0.0	0.0	0.0	0.7	0.0	0.7	0.3	0.0	0.0	0.0	0.1	0.1
InitQueueDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00
Delay/Veh:	0.0	0.0	0.0	21.1	0.0	21.1	24.2	3.3	0.0	0.0	6.8	6.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	21.1	0.0	21.1	24.2	3.3	0.0	0.0	6.8	6.8
LOS by Move:	A	A	A	C	A	C	C	A	A	A	A	A
HCM2kAvgQ:	0	0	0	2	0	2	0	2	0	0	4	4

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative PM

Intersection #504: Perimeter Road and Vallco Pkwy (CUP)



Street Name:	Perimeter Rd				Vallco Pkwy										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	0	0	72	0	38	28	980	0	0	910	48
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	72	0	38	28	980	0	0	910	48
Added Vol:	0	0	0	0	0	0	0	90	0	0	221	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	0	0	72	0	38	28	1070	0	0	1131	48
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	72	0	38	28	1070	0	0	1131	48
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	72	0	38	28	1070	0	0	1131	48
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	72	0	38	28	1070	0	0	1131	48

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.85	1.00	0.85	0.88	1.00	0.92	0.92	0.99	0.92
Lanes:	0.00	0.00	0.00	0.65	0.00	0.35	1.00	2.00	0.00	0.00	1.91	0.09
Final Sat.:	0	0	0	1057	0	558	1663	3800	0	0	3611	153

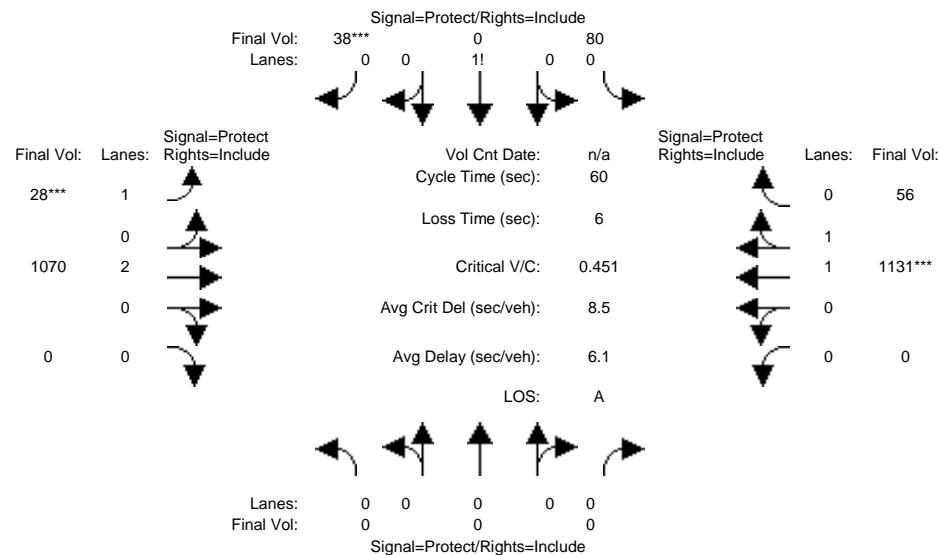
Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.07	0.00	0.07	0.02	0.28	0.00	0.00	0.31	0.31
Crit Moves:						****	****				****	
Green Time:	0.0	0.0	0.0	10.0	0.0	10.0	7.0	44.0	0.0	0.0	37.0	37.0
Volume/Cap:	0.00	0.00	0.00	0.41	0.00	0.41	0.14	0.38	0.00	0.00	0.51	0.51
Delay/Veh:	0.0	0.0	0.0	23.4	0.0	23.4	24.2	3.1	0.0	0.0	6.6	6.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	23.4	0.0	23.4	24.2	3.1	0.0	0.0	6.6	6.6
LOS by Move:	A	A	A	C	A	C	C	A	A	A	A	A
HCM2kAvgQ:	0	0	0	2	0	2	0	4	0	0	6	6

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Operations (Future Volume Alternative)
Cumulative + P PM

Intersection #504: Perimeter Road and Vallco Pkwy (CUP)



Street Name:	Perimeter Rd				Vallco Pkwy										
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Min. Green:	7	10	10	7	10	10	7	10	10	10	7	10	10	10	10
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0

Volume Module:

Base Vol:	0	0	0	72	0	38	28	980	0	0	910	48
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	72	0	38	28	980	0	0	910	48
Added Vol:	0	0	0	0	0	0	0	90	0	0	221	0
Hyatt:	0	0	0	8	0	0	0	0	0	0	0	8
Initial Fut:	0	0	0	80	0	38	28	1070	0	0	1131	56
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	80	0	38	28	1070	0	0	1131	56
Reduc Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	80	0	38	28	1070	0	0	1131	56
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	80	0	38	28	1070	0	0	1131	56

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.85	1.00	0.85	0.88	1.00	0.92	0.92	0.99	0.91
Lanes:	0.00	0.00	0.00	0.68	0.00	0.32	1.00	2.00	0.00	0.00	1.90	0.10
Final Sat.:	0	0	0	1098	0	522	1663	3800	0	0	3581	177

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.00	0.07	0.00	0.07	0.02	0.28	0.00	0.00	0.32	0.32
Crit Moves:						****	****				****	
Green Time:	0.0	0.0	0.0	10.0	0.0	10.0	7.0	44.0	0.0	0.0	37.0	37.0
Volume/Cap:	0.00	0.00	0.00	0.44	0.00	0.44	0.14	0.38	0.00	0.00	0.51	0.51
Delay/Veh:	0.0	0.0	0.0	23.6	0.0	23.6	24.2	3.1	0.0	0.0	6.6	6.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	23.6	0.0	23.6	24.2	3.1	0.0	0.0	6.6	6.6
LOS by Move:	A	A	A	C	A	C	C	A	A	A	A	A
HCM2kAvgQ:	0	0	0	3	0	3	0	4	0	0	6	6

Note: Queue reported is the number of cars per lane.

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